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RED RIVER WATERSHED

DIERKS LAKE

SALINE RIVER, ARKANSAS

EMBANKMENT CRITERIA AND PERFORMANCE REPORT

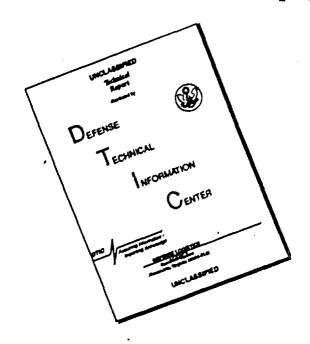


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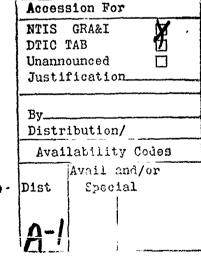
DIERKS DAM AND RESERVOIR SALINE RIVER, ARKANSAS

EMBANKMENT CRITERIA AND PERFORMANCE REPORT

October 1987

U. S. Army Engineer District, Little Rock

Corps of Engineers



Prepared by Grimes & Johnson, Inc. Little Rock, Arkansas and Grubbs, Garner & Hoskyn, Inc. Little Rock, Arkansas

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DIERKS DAM AND RESERVOIR SALINE RIVER, ARKANSAS

EMBANKMENT CRITERIA AND PERFORMANCE REPORT

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DIERKS DAM AND RESERVOIR SALINE RIVER, ARKANSAS

EMBANKMENT CRITERIA AND PERFORMANCE REPORT

PERTINENT DATA

LOCATION

Saline River mile 56.5, a tributary of the Little River, in Sevier and Howard Counties, Arkansas.

AUTHORITY

Authorized by the Flood Control Act of 3 July 1958

TYPE OF PROJECT

Rockfill dam with compacted-impervious earth core, uncontrolled spillway, outlet works, and supporting facilities.

DRAINAGE AREA

114 square miles

ELEVATIONS, AREAS AND STORAGES

	:	Elevation	:	Area	:	Sto	ra	ge
Feature	: (feet,m.s.l.	<u>):</u>	Acres):(Acre-feet	<u>):</u>	(inches
	:		:		:		:	
Top of Dam	:	593.0	:	-	:	-	:	-
Maximum pool	:	587.9	:	5,400	:	221,600	:	36.45
Spillway crest	:	575.0	:	4,260	:	159,500	:	26.23
Top flood control pool	:	557.5	:	2,970	:	96,800	:	15.92
Top conservation pool	:	526.0	:	1,360	:	29,700	:	4.88
Top inactive pool	:	512.0	:	810	:	14,600	:	2.40
Flood control storage	:	526.0-557.5	:	-	:	67,100	:	11.04
Water supply storage	:	512.0-526.0	:	-	:	15,100	:	2.48
50-year pool	:	564.5	:	3,440	:	119,100	:	19.59
•	:		:	•	:	•	:	

SPILLWAY DESIGN FLOOD

Peck flow into full pool
Volume in full pool
Total runoff
Peak flow, natural
Reservoir operational channel capacity

210,000 c.f.s 206,900 acre-feet 34.03 inches 202,000 c.f.s. 1,000 c.f.s.

PERTINENT DATA (Cont.)

HYDRAULIC DESIGN DATA

Spillway	
Discharge at maximum pool	102,000 c.f.s.
Design head	12.9 feet
Outlet works	
Discharge at maximum pool	2,640 c.f.s.
Discharge at top of flood control pool	2,370 c.f.s.
Discharge at top of conservation pool	1,980 c.f.s.
Low flow discharge at bottom of conservation pool	140 c.f.s.

STRUCTURES

Main dam	
Туре	Rockfill with impervious earth core
Height	
Maximum above streambed	153 feet
Average above valley floor	119 feet
Crest width	32 feet
Roadway width	24 feet
Length (main dam)	1,830 feet
Length (right embankment)	670 feet
Height (right embankment)	113 feet
Crest elevation	593 feet, m.s.l.
Spillway	
Туре	Uncontrolled
Location	Right Abutment
Width	780 feet
Crest elevation	575 feet, m.s.l.
Outlet works	
Type	Oblong conduit
Size	6 feet by 9 feet
Control	2-service, 2-emergency 3'-3" x 8'-0" hydraulic slide gates

1. INTRODUCTION

1.1 <u>Purpose and Scope of Report</u>. This report provides a summary record of significant design, construction and operational data on the Dierks Dam and Reservoir. It was prepared in accordance with ER 1110-2-1901, "Embankment Criteria and Performance Report," dated 31 December 1981 and is for use by engineers to familiarize themselves with the project, reevaluate the embankment when needed, and for guidance in design of comparable future projects. This report was prepared jointly by Grimes & Johnson, Inc. and Grubbs, Garner & Hoskyn, Inc., for the Soils and Materials Section, Little Rock District, under the provisions of Contract No. DACW38-86-D-0070, Delivery Order No. 0007.

The report presents a general description of the foundation conditions, type of material and placement methods of the various sections of the embankment, the design considerations on stability and seepage control, significant operational events, and an evaluation of the condition of the embankment. Pertinent drawings design and construction data, and photos are also included. More detailed descriptions of the foundation conditions are contained in the Dierks Dam foundation reports: Part I - "Embankment Foundation Grouting," dated April 1974, and Part III - "Embankment Foundation and Spillway," dated September 1978.

1.2 <u>Brief Description and Purpose of Project</u>. Dierks Dam is located at river mile 56.5 of the Saline River in Sevier and Howard Counties in west-central Arkansas, a tributary of Little River. The dam and reservoir are located about 5 miles northwest of the town of Dierks and about 66 miles north of Texarkana, Arkansas.

The major structures include a 780-foot-wide uncontrolled spillway with weir crest at elevation 575.0, a 670-foot-long right embankment located in a saddle between the spillway and the hill forming the right abutment of the main embankment, a 1,830-foot-long main embankment rising to a maximum of 153 feet above the riverbed, and an outlet works consisting of a 6-foot by 9-foot oblong reinforced concrete conduit controlled by two service and two emergency hydraulically operated slide gates. Other structures consist of an administration and maintenance building, paint and oil storage building,

gasoline pump and storage facilities, vehicle parking area, and public overlook facilities.

The purpose of the project is flood control and other beneficial public uses, which include water supply, recreation and fish and wildlife. The recreation area, operated and maintained by the Corps of Engineers, provides facilities year-round.

- 1.3 <u>Project Authorization</u>. The Dierks Dam and Reservoir project was authorized by the Flood Control Act approved 3 July 1958 (Public Law 85-500, 85th Congress, 2d Session). Related legislation included (a) House Document No. 170, the report "Millwood Reservoir and Alternate Reservoirs, Little River, Oklahoma and Arkansas," and (b) Public Law 87-88, which concerns requirements of local cooperation by State or local interests with regard to the water supply features of the project.
- 1.4 <u>Project Construction History</u>. Construction was initiated on the right and left abutment access roads in June 1968 and completed in March 1972. Construction of the main embankment, outlet works, spillway and project buildings was initiated in June 1970 and completed in July 1975. The maximum pool of record occurred on December 5, 1982, and reached elevation 558.0 NGVD.

A summary of construction contracts for the entire project is contained on the following page.

LIST OF PROJECT CONTRACTS

CONTRACT NO.	CONSTRUCTION FEATURE	CONTRACT AMOUNT	START DATE	COMPLETION DATE
DACW56-68-C-0235	Left Abutment Access	\$ 582,985	29 May 68	1 Mar 72
Contractor: E. W	. Blair, Inc., P.O. Bo	x 450, Broken	Bow, OK	
DACW56-70-C-0159	Embankment, Outlet Works, Spillway & Project Buildings	\$ 7,915,460	19 June 70	11 July 7
Contractor: Amis	Construction Co., P.	0. Box 1871,	Oklahoma City	y, OK 73104
DACW56-74-0066	Landscaping of Project Bldg. Area	\$ 19,636	19 Oct 73	18 Dec 74
Contractor: Gree	n Country Landscaping,	P. 0. Box 60	5, Muskogee,	ок
DACW56-75-C-0080	Public Use Areas Stage I, Roads and Parking	\$ 255,323	28 Feb 75	1 Mar 76
Contractor: AAA	Excavating Company, 80	O Arch Street	, Little Rocl	k, AR 72201
DACW56-75-C-0196	Public Use Areas Stage I, Jefferson Ridge & Blue Ridge	\$ 572,527	May 75	Feb 76
Contractor: W. R	. Austin Const. Co., P	. O. Box 998,	Shawnee, OK	74801
DACW56-76-C-0092	Public Use Areas Downstream Yorseshoe Bend	\$ 300,000	10 Jan 76	27 Oct 76
Contractor: LaFo	rge & Budd Const. Co.,	P. 0. Box 88	3, Parsons, F	(S 67357

1.5 <u>Periodic Inspections</u>. In accordance with the requirements for periodic inspection and continuing evaluation of completed civil works structures, the following reports have been published for periodic inspections performed on the embankment, spillway and outlet works at Dierks Dam and Reservoir. They contain the problems developed after construction and instrumentation data.

Periodic Inspection Report No. 1

Periodic Inspection Report No. 2

February 1976

Periodic Inspection Report No. 3

June 1977

Periodic Inspection Report No. 4

November 1979

Periodic Inspection Report No. 5

April 1983

Periodic Inspection Report No. 6

March 1985

2. GEOLOGY

- 2.1 Physiography. Dierks dam is situated on the Athens Plateau, a dissected piedmont belt about 15 miles wide located between the Ouachita Mountain region to the north and the Gulf Coastal Plain to the south. Saline River, which heads in the Cossatot Mountains to the north, flows southerly across the Ouachita Mountain region to its confluence with Little River about 30 miles downstream from the dam. Drainage in the area features a trellis pattern and is expressed topographically by rugged east-west ridges These alternate ridges and valleys are a reflection of the and valleys. differential erosion of the soft and hard beds that comprise the foundation. Only an occasional gap breaks the continuity of the ridges. The trunk streams or consequent streams, such as the Saline River, presently flow in rather crooked channels nearly coincident with their ancestral courses. ancestral courses were eroded into relatively soft Cretaceous formations but subsequent erosion removed the Cretaceous, and the streams presently flow on the underlying, upturned Paleozoic strata.
- 2.2 Overburden. Except for the alluvial material deposited in the flood plain of the Saline River, there is very little overburden. The right (west) abutment supports only the residual clay, sand, and sandstone boulders that can be held in place by vegetation, and the left (east) abutment supports only the residual materials that have accumulated between sandstone outcrops. These deposits range from 2 to 10 feet in thickness and consist of weathered sandstone fragments in a clayey sand matrix. The flood plain alluvium averages about 20 feet in thickness and consists of sandy clay or clayey sand overlying a heterogeneous mixture of weathered sandstone gravels, cobbles, and boulders.
- 2.3 <u>Geological Formations</u>. Three formations are exposed in the vicinity of the dam. The Stanley shale (Mississippian age), which is about 6,000 feet thick, consists of shale and interbedded sandstone, and outcrops a short distance upstream of the dam. The Jackfork formation (Mississippian age) also about 6,000 feet thick, consists of quartzitic sandstone and shale interbeds, conformably overlies the Stanley, and forms the bedrock for the dam and spillway. The Pike gravel member of the Trinity formation (Cretaceous age) is comprised of gravel, sand, and clay and unconformably overlies the Jackfork

formation. Remnants of this member cap the hills and ridges a short distance south and west of the dam. The Stanley and Jackfork formations, along with the other pre-Cretaceous deposits in the Ouachita Mountain region, have undergone intense lateral compression resulting in a series of anticlinal and synclinal folds. Evidence of this activity can be seen in the area to the north of the dam. At the damsite, the rocks strike northeast-southwest and dip steeply to the south. There is no evidence of any significant faulting within the damsite although jointing is quite prominent in the hard sandstone beds.

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2.4 Stratigraphy. The Jackfork formation, which comprises the foundation rock for the dam and the appurtenant structures, is interbedded sandstone, shaly or argillaceous sandstone, and shale. The sandstone is hard to very hard, light gray, highly quartizitic, and forms ridges up to 15 feet wide that strike almost parallel to the dam axis. These ridges run the entire length of the dam. In some places they form the upstream and/or downstream faces of the cutoff trench. The shaly sandstone is hard, gray to dark gray, and includes zones of shaly inclusions and partings. The shale is moderately hard, slightly fissile, dark gray to black, with occasional thin sandstone seams. Joints, frequently lined with crystals of quartz, calcite, or pyrite, were found throughout the formation.

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There was no evidence of faulting at the 2.5 Foundation Structure. This absence of faulting can be attributed to the presence of interbeds of shale which can absorb the compressive stresses (crustal shortening) by crumpling and shearing rather than rupturing. The strikes observed in the general area were found to be quite variable, ranging from about North 60° East to North 75° East. The dip of the rock varies from about 50° to 60° to the south. Because of the steep dip, any slight change in the strike of the strata gives an abrupt change in the apparent dip in the geologic sections. Investigation of the Jackfork sandstone outcrops at the damsite revealed three joint sets. The principal set strikes about North 80° East, which is slightly oblique to the strike of the rock, and dips 50° to 60° to the north. A secondary set strikes roughly North 10° West and dips steeply to the west. The third set, less apparent than the other two, strikes about parallel to the bedding but dips no nerly. Jointing in the sandstone beds adjacent to the river valley appears to have been affected by elastic

rebound in that the joints are more open and partially rearranged. Jointing was not prominent in the shale, but numerous slickensided planes found throughout the shales give evidence of some differential adjustments, possibly resulting from past regional tectonic activity.

Weathering. The foundation rock has been subjected to both chemical The chemical weathering has occurred by both and mechanical weathering. hydration and oxidation. Mechanical weathering, aided slightly by hydration which produces a swelling that widens existing joints and fractures, is evident only at 2- to 3-foot depths. The weathered sandstones are generally moderately hard, tan to rust-brown, with clay-lined, highly stained joints throughout. The weathered shale beds are soft, tan to gray, highly stained, and where highly weathered are sometimes completely altered to clay. bedrock comprising the abutments at the damsite is chemically weathered to a depth of about 30 feet. Below 30 feet the rock is usually fresh except for staining and minor rock softening adjacent to and on the joint surfaces. Staining, primarily iron oxide, extends considerably deeper in the rock but appears to be negligible below a depth of about 50 to 60 feet. As determined from core borings, the joint staining appears to be the most prominent on the steep right abutment. The rock in the flood plain is fresh except for the top 2 or 3 feet which is very slightly weathered.

- 2.7 Ground Water. Water occurs in the Jackfork formation in a free state and is replenished by surface runoff and streamflow filtering into the formation through open joints and fractures. The depth of the regional water table was not established. The water table is tributary to the river at both abutments of the main embankment. In the right embankment, artesian flow occurred in several grout holes between stations 2+50 and 3+80 at depths of 28 to 76 feet. In the main embankment, artesian flow was encountered between stations 18+90 and 20+25 at depths of 44 to 82 feet.
- 2.8 <u>Characteristics of Bedrock</u>. In addition to the bedrock characteristics discussed in the preceding paragraphs, tests were made on a 6-inch rock core taken in the spillway. On the basis of investigations before and during construction, the foundation rock is considered adequate to support the structure.

2.9 Investigations. Tulsa District drilled six site selection borings during January 1964. Later in 1964, and in 1965, and 1966, Fort Worth District completed an additional 67 dam site borings and excavated 14 floodplain test pits for design of the dam, spillway, and outlet works. Nineteen of these borings were drilled along the originally proposed dam axis, and eight were drilled on or adjacent to the design dam axis, including 4 offset borings for exploration of an unexpectedly deep bedrock interval in the floodplain near Boring 169. The initially proposed controlled spillway, located in the deep ravine right of the river, was explored with 12 borings. These borings were drilled and a trench was excavated to bedrock in the design spillway. Two of these borings were drilled inclined 35 degrees from horizontal and normal to bedding dip. Overburden was sampled with an auger and/or Denison core barrel. Where the overburden could not be sampled due to gravelly and cobbly material, the interval was penetrated with a rock bit. Test pits were dug to provide soil samples and to determine top of rock, where practicable. Hydraulic pressure tests were conducted in all NX-size borings, except boring 2C-20 and a few shallow holes along the axis of the outlet works. Electric logs were made in selected borings to aid in correlation of bedrock strata. Surface mapping was done to determine dip and strike of strata, joint patterns, rock type, and rock conditions in outcrops and along the shallow trench excavated in the design spillway. After 1966, 61 additional holes were drilled, making a total of 141 borings. The majority of the additional holes were drilled in the spillway-quarry area. The plans of borings for structures, spillway and borrow areas are given on Plates 10 and 12. Geologic sections with borings and water levels are shown on Plates 12 through 17. Logs of overburden borings and test pits are shown on Plates 5 through 11.

3. FOUNDATION AND ABUTMENT TREATMENT

3.1 Design Considerations.

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- 3.1.1 Embankment. All overburden and loose, weathered material were removed from the embankment areas, and a cutoff trench excavated to firm rock In a few instances, where deeper weathering was in the abutments. encountered, the excavation was planned to stop at 10 feet below the rock line, in order to remove all severely weathered material to a satisfactory The upstream slope of the cutoff trench coincided with the foundation. downstream dip of the rock. The downstream slope was planned as a 4 on 1 slope, but final slopes adjusted during construction to conform to other geologic structures such as jointing. The proposed alignment of the trench crossed various beds of sandstones and shales due to the divergence between the dam axis and strike of the rock. The cutoff trench alignment was adjusted during construction, as limits of the impervious zone permitted, to take advantage of the strike and dip of the rocks. A grout curtain was constructed along the entire bottom length of the cutoff trench by means of grout holes inclined upstream and drilled on a minimum of ten-foot centers. The degree of angle was controlled so as to intersect the downstream dipping bedding plane, It was believed this plane would control the greatest amount of grout movement and/or communication.
- 3.1.2 Spillway. The spillway contains generally the same bedrock distribution and conditions as the embankment foundation, as it is essentially on the strike of the formations. The excavation, though curved, is essentially across the strike, with the rocks dipping in a downstream, southerly, direction. The side slopes in the spillway were presplit 10V on 1H with two 10-foot berms. Since the spillway excavation cuts across the strike, the resulting side slope and dip of rock relation are favorable for stability. Due to the orientation of the rock, controlled blasting and machine trimming methods near final grade were required to produce a generally uniform surface. The curved alignments on the approach channel could not be excavated upstream from Hole 23 since the cut paralleled the strike. Downstream from Hole 23, the indicated alignment was followed and the 10-foot berm provided. Spillway details are shown on Plate 22.

3.1.3 Outlet Works.

A. Carrie

- (a) <u>Foundation Conditions</u>. The interbedded sandstone and shale provides an adequate foundation for the outlet works structure, whereas hard and competent sandstone underlie the gate tower. Unconfined compressive strengths for this sandstone were considered similar to those values for the same rock type at Gillham Dam which ranged from 2,500 psi for alternating shale and sandstone beds to 8,666 psi for a hard, fine-grained sandstone. The foundation grade for the intake structure, upper portion of the cut-and-cover conduit, and stilling basin were in unweathered rock (see Plates 1 and 2). The foundation grade at the axis of the dam and for about the last 200 feet of the downstream end of the conduit approximated the top of rock. Therefore, additional excavation of weathered zones was required to secure a suitable foundation in these zones.
- (b) <u>Water Table</u>. The water table along the centerline of the outlet works varied from about elevation 445 at the upstream and downstream ends to about elevation 470 at Station 18+60 (see Plates 5 through 9). Although pump tests were not made, ground water inflow was expected to be moderate, and conventional sump and pump methods were used.
- (c) Excavation. The excavation grade along the centerline of the outlet works is shown on Plate 1. The total depth of cut in the overburden and rock along the conduit will vary from about 20 to 32 feet. The maximum cut occurred in the stilling basin area where the foundation grade is 39 feet below the original ground surface. Overburden excavation depths varied from about 7 feet at the intake structure to about 34 feet near the center of the conduit section. Rock excavation depths varied from about 2 feet at the downstream end of the conduit to about 22 feet at the stilling basin end sill. Excavation below grade to firm rock and concrete backfill was performed where top of rock was encountered at a lower elevation. The excavation grade for the intake structure was about 17 feet below the top of rock. Overburden cut slopes of 1V on 3.5H, and rock cut slopes of 4V on 1H were selected.

Control blasting techniques, such as presplitting, smooth blasting, or cushion blasting were used to minimize overbreakage and shattering in the sandstone

and shale. The floor of the excavation broke out unevenly because of the steep dip and interbedded nature of the rock. Hand grading was utilized for the final cleanup and foundation preparation. From inspection of local outcrops it was determined that the shales slake slowly when exposed to air. Consequently, due to the low slaking potential of this compaction type clay shale, and the predominance of sandstone, foundation protection was not required. Permanent excavated slopes for the discharge channel were selected to be 1V on 3.5H through overburden and 2V on 1H through rock with a 5-foot bench at the top of rock.

3.2 Embankment Design.

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- 3.2.1 Main Embankment. A typical section of the main embankment is shown on Plates 1 and 28. The maximum height of the main embankment is 153 feet. The embankment was designed with a 1-foot overbuild to compensate for settlements during reservoir filling. The section is essentially symmetrical, with a central impervious core and with random fill zones flanked by transition material and outer shell zones of rock fill. A 4-foot-thick inclined filter is connected to a horizontal 6-foot thick select-rock drain at the base of the downstream shell. The embankment was designed to make maximum use of the materials from the required excavation of the spillway, supplemented where necessary by borrow materials.
- 3.2.2 <u>Right Embankment</u>. The typical section for the right embankment is shown on Plate 28. The maximum height of the embankment is 120 feet. The section consists of random fill zones, which were mainly a heterogeneous mix ure of all soils from the main embankment foundation excavation, with a centrally-located impervious core. A select rock-fill zone was placed on the upstream random fill to increase stability and to provide slope protection for the embankment. Internal drainage was provided by a 3-foot-thick inclined filter located on the downstream face of the impervious core which connects to a 4-1/2-foot horizontal filter blanket under the downstream random fill shell. The right embankment was designed with a 1-foot overbuild to compensate for settlements during reservoir filling.

4. CONSTRUCTION PROCEDURES

4.1 Embankment Foundation.

4.1.1 Main Embankment.

- (a) <u>Soils Description</u>. Profiles of the foundation soils are presented in the sections shown on Plates 5 through 11. These sections indicate that the main embankment overburden consisted of a surface layer of low plasticity silts, sands, and sandy clays; interspersed with small quantities of gravels, cobbles, and boulders. A heterogeneous mixture of gravels, cobbles, and boulders with a silty to clayey matrix underlies the upper sand and clays. The thickness of these deposits ranged from near zero on the abutments to a maximum of about 35 feet at boring 169 in the flood plain. The average depth of materials in the flood plain was about 20 feet.
- Treatment. Amis Construction Company of Oklahoma City, Oklahoma, the prime contractor, began clearing the embankment area on 20 August 1970. Beginning 20 September 1970, all overburden materials were removed from the foundation of the main embankment to provide a firm rock-to-rock contact for the rock-fill shell zones. Unsuitable material was hauled to the designated waste area or used to build haul roads. Suitable materials were stockpiled for later use as random fill or filter material or were incorporated into the first stage cofferdam. This excavation was accomplished with the use of 621 and 660 Caterpillar scrapers, 769 Caterpillar tail dumps, D-8 and D-9 Caterpillar dozers, and 988 and 980 Caterpillar loaders. Treatment of the foundation rock was limited to removal of all loose and highly weathered materials by blade cleaning outside the limits of the random fill and impervious core zones. foundation rock within the limits of the random fill and impervious core zones consisted of removing all dirt and other loose and highly weathered material by washing, air jet, or both. All open joints and fractures were cleaned and covered or filled with cement mortar or thick grout. The final excavated surface was protected by layers of unexcavated rock until just prior to final excavation, cleanup, treatment, and fill placement.
 - 4.1.2 Right Embankment. The depths of the foundation overburden

material under the right embankment were generally shallow, ranging from 1 to 4 feet, and were composed of forest litter underlaid by a heterogeneous residual soil (clay, sand, boulders, etc.). These overburden materials were excavated and wasted. Treatment of the foundation rock under the impervious core and random fill was the same as that for the main embankment impervious core and random fill zones. The foundation rock under the downstream random fill zone was blade cleaned.

4.2 Embankment Foundation Rock.

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4.2.1 General. The discussion of rock excavation in this report will be limited to the excavation within the the cutoff trench. excavation by drilling and blasting began on 2 December 1970 in the right embankment. Rock excavation was required from station 0+00 to 2+60 to a depth of 10 feet at centerline. The right abutment of the right embankment (station 0+00 to 2+60) sloped steeply upstream. Thus, to achieve a 10-foot cut along centerline of the cutoff trench, it was necessary to excavate as much as 25 feet of rock along the downstream face. See Photograph No. 1 for a general view of the trench excavation. The rock from station 2+60 to 3+80 was considered fresh and firm. Rock was excavated from station 3+80 to 6+60 to a depth of 6 to 10 feet at centerline. The left abutment of the right embankment (station 3+80 to 6+60) sloped very steeply downstream. This required a 15- to 20-foot cut in the upstream face to achieve the desired 6- to 10-foot cut at centerline. This also produced a very shallow cut on the downstream Except for several narrow sandstone ridges, the small mass of rock downstream of the presplit line was insufficient to contain the energy of the presplit shot. Consequently, there was almost no downstream face in the cutoff trench from station 3+80 to 6+60 (see Photograph No. 2). There was no cutoff trench from station 6+60 to 9+00. Rock was excavated from station 9+00 to 12+00 to a depth of 10 feet measured at centerline (see Photographs Nos. 4 and No cutoff trench was constructed from station 12+00 to 20+30. Rock was excavated from station 20+30 to 27+00 to a depth of 6 to 10 feet at centerline (see Photographs Nos. 12 through 18). All explosives used by Amis Construction Company at this project were manufactured by the Atlas Powder Company.

4.2.2 <u>Presplitting</u>. To determine the best method of presplitting the downstream face of the cutoff trench, a test section was designated from

station 21+00 to 24+00. Within this area, from station 21+75 to 22+00, the contractor elected to use Kleen-Kut and E-Cord to compare the results with those obtained by the loading procedure outlined in the contract specifications. Holes 2-1/2 inches in diameter were drilled on 24-inch centers to a depth of 6 feet. The holes from station 21+75 to 22+00 were each loaded with one stick of Kleen-Kut 17 percent (1-1/8-inch by 36-inch) with a cap and E-Cord downline. The holes in the control sections were each loaded with one stick of Giant Gelatin 40 percent (1-1/4-inch by 8-inch) in the bottom and two one-half sticks of Giant Gelatin hung on E-Cord on 12-inch centers above it. All holes were stemmed in the top 3 feet. Results of the test showed no discernable difference in the face produced; therefore, the Contractor continued to use Kleen-Kut because of the ease of loading. Typically, the holes were drilled on 24-inch centers and loaded with a continuous column of Kleen-Kut to within 3 feet of the collar. The remaining 3 feet was stemmed.

Primary or excavation shot holes were 3-1/2 inches in diameter. The hole spacing and loads varied with the lift thickness and competence of the rock. Holes adjacent to the presplit line were loaded with one stick of Power Primer 75 percent (2 inches by 8 inches) in the bottom followed by one or more sticks of Ammodyte 57 percent (2-1/2 inches by 16 inches). The amount of stemming in holes less than 8 feet deep ranged from 2 feet to 4 feet. In the deeper holes the depth of stemming approximately equaled the shorter dimension of the hole spacing. Generally, the shallower holes were spaced 4 feet by 6 feet with the spacing increasing with hole depth to a maximum of 7 feet by 11 feet. The holes toward the center of a shot were usually loaded with one stick of Power Primer in the bottom followed by ammonium nitrate blasting agent in the form of prills. These holes were stemmed in the same manner as the holes loaded with Ammodyte. Total quantities of explosives used for excavation of the cutoff trench are shown in Table 4.2.2.

TABLE 4.2.2
EXPLOSIVES SUMMARY

Trade Name	:	Quantity
Kleen-Kut (1-1/8 inches by 36 inches) 17 percent Giant Gelatin (1-1/4 inches by 8 inches) 40 percent Power Primer (2 inches by 8 inches) 75 percent Ammodyte (2-1/2 inches by 16 inches) 57 percent ANFO (ammonium nitrate prills) Primacord E-Cord	:	1,028 pounds 223 pounds 1,789 pounds 3,731 pounds 9,475 pounds 2,112 feet 7,701 feet

4.3 Foundation Preparation.

- 4.3.1 General. All foundation areas to receive concrete, impervious fill, or random fill required foundation pr paration. Subsequent change orders widened the foundation preparation under random fill zones, and in August 1972 the foundation preparation under random zones was limited to the 20 feet immediately upstream and 15 feet immediately downstream of the impervious zone. In those areas where the top of rock was considered to be top of firm rock, foundation preparation directly followed overburden excavation. excavation was necessary, it was carried to within 1 foot of final grade. The final foundation was achieved by removing the large, loose, weathered rock with a Poclain backhoe and a Caterpillar 966 rubber- tired loader. Then a labor crew removed smaller rock with picks, shovels, and pry bars. Final cleanup was achieved by using compressed air and water jets to blow off the remaining loose material. This final cleanup was timed to immediately precede the placement of concrete or earth fill. Generally, placement followed cleanup by less than 1 The foundation was kept wet after the completion of cleanup until the start of backfilling operations.
- 4.3.2 <u>Foundation Protection Concrete</u>. Several areas in the foundation required the application of foundation protection concrete. Where open joints or badly weathered thin shale seams occurred, they were cleaned to a depth twice as great as their width. They were then filled with 3/4-inch aggregate concrete and allowed to set before covering with fill. In several areas, large overhangs existed in the downstream

portion of the natural cutoff trench created by the weathering of a shale bed between two more resistant sandstone beds. In one area the overhang was backfilled with foundation protection concrete by forming the open side and filling with high slump 3/4-inch concrete. In another area the overhang was eliminated by ripping and dozing. The overhanging rock was sufficiently jointed to allow its removal in this manner without disturbing the underlying rock.

4.4 Grout Curtain.

4.4.1 <u>Drilling and Grouting</u>. The drilling and grouting was performed by the Judy Company, a subcontractor. The Judy Company began moving equipment to the jobsite on 16 December 1970. Drilling and grouting began on 1 March 1971 and continued to 15 August 1971. Following diversion of the river through the outlet works, drilling and grouting began again on 1 June 1972 and was completed on 29 August 1972. Equipment used is listed in Table 4.4.1. Photograph No. 3 shows grouting operations.

The grout curtain was formed by drilling and groucing a single line of holes, varying in depth from 50 to 100 feet, along the dam axis from station 0+00 to 27+00 (see Plates 39 through 42). Generally, the holes were angled 30° upstream to intersect the maximum number of bedding planes, which were dipping downstream. The primary holes (20-foot centers) were drilled to full depth, washed, pressure tested, and grouted in accordance with the "Stop Grouting, Split Spacing" method. Briefly, this method requires the holes to be drilled to final depth and grouted by stops through a packer set at successively shallower depths. The secondary holes were then located midway between the previously drilled and grouted holes. This split spacing continued until the holes were determined to be tight. Generally, a hole was considered tight if it took less than 0.1 sacks of cement per foot.

TABLE 4.4.1

GROUTING EQUIPMENT

Item		Quantity
	:	
Chicago-Pneumatic 600 Air Compressor	:	1
Gardner-Denver 900 Air Compressor	:	1
Chicago-Pneumatic Airtrac Drill	:	1
Gardner-Denver Airtrac Drill	:	1
Atlas-Copco Airtrac Drill	:	1
Gardner-Denver Pump, 5'x 3' - 1/2' x 4'	:	.1
Wilden Air Pump, 2-inch	:	1
Deming Air Pump, 3-inch	:	1
Trucks, 2-ton	:	2
Pickups, 1/2-ton	:	2
Parts Trailer	:	1
Assorted Valves, Gages, and Hoses	:	
, ,	:	

4.4.2 <u>Difficulties Encountered</u>. There were three areas of the grout curtain which required special treatment.

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- (a) Area No. 1. The first area occurred between stations 16+75 and 19+00 (see Photograph No. 21 for excavation difficulties). From about station 20+10 to 18+70 the dam axis falls between two of the resistant sandstone ridges which strike almost parallel to the axis. At station 18+70 the axis intersects the toe of the steeply dipping sandstone bed which forms the upstream face of this natural cutoff trench. The dam axis falls on this dip slope face from station 18+70 to 17+25, at which point the top of the dip slope is reached (see Photographs Nos. 23 through 26). It was not practical to drill grout holes in this face; therefore, an overlap section was created. This was done by moving the grout curtain gradually upstream from a point on the centerline at station 16+80 to a point 12 feet upstream at station 17+80. Likewise, the grout curtain was moved gradually downstream from a point on the centerline at station 19+00 to a point 20 feet downstream at station 16+75. This, in effect, created two lines of grouted holes approximately 20 feet apart from station 16+75 to station 17+80 (see Plate 41).
- (b) Area No. 2. The second problem occurred between stations 12+30 and 12+80. The slope of the right abutment presented a safety hazard to the drilling operation. The subcontractor proposed to drill and grout this

area with two series of angle holes drilled from stations 12+30 and 12+80. The subcontractor agreed to drill two holes at angles greater than the maximum required by the specifications to assure complete grouting of this area. The proposal was accepted, and seven angle holes, ranging from 10° to 45° and 75 feet to 100 feet, were drilled and grouted (see Plate 40).

- (c) Area No. 3. The third area requiring special treatment occurred between stations 6+60 and 9+00. The plans did not require a grout curtain across the hill between the right embankment and the main embankment. However, on the recommendation of the Foundations and Materials Section, Southwestern Division, five holes were drilled on 40-foot centers from stations 6+60 and 9+00. All these holes were tight and no further grouting was deemed necessary in this area.
- 4.4.3 Conclusions on Grouting Effectiveness. The entire length of the grout curtain for its full depth was pressure grouted to refusal, with a very small quantity of grout lost through surface leaks. In areas where seeps occurred in the foundation downstream of the centerline or artesian flow was encountered during drilling, there was a noticeable relocation of seeps to areas upstream of the grout curtain. This indicates the grout curtain has intercepted ground-water flow. Subsequent high water retained by the cofferdam produced no unusual increase in seepage downstream. These facts indicate that the grout curtain is performing its design function.

4.5 Material Sources

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4.5.1 <u>Borrow Materials</u>. Borrow soils for the embankment were obtained from borrow area E located upstream of the axis and from areas located within the limits of the conservation pool and outside of borrow area E, which was a value-engineered proposal to the contract. Borrow area D was used as a source of granular materials for the filter. Borrow area C, which was to be a source of impervious borrow, but was not used. The investigation program and plan of the borrow areas are shown on Plates 10 and 11. Characteristics of soils in borrow area E are contained in the following tabulation:

Soil	No. Samples	Average Value			
Type	Tested		Percent Fines		
CL	8	31	60		
ML-CL	3	19	60		
ML	4	NP	61		
SC	24	27	36		
SM, SP	44	NP	33		
CH	1	65	88		
GC	18	32	17		
GM, GP	3	NP	22		

- 4.5.2 <u>Filter Materials</u>. Materials for the filter A were obtained from selective excavation of the main embankment foundation and borrow area D. The materials were then processed to the extent necessary to meet the specifications.
- 4.5.3 Rockfill and Transition Materials. The rockfill material was obtained from the required spillway and approach channel excavations. Plate 17 defines the geologic quality of the in situ material. The select rock fill was obtained from the better quality, least weathered material and was processed over a 3-inch grizzly to insure drainage of the mass and to remove poor quality materials. The minus-3-inch byproduct of the select rock was used for transition. The random rock fill material came from any of the zones except the most weathered. The original contract documents did not require processing of the random rock fill materials; however, the quality of the excavated material was so poor that the processing over a 3-inch grizzly was required. The minus-3-inch byproduct was then used in the random fill zone of the embankment.

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5. CONSTRUCTION CONTROL DATA

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- 5.1 <u>Construction Sequence</u>. The general construction sequence was as outlined below:
- (a) Excavated Stage 1 diversion channel and constructed first stage cofferdams. (See Plate 27 for details of first and second stage diversions.)
- (b) Constructed outlet works and embankment within first stage cofferdams to the minimum elevations and sections shown on Plate 26.
- (c) Completed right embankment except upstream transition and select rock sections prior to beginning second stage construction.
- (d) Removed first stage cofferdam and constructed second stage diversion cofferdam.
 - (e) Constructed second stage cofferdam.
 - (f) Constructed remainder of main and right embankments.
- 5.2 <u>Impervious Fill</u>. The specifications required that the impervious material consist of clays having a liquid limit of 25 or greater and a percent fines of not less than 35. In addition, the materials were to be placed at 95 percent or greater of standard density as determined by the Rapid Method of Compaction Control (Water Resources Technical Publication EM No. 26). The moisture content was to be within minus-one and plus-three percentage points of optimum. A summary of control tests performed on the impervious fill materials placed in the embankment and reported in Monthly Summaries in Appendix F is given below.

<u>Parameter</u>	No. Tests	Average	High	Low
% Comp.	203	99.5	106	94
Wf-Wo (%)	203	+ 0.3	+ 3.3	- 1.4
-200 Sieve (%)	46	49	77	21
LL (%)	43	34	64	25

Results of impervious fill density tests are plotted on Plates 18 through 21.

5.3 <u>Random Fill</u>. The specifications required a moisture content of plus or minus three percentage points of optimum and 95 percent of standard density.

<u>Parameter</u>	No. Tests	Average	<u>High</u>	Low
% Comp.	271	96.6	110	94
Wf-Wo (%)	271	- 0.5	+ 3.1	- 3.4

Results of the above tests indicate the design requirements were achieved. The random fill density test results are shown plotted on Plates 17 through 21.

5.4 Design Modifications.

5.4.1 <u>Typical Section</u>. The typical section as shown on Plate 28 is the final section resulting after several modifications were made. The sequence of modifications to the main embankment section is shown on Plate 29.

The original section had an upstream random fill zone 20-feet wide and a downstream random fill zone only 15-feet wide; however, the quality of the rock from the required excavation was so poor that the rockfill zones had to be reduced and the screenings from the random rockfill were absorbed in the enlarged random fill zones.

- 5.4.2 <u>Filter</u>. The specifications did not require a washing sequence in the production of the filter materials. After some material was produced, it became evident that the material would not function as a high capacity drain. A change order was initiated in which the contractor blended 60 percent of the material from the foundation with 40 percent from borrow area D to reduce the minus-200 material. The horizontal filter thickness below elevation 490.0 in the right embankment was increased from 3 feet to 4.5 feet, and above 490.0 the thickness was reduced to 2 feet. This change was made to increase the capacity of the drain placed prior to the change order.
- 5.4.3 <u>Select Rockfill</u>. The spillway was excavated by systematic drilling and blasting commencing in February 1971 and completed in May 1974. The required excavation in the spillway was the primary source of rockfill material for the embankment. The fresh, hard, unweathered sandstone was processed to produce select rockfill by removing all minus 3-inch material. The minus 3-inch material was placed in the transition zone. Softer, slightly weathered sandstone meeting specifications for random rockfill was used as quarry run. The portion of the remaining spillway excavation meeting

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specifications for random rockfill was used as quarry run. The portion of the remaining spillway excavation meeting specifications was used as random fill. Materials unsuitable for zone placement were wasted or used to construct haul roads.

5.4.4 <u>Placement Quantities</u>. The original design contained 1,890,000 cubic yards of compacted random rockfill, select rockfill, and transition material. As a result of the embankment zoning revisions, the actual quantities placed were as follows:

Select rockfill	328,059 cubic yards
Random rockfill	900,564 cubic yards
Transition material	223,757 cubic yards -
Total	1,452,380 cubic yards

5.4.5 <u>Seepage During Construction</u>. As shown on Plates 1 and 27, a random rock fill cofferdam was incorporated into the upstream toe of the main embankment. During construction, the pool rose against the cofferdam and seepage occurred through the cofferdam and emerged downstream across a haul ramp. This cofferdam was part of the second stage diversion as shown on plate 27. As a result of the observed seepage, a modification was made to the main embankment section which consisted of placing a five ft. thick blanket of impervious material on the cofferdam upstream slope.

5.5 Embankment Stability.

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5.5.1 Tests and Design Values. Tests were conducted on representative soils in the remolded state. The summary of soil shear strength data and graphical presentations are contained in Appendices B through G. Test results from test fills at Gillham Dam and Dierks Dam were used in determining the unit weights and design shear strengths for the rock fills. The adopted design strengths are shown in the following tabulation:

Adopted Design Data

		Design Strengths					
Saturate	d Submerged		Q		R	S	
Unit Weig	ht Unit Weigh	t Ø	С	Ø	C	Ø	C
Materials (pcf)	(pcf)	(degrees	s)(tsf)	(degrees)(tsf)	(degrees)	(tsf)
Coloot wool (1) 1	25 70 6	20	^	20	^	39	0
	35 72.6	39	٥.	39	Ŏ	• •	Ŏ
	35 72.6	25	0.4	33	O	33	Ū
Impervious 1	25 62.5	10	0.6	14	0.1	27	0
	25 62.5	12	0.8	15	0.3	30	0
	25 62.5	12	0.8	15	0.3	30	0
Filter 1	<u>25 62.5</u>	33	00	33	0	33	0

 [#] Angle of internal friction. C = cohesion.
 Material assumed to have 40 percent voids and a saturated surface dry unit weight of 105 pcf.

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- (2) Design shear strengths for the random rock fill zones were based on test results for the minus No. 4 fraction of rock.
- (3) Design shear strengths for the random fill zones were based on the assumption that these zones would be entirely shale material. These zones actually contain large percentages of sandstone fines and plus 3/4-inch material. Therefore, the design values are conservative.
- 5.5.2 Construction Control Record Shear Tests. The following summary contains the results of tests performed as construction control samples from the impervious and random fill zones, with six samples obtained from each zone.

Shear tests on the record samples reflected strengths higher than design values with the exception of the unconsolidated-undrained, Q, strengths which were lower. The consolidated-undrained, R, tests on the random fill record samples have not been completed since testing apparatus of sufficient size to test material containing gravel was not available.

Construction Control Shear Tests

		<u>Rardom Material</u>					
		<u>Test St</u>	Test Strength		trength		
No.	Type	ø	C	Ø	C		
Tests	<u>Test</u>	Degrees	t.s.f.	Degrees	t.s.f.		
6	Q	6 *	0.4	12	0.8		
6	S	30	0	30	0		
		Impervious Material					
		<u>Test St</u>	Test Strength Desi				
No.	Type	Ø	C	Ø	C		
<u>Tests</u>	Test	Degrees	t.s.f.	Degrees	t.s.f.		
6	Q	6	0.6	10	0.6		
5	R	17	0.5	14 .	0.1		
5	S	31	0	27	0		

*The random materials included minus-3-inch byproduct of the random rock and were less plastic than originally designed. (Ref: Periodic Inspection Report No. 1. November, 1974).

5.5.3 <u>Procedures</u>. The embankments were analyzed for stability using the circle arc and wedge methods under conditions of end of construction, partial pool, sudden drawdown, and steady seepage. The design analyses are presented on Plates 30 through 33 for the right embankment and Plates 34 through 37 for the main embankment. The "Adopted" factors of safety for the

main embankment, per Plates 34 through 37, were computed using the adopted design shear strengths and the revised embankment configuration. This reanalysis refined the original slope stability calculations in view of shear strength values developed from the Gillham and Dierks Dam test fills. The stability of the right embankment was not reanalyzed because the design factors of safety were generally higher than those for the main embankment. In addition to the above circle arc and wedge analyses, the main embankment was analyzed for the end of construction and steady seepage conditions by a procedure outlined in an ASCE paper titled, "Rockfill Design - Carters Dam," by Robeson and Crisp, September 1966. This procedure, which basically utilizes earth pressure theories, was believed to be more applicable to a narrow core rockfill structure than those contained in EM 1110-2-1902. Results of these analyses indicate the embankment will be stable for all design conditions.

5.5.4 Results. The following tabulations indicate the safety factors obtained for the most critical arcs.

Factors of Safety for Right Embankment Stability Analysis

	Shear Strength	Type of	Safety Factor	
Condition Analyzed	Used	Analysis	Design	Adopted
End of const., downstream	(1) Q	Arc	1.36	N/A
Partial pool	· R	Arc	1.51	N/A
Sudden drawdown (2)	R	Arc	1.40	N/A
Steady seepage (3)	R	Arc	1.41	N/A
	S	Arc	1.89	N/A

- (1) Results are for downstream slope which is more critical than the upstream slope.
- (2) Analysis for drawdown from maximum pool elevation, 587.9, to conservation pool elevation 526.0. The select rock zone was considered to be free draining.
- (3) Analysis based on differential head between maximum water surface elevation 587.9, and maximum tailwater elevation 451.0.

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Factors of Safety for Main Embankment Stability Analysis

Condition Analyzed	Type of Shear Strength	Type of Analysis	Safety <u>Design</u>	Factor Adopted
End of Construction U.S.	Q	Arc	1.71	1.51
Partial Pool	S,(R+S)/2	Arc	1.40	1.77
Sudden Drawdown	R	Arc	1.03	1.12
Steady Seepage	S,(R+S)/2	Arc	1.37	1.63
End of Construction D.S.	Q	Arc	1.58	1.32

- (1) Results are for the downstream slope which is more critical than the upstream slope.
- (2) Analysis for drawdown from maximum pool elevation 587.9, to conservation pool elevation 526.0. The select rock zone was considered to be free-draining.
- (3) Analysis based on differential head between maximum pool elevation 587.9, and maximum tailwater elevation 457.9.
- 5.5.5 <u>Conclusion</u>. Based on a comparison of test results of the in-place materials with those used in design, the stability of the structure is considered adequate.

6. INSTRUMENTATION

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- 6.1 <u>Surface Control Monuments</u>. A total of thirty-two (32) embankment surface monuments and seventeen (17) abutment control monuments were established to monitor the embankment movements along eight lines of monuments. The locations of the monuments are shown on Plate 4. Results of alignment and settlement surveys are first reported in the second periodic inspection report. Movements observed were less than .05 feet and were well within the accuracy of the survey techniques used. Subsequent readings through the sixth periodic inspection report (March 1985) indicated decreasing amounts of annual settlement, and insignificant amounts of horizontal movement.
- 6.2 Conduit Alignment Plugs. A total of sixty-four (64) plugs were installed in the conduit invert, as shown on Plate 24. Damage to several of the plugs occurred during construction and necessitated repair and reestablishment of the initial survey in May 1975. A crack was noted at the crown of the conduit during the second periodic inspection. Monolith joint 13-14 had opened 1/2 inch, but the waterstop was still totally effective. Later periodic inspections indicated minor settlements only and generally satisfactory condition of the conduit. Conduit alignment has not been run since the initial alignment of May 1975 because the gates at the upstream end of the conduit do not seal tight enough to decrease flow of water through the conduit enough to take measurements on the plugs located on both sides of each monolith in the conduit invert.
- 6.3 <u>Bridge Alignment Plugs</u>. Thirteen (13) alignment plugs were installed on the gate tower and service bridge in June 1976, as shown on Plate 24. The maximum settlement recorded by the sixth periodic inspection was approximately .08 feet at the end of the bridge farthest from the gate tower. The maximum deviation horizontally from a line of sight between centers of abutments is approximately 0.15 inch. Movements of the service bridge are not considered significant.
- 6.4 <u>Preumatic Pore Pressure Cells</u>. Fourteen (14) nitrogen-gas-operated pore-pressure transducers were installed in the embankment; however, the instrumentation malfunctioned and is not operating. Since these were installed for the purpose of determining pore pressure buildup in the impervious core

during construction, the loss of the cells does not affect the monitoring or the safety of the structure.

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- 6.5 <u>Carlson Stress Cells</u>. A total of twenty-six (26) Carlson stress cells were installed on or along the conduit as shown on Plate 43. Cell No. 23 did not function at the second periodic inspection. In addition, the readings and resulting analysis of stress conditions strongly suggested that some of the cell data was misunderstood or unreliable. By 1977 all Carlson stress cells had ceased to function.
- 6:6 Seepage. The quantity of controlled seepage in the main embankment increased from the first periodic inspection to the second. Seepage along the stilling basin walls reported in the first report was stopped by plugging selected drain holes in the upper part of the drop section. An appreciable flow of clear water was noted coming from the right side toe drain. Observations for seepage through the sixth periodic inspection reported no apparent problems from the seepage, though the clear water discharge from the right toe drain continues. Some intermittent wet areas have been noted on the downstream face coincident with a construction phase haul ramp. Significant seepage has not been reported.

Observations of the right embankment continue to note flow in the paved ditches along both abutments and at isolated locations near the base of the downstream slope. The areas are for the most part seasonal and are considered to be due primarily to rainfall infiltration. However, a small area of perennial flow does exist in the right abutment paved ditch.

In summary, periodic inspections have indicated that seepage flow rates are negligible and reflect a non-changing condition. Continued monitoring is recommended.

6.7 <u>Inspection Schedule</u>. The schedule for monitoring this instrumentation is contained in the "Operation and Maintenance Manual" dated January 1987.

APPENDIX A

PHOTOGRAPHS

DIERKS DAM AND RESERVOIR SALINE RIVER, ARKANSAS

EMBANKMENT CRITERIA AND PERFORMANCE REPORT

APPENDIX A - PHOTOGRAPHS

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A-1	Photo. No. 2. Cut-off trench in rt. embankment from sta. 4+00 to 6+00. Note absence of a d/s face. 14 Jun 71
A-2	Photo. No. 3. Grout pump used on this contract. Mixing tank is being charged. The mixed grout is emptied into the holding tank to be pumped into the grout holes.
A~2	Photo. No. 4. Cut-off trench in the main embankment from sta. 9+00 to 11+50. Grout lines are being layed in the foreground. 10 Sep 71
A-3	Photo. No. 5. C. O. T. from sta. $12+40$ to $11+00$ u/s face is excavated on the dip slope. 26 Jan 73
A-3	Photo. No. 6. Cut-off trench from sta. 19+50 to 23+00. The deep hole in foreground occurred as a result of stream erosion. 27 Sep 71
A-4	Photo. No. 7. D/s face of cut-off trench from sta. 19+50 to 19+70. Face is very irregular as a result of differential erosion. 4 Oct 71
A-4	Photo. No. 8. Overall view of d/s face of C. 0. T. from sta. $19+50$ to $19+70$. 4 Oct 71
A-5	Photo. No. 9. U/s (dip slope) face of C. O. T. from sta. 19+00 to 19+25. 4 Oct 71
A-5	Photo. No. 10. View along center line of C. O. T. from sta. 19+00 to 19+25. Note alluvial fill behind skip box. 14 Oct 71
A-6	Photo. No. 11. Overhang in d/s face of C. O. T. at sta. 19+00 being cleaned prior to backfilling with concrete. 21 Oct 71
A-6	Photo. No. 12. Typical view of the cut-off trench from sta. 20+30 to 21+50. Note dip slope face u/s and absence of d/s face. 7 Oct 71
<u>A</u> -7	Photo. No. 13. D/s portion of the C. O. T. from sta. 21+25 to 22+00. Note presplit holes in the remaining portion of the d/s face. 13 Nov 71
A-7	Photo. No. 14. C. O. T. from sta. 21+00 to 23+00. Note grout hole

APPENDIX A - PHOTOGRAPHS (continued)

Page No.	Photo. No. and Description
A~8	Photo. No. 15. C. O. T. from sta. 22+50 to 23+50. A deeper cut at center line results in a better defined d/s face. Note grout hole casings along centerline. 26 Jan 72
A-8	Photo. No. 16. Typical view of C. O. T. from sta. 23+00 to 24+50. 3 Feb 72
A-9	Photo. No. 17. Typical view of C. O. T. from sta. 24+50 to 27+00. 10 May 72
A-9	Photo. No. 18. Dozer excavating shot rock from cut-off trench in vicinity of sta. 26+00. 12 May 71
A-10	Photo. No. 19. Clean-up of foundation along center line, sta. 18+00 to 14+50. Note dozer working in weathered shale seam on the left. 13 Apr 72
A-10	Photo. No. 20. Excavation of overburden and foundation clean-up for impervious and downstream random zones. Note depth of weathered shale seam. 13 Apr 72
A-11	Photo. No. 21. Clean-up of foundation for upstream random fill zone near sta. 17+50 to 18+50. Note overhand in downstream face. 17 Apr 72
A-11	Photo. No. 22. D-9 Ripper removing rock overhang under upstream random fill zone near sta. 17+50. 19 Apr 72
A-12	Photo. No. 23. Overall view of main embankment C. O. T. Foreground showns C. O. T. from sta. 9+50 to 11+00 right center of photo shows the natural C. O. T. 26 May 72
A-12	Photo. No. 24. Foundation along center line from sta. 16+00 to 18+00. The upper line of grout holes in the overlap section occurs where the men are standing. 4 Aug 72
A-13	Photo. No. 25. Natural C. O. T. from sta. 18+00 to 19+00. Note overhang to be removed in d/s face also the lower line of grout holes in the overlap section occurs in this area. 27 Jul 72
A-13	Photo. No. 26. Overall view of the center line form sta. 15+00 to 18+50. Center line in the foreground falls in the water in the lower left corner. Centerline in the background fall under the dozer. 4 Aug 72
A-14	Photo. No. 27. Damsite, looking toward left abutment. Dec 70
A-15	Photo. No. 28. Construction of outlet works, looking downstream. Jul 71
A-16	Photo. No. 29. Construction of outlet works and embankment, looking downstream. Mar 72
A-17	Photo. No. 30. Main embankment construction, looking toward left abutment. Jul 72

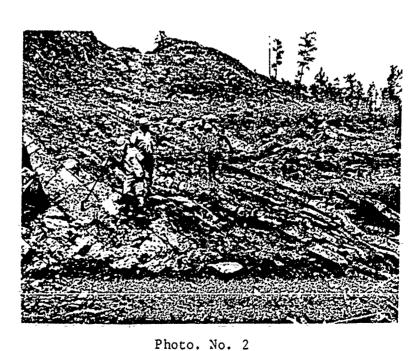
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APPENDIX A - PHOTOGRAPHS (continued)

Page No.	Photo. No. and Description
A-18	Photo. No. 31. Conduit being used for diversion while work progresses on main embankment. Spillway excavation and right embankment in background.
A-19	Photo. No. 32. Heavy rains caused flooding of construction site. Lake elevation 512+/ Apr 73
A-20	Photo. No. 23. Downstream view of gate tower construction. Oct 73
A-21	Photo. No. 34 Main embankment and outlet works essentially complete. Work progressing on service bridge. Apr 74



Photo. No. 1
Cut-off trench in the rt. embankment from sta 0+00 to 5+00.
Skid-mounted grout pump is located in the ravine between the truck and the compressor. Note excavation of the u/s face on the dip slope.



6-14-71 Cut-off tranch in rt. embankment from sta. 4+00 to 6+00. Note absence of a d/s face.

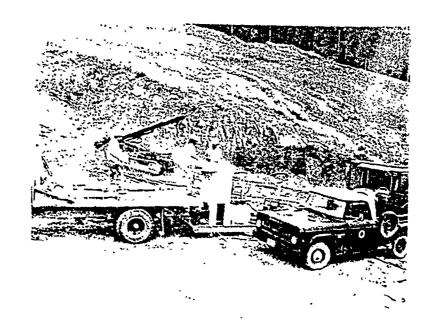
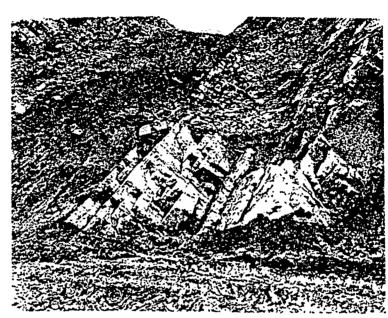


Photo. No. 3

Grout pump used on this contract. Mixing tank is being charged. The mixed grout is emptied into the holding tank to be pumped into the grout holes.



9-10-71 Photo. No. 4 Cut-off trench in the main embankment from sta 9+00 to 11+50. Grout lines are being layed in the foreground.



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Photo. No. 5

1-26-73 C. O. T. from sta 12+40 to 11+00 u/s face is excavated on the dip slope.

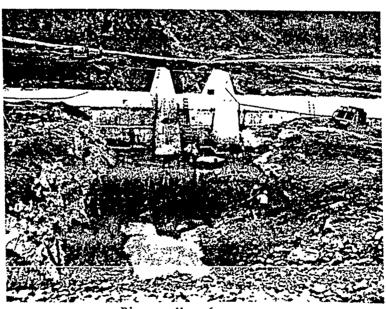


Photo. No. 6

9-27-71 Cut-off trench from sta 19+50 to 23+00. The deep hole in foreground occurred as a result of stream erosion.

Photo. No. 7

10-4-71 D/s face of cut-off trench from sta 19+50 to 19+70. Face is very irregular as a result of differential erosion.

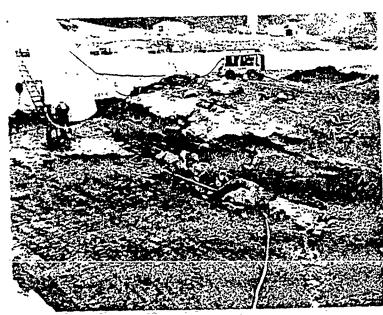


Photo. No. 8

10-4-71 Overall view of d/s face of C.O.T. from sta 19+50 to 19+70.

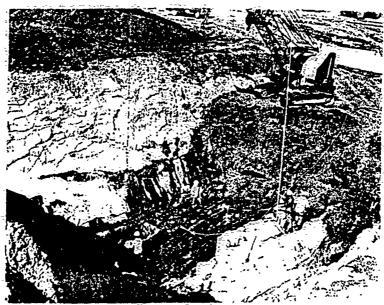


Photo No. 9

10-4-71 U/s (dip slope) face of C.O.T. from sta 19+00 to 19+25.

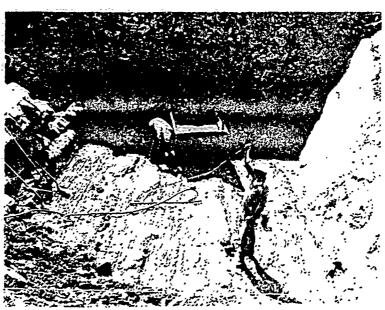


Photo. No. 10

10-34-71 View along center line of C.O.T. from sta 19+00 to 19:25. Note alluvial fill behind skip box.



Photo. No. 11

10-21-71 Overhang in d/s face of C.O.T. at sta 19+00 being cleaned prior to backfilling with concrete.

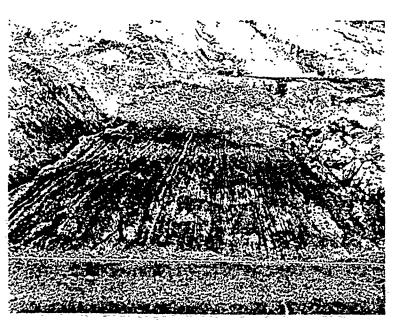


Photo. No. 12 10-7-71 Typical view of the cut-off trench from sta 20+30 to 21+50. Note dip slope face u/s and absence of d/s face.

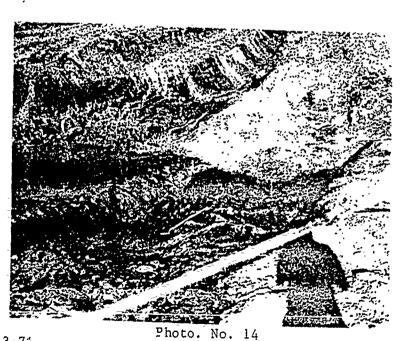
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Photo. No. 13

11-13-71 D/s portion of the C.O.T. from sta 21+25 to 22+00. Note presplit holes in the remaining portion of the d/s face.

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11-13-71
C.O.T. from sta 21+00 to 23+00. Note grout hole casings along center line.

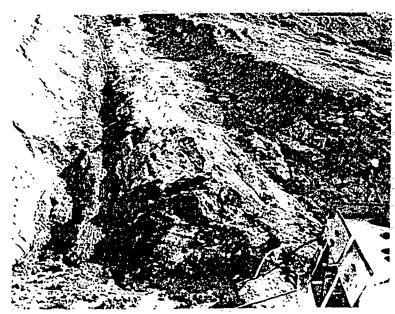


Photo. No. 15

1-26-72 C.O.T. from sta 22+50 to 23+50. A deeper cut at center line results in a better defined d/s face. Note grout hole casings along centerline.



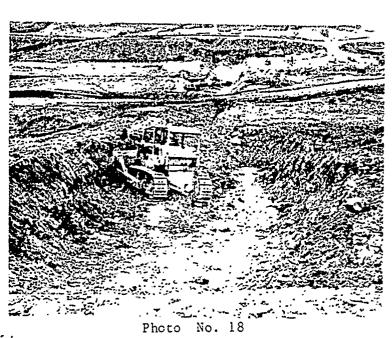
Photo. No. 16

2-3-72 Typical view of C.O.T. from sta 23+00 to 24+50.

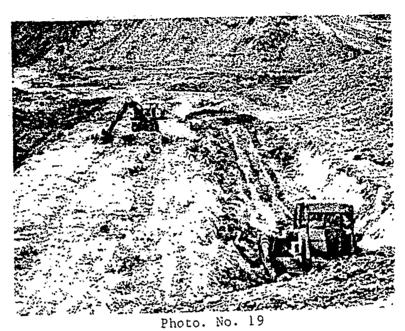


5-10-72

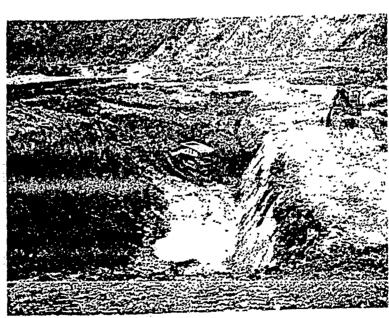
Photo. No. 17 Typical view of C.O.T. from sta 24+50 to 27+00.



5-12-71 Dozer excavating shot rock from cut-off trench in vicinity of sta 26+00.



4-13-72 Clean-up of foundation along center line, sta 18+00 to 14+50. Note dever working in weathered shale seam on the left.



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Photo. No. 20
Excavation of overburden and foundation clean-up for impervious and downstream random zones. Note depth of weathered shale seam.

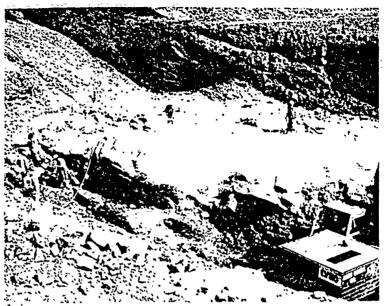
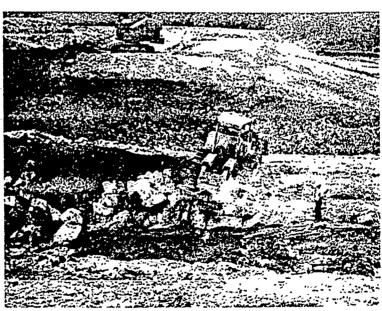


Photo- No 21

4-17-72 Clean-up of foundation for upstream random fill zone, near sta 17+50 to 18+50. Note overhang in downstream face.



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Photo. No. 22

4-19-72 D-9 Ripper removing rock overhang under upstream random fill zone near sta 17+50.



5-26-72 Overall view of main embankment C.O.T. Foreground shows C.O.T. from sta 9+50 to 11+00 right center of photo shows the natural C.O.T.



Foundation along center line from sta 16+00 to 18+00. The upper line of grout holes in the overlap section occurs where the men are standing.



Photo. No. 25

7-27-72 Natural C.O.T. from sta 18+00 to 19+00. Note overhang to be removed in d/s face also the lower line of grout holes in the overlap section occurs in this area.

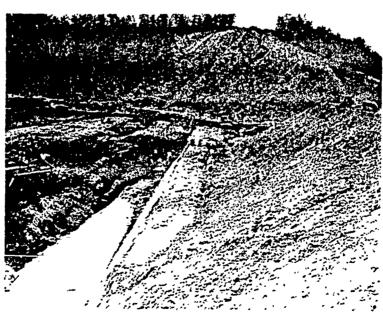
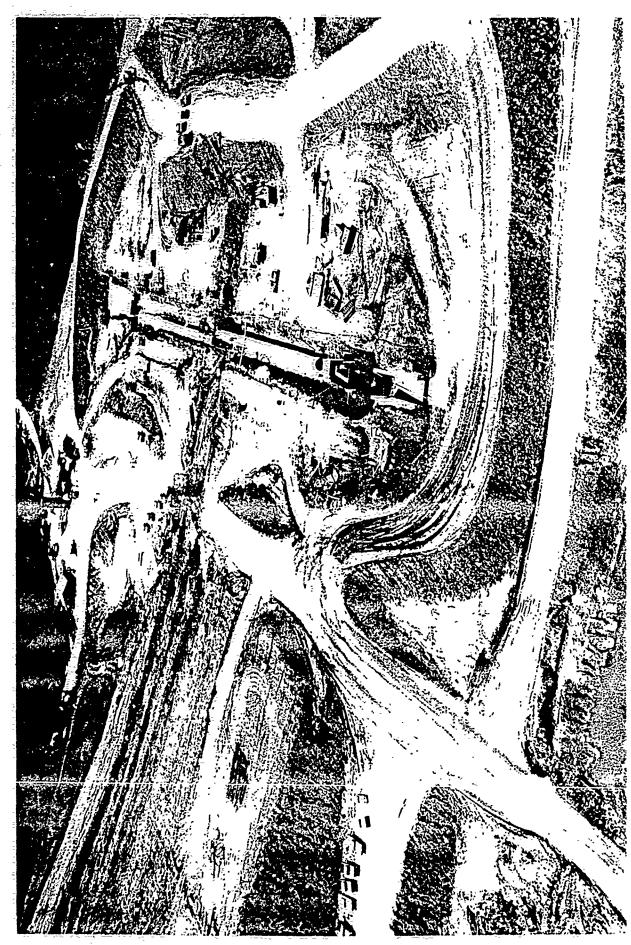


Photo. No. 26

0verall view of the center line from sta 15+00 to 18+50. Center line in the foreground falls in the water in the lower left corner. Centerline in the background fall under the dozer.

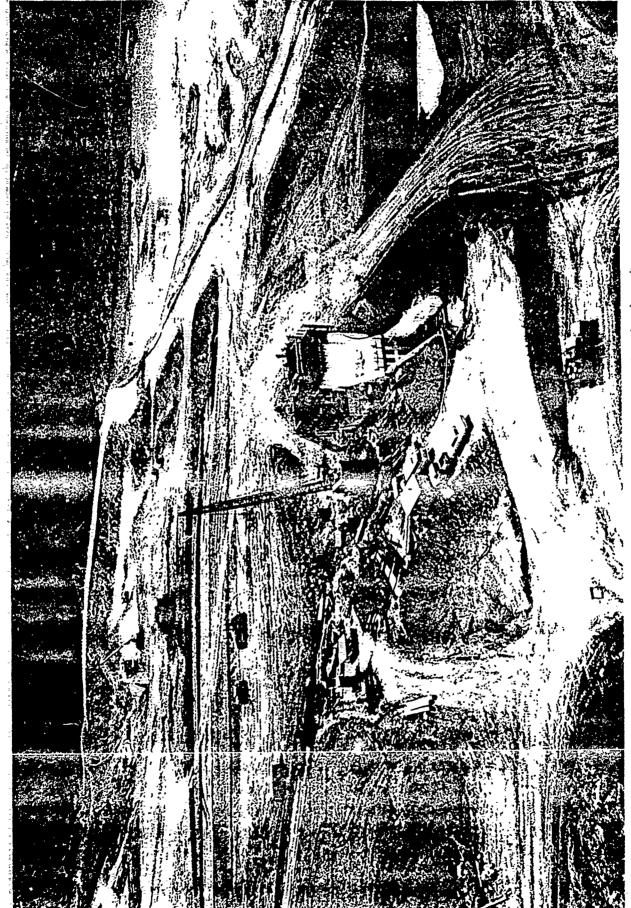


Photograph No. 27



Photograph No. 28

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DIERKS DAM - March 1972 - Construction of outlet works and embankment, looking downstream.



DIFRKS DAM - July 1972 - Main embankment construction, looking toward left abutment.

Photograph No. 30

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progresses on main embankment. Spillway excavation and right embankment in background. - Conduit being used for diversion while work DIERKS DAM - October 1972



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DIERKS DAM - April 1973 - Heavy rains caused flooding of construction site. Lake elevation $512\pm$.

DIERKS DAM - October 1973 - Downstream view of gate tower construction



- Main embankment and outlet works essentially complete. Work progressing on service bridge. DIERKS DAM - April 1974

APPENDIX B

SWDED - GL REPORT NO. 11227

DATED 27 SEPTEMBER 1971

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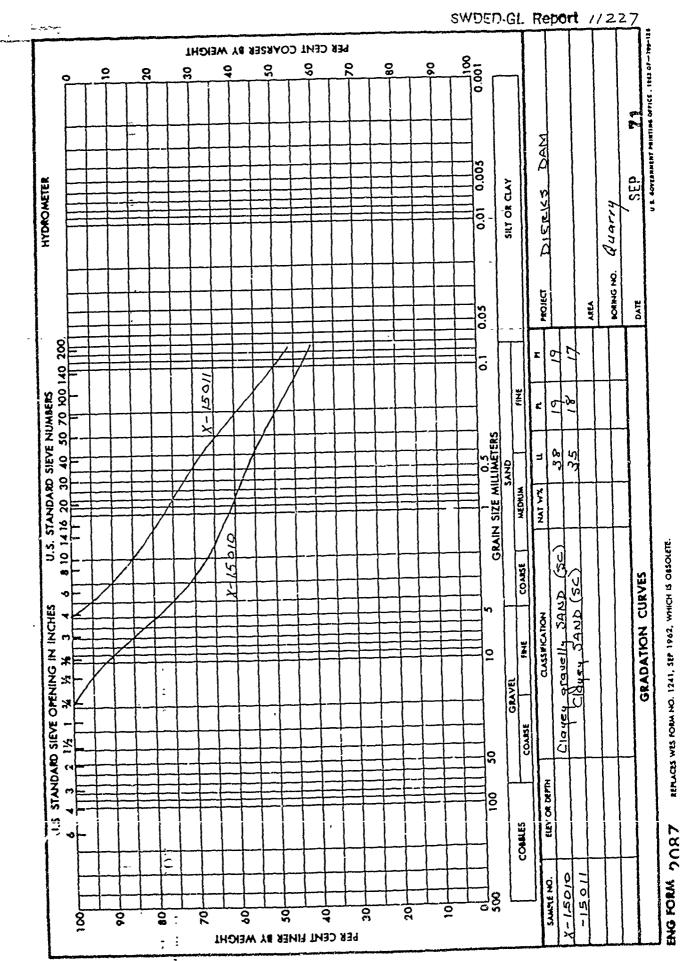
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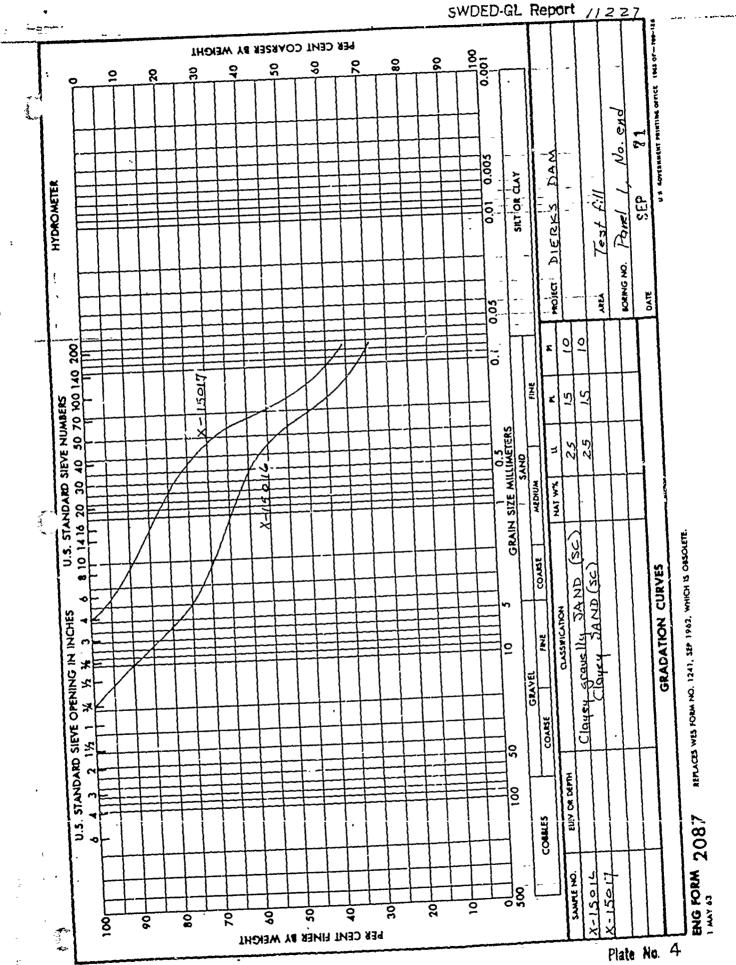
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SWDED-GL Report 11227 110 105 14 Water content, per cent of dry weight Standard compaction test 5L blows per each of 3 layers, with 5.5 1b rammer and 12 inch drop. ________inch diameter moil % > 3/4 in. \$ >
No. 4 Sample | Elev or Classification Depth Clayou gravelly SAND (5C) 2.69 est. 38 22 Sample No. X-15010 Matural water content in per cent Optimum water content in per cent 15.2 Max dry density in lb/cu ft 113.8 Remarks Project DIERKS DAM Area SEP 71 Boring No. COMPACTION TEST REPORT

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Plate 4. 10

SWDED-GL Report // 227 - 130 -हैं -125 120 115 Water content, per cent of dry weight Standard compaction test 56 blows per each of 3 layers, with 5.5 1b rammer and 12 inch drop. 6 inch diameter mold % > 3/4 in. % > Ro. 4 Sample | Kley or PL Classification No. Depth Clayey gravelly SAND (50) 2.68 est, 25 22 Sample No. X-15016 Matural water content in per cent Optimum water content in per cent 10.5 Max dry density in 1b/cu ft 123.9 Project Remarks DIERKS DAM Test fill Boring No. Panel 1, No. end Date SEP 73 COMPACTION TEST REPORT

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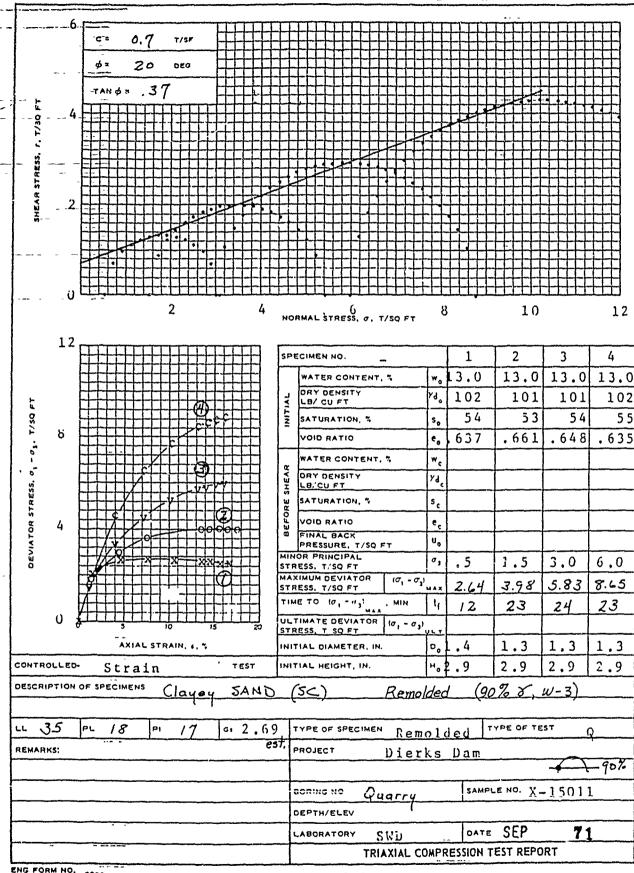
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SWDED-GL Report //227 115 -- -/10 Water content, per cent of dry weight Ste idard compaction test 25 blows per each of 3 layers, with 5.5 1b rammer and 12 inch drop. 4 inch diameter mold $\frac{$>}{3/4}$ in. Sample Elevior LL Classification Depth No. Clayey SAND (SC) 2.68 est. 25 15 Sample No. X-15017 Matural water content in per cent Optimum water content in per cent 12.3 Max dry density in 1b/cu ft 119.8 Remarks Project DIERKS DAM Test fill SEP Boring No. Panel 1, No. end Date 71 **COMPACTION TEST REPORT**

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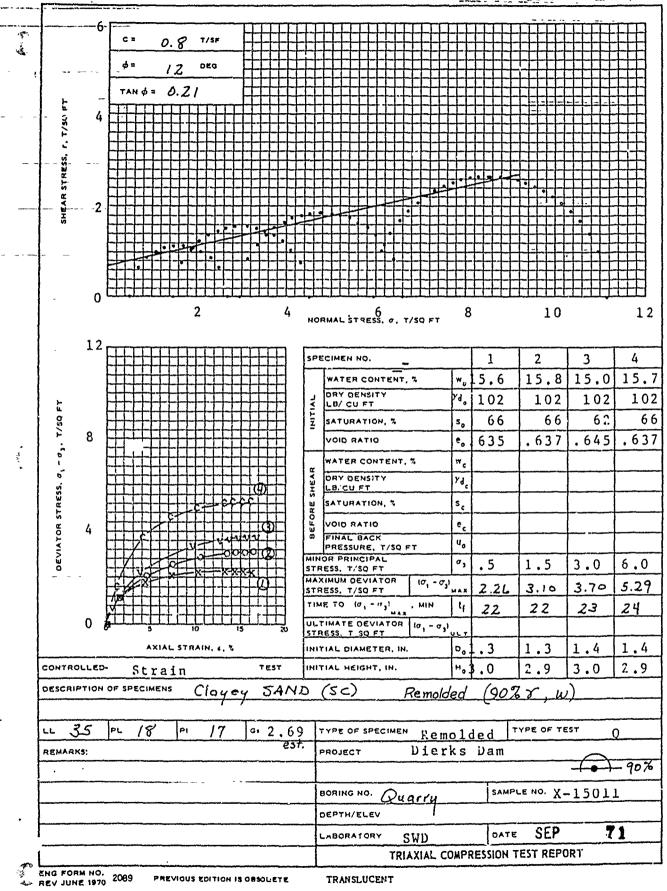
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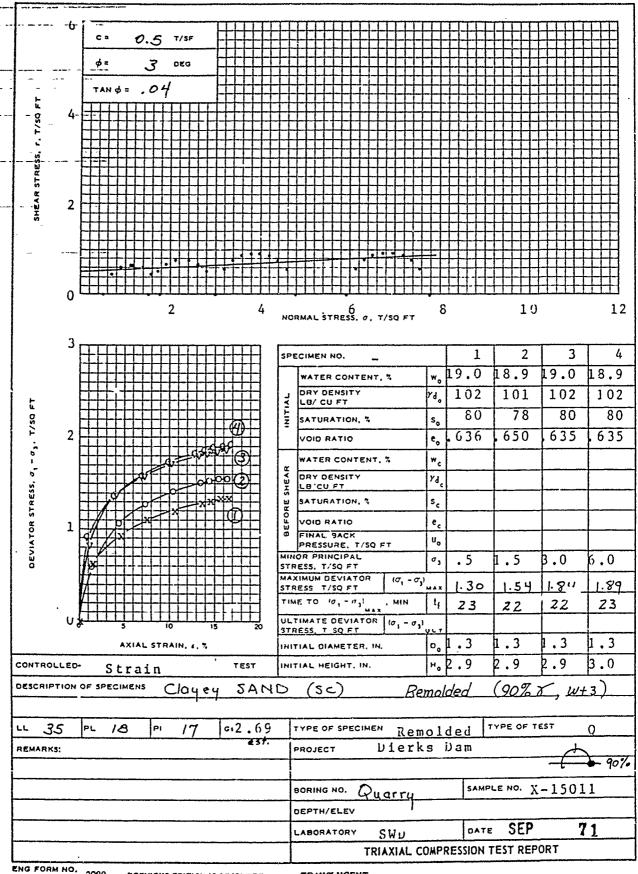


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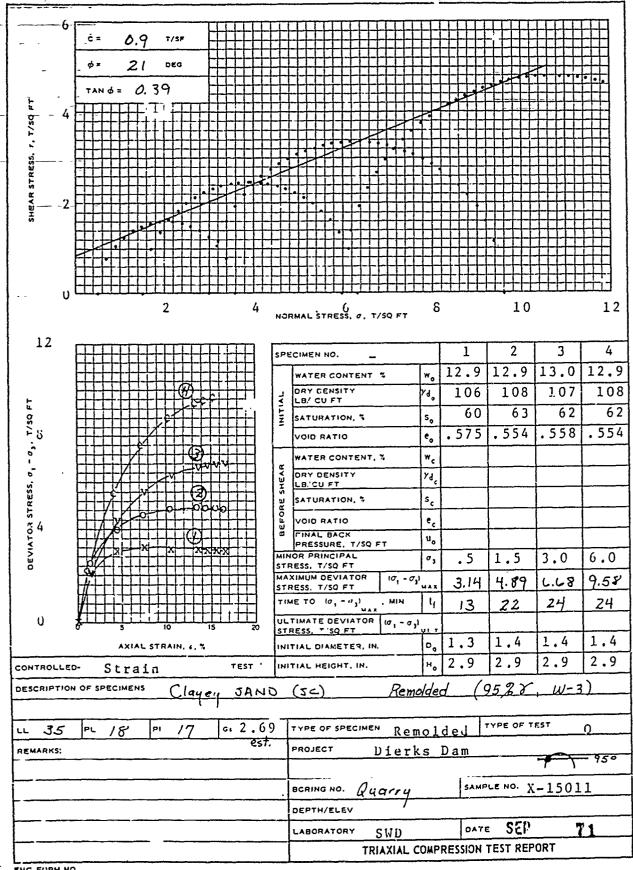
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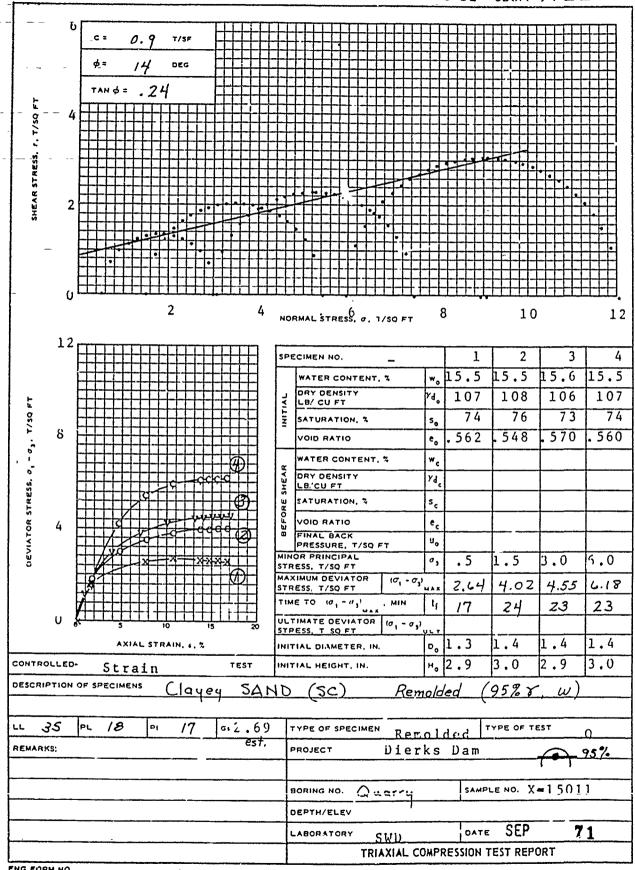
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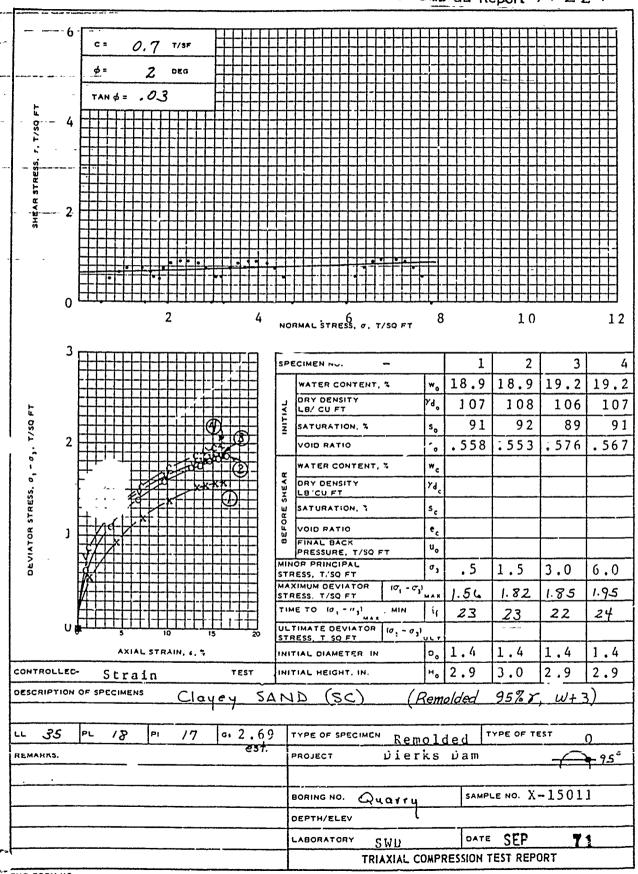
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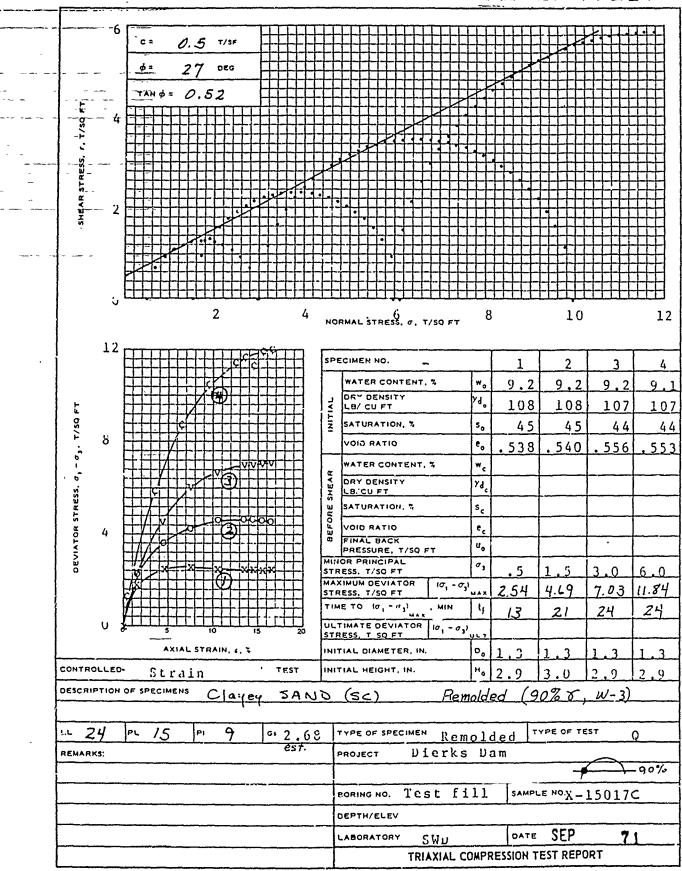
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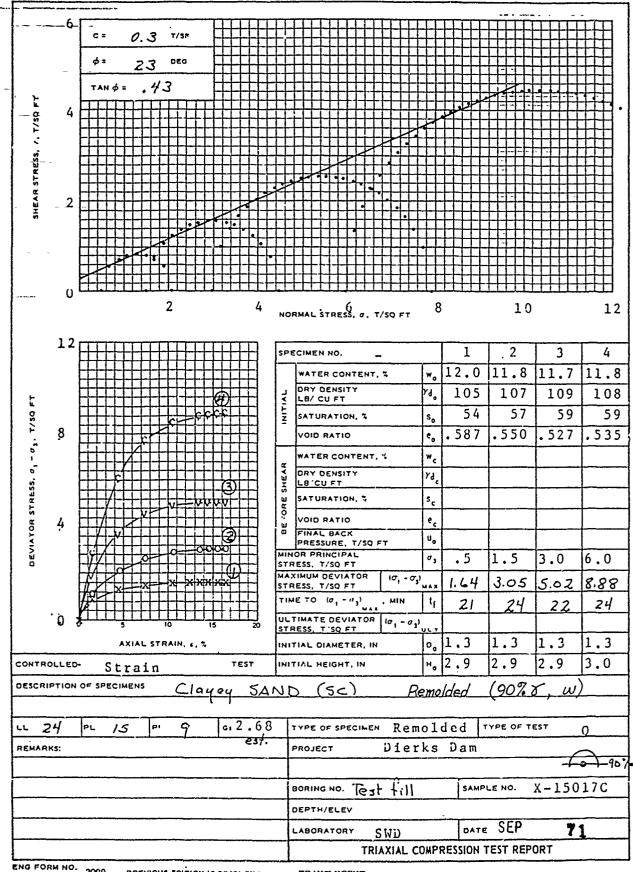
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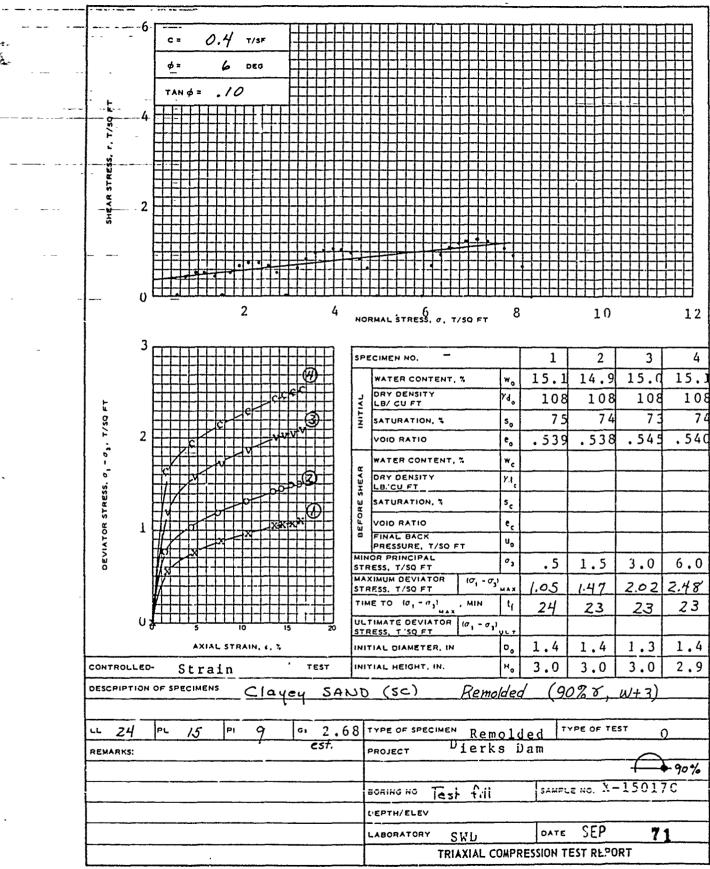
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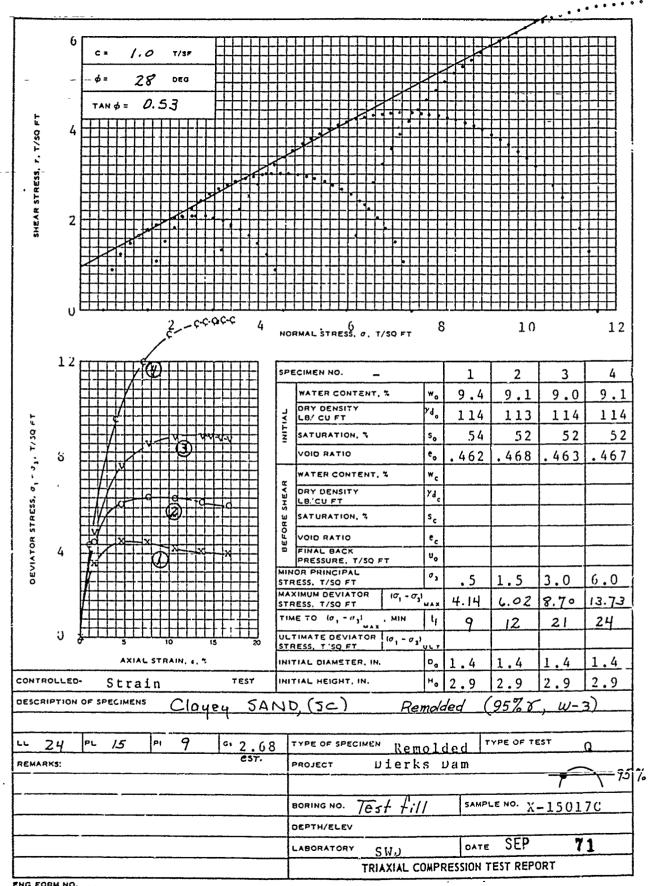
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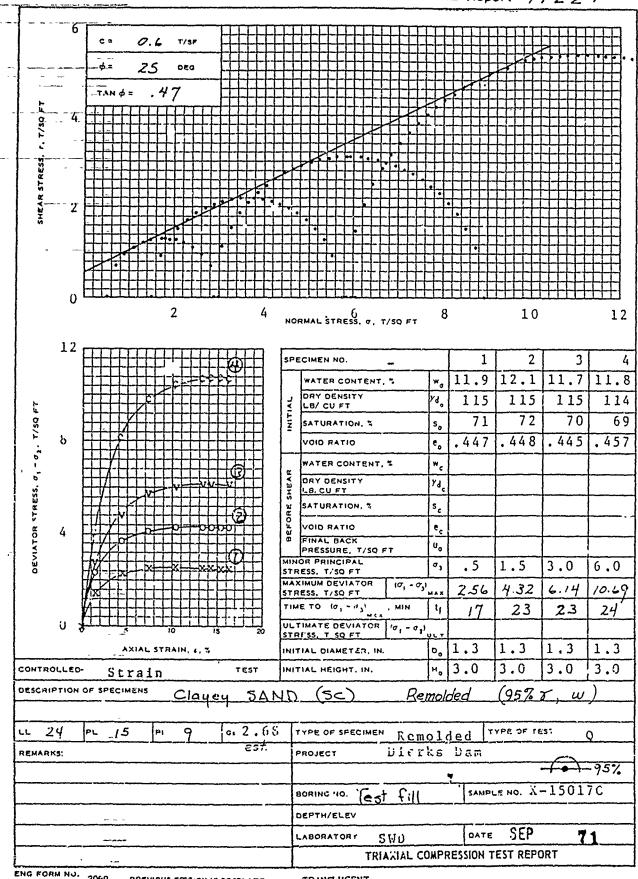
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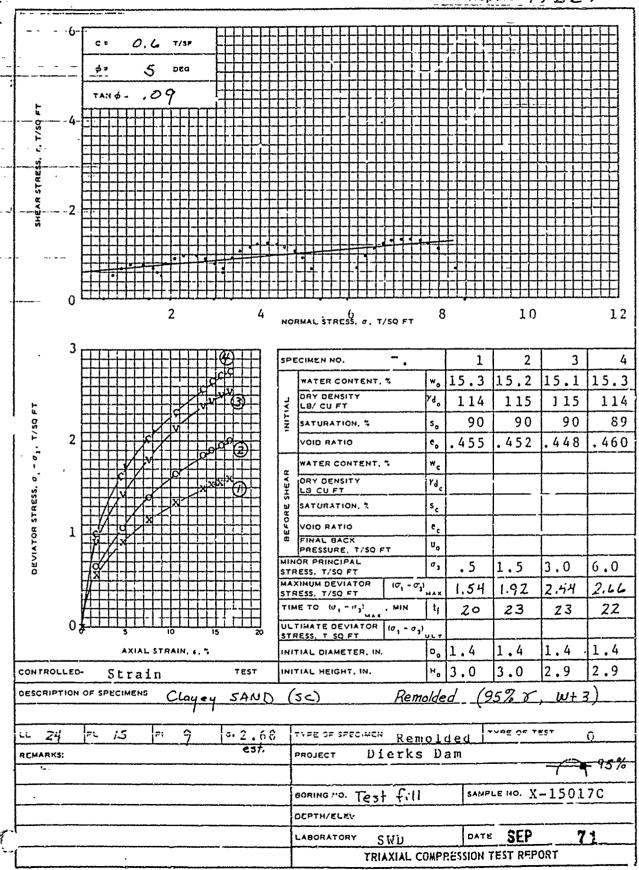
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ENG FORM 2092 (EM 1110-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

Plate No. PLATE IX-3

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The state of the s	g 6		6								
	SHEAR STREET	SHEAN SIKEN	2 0 2	4 NOR	6 MAL STRESS,	8 o, 1/SQ FT	10	1 2			
-	.010	TES	T NO.		1	2	3				
	z z	-	WATER CONTENT	w.	16.1%	15.9%	%	%			
	Z .020	₹	VOID RATIO	e,							
	VERTICAL DEFORMATION.	INITIAL	SATURATION	S.	%	%	%	%			
	₹ .090	L	DRY DENSITY, LB/CU FT	74	101	101					
	> .040		D RATIO AFTER NSOLIDATION	e,							
<u>.</u>	0 0.1 0.2 0.3 0.4 0.5		E FOR 50 PERCENT NSOLIDATION, MIN	tsn							
	HORIZ. DEFORMATION, IN.		WATER CONTENT	wı	18.6%	16.5%	%	%			
	SHEAR STRENGTH PARAMETERS	FINAL	YOID RATIO	er							
	•' = 30°		SATURATION	S,	%	%	%	%			
			RMAL STRESS, Q FT	σ	6.0	6.0					
	$fan \ p' = \frac{0.57}{0.57}$		KIMUM SHEAR ESS, T/SQ FT	-maz	3.64	3.24					
	$\epsilon' = 0, 0$ 1/SQ FI	ACT FAR	UAL TIME TO URE, MIN	tı	4260	3 1.50					
	CONTROLLED STRESS	RATI	E OF STRAIN, IN./MIN		.0001	.0001					
	CONTROLLED STRAIM		MATE SHEAR SS, T/SQ FT	Fult							
	TYPE OF SPECIMEN Remolded (90	%σ,ω)		3.0 11	N. SQUARE	. 5	IN. THICK			
	CL SIFICATION Clayey SAND (S	<u>c)</u>									
	u 35 PL 18		PI 17				G,				
	REMARKS		PROJECT DIER	KS	DAN						
		_]				······································		90%			
	•]	- AREA - 802ING NO. 0								
		_	BOSING NO. QUART!	/	DATE	SEP	7				
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1 JUN 45 2092 (EM 1113-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

Plate No. PLATE IX-3

					SWDE	D-GL Re	oort //2	227
	SHEAN STRESS. 7. 1750	SHEAR STRENGTH, s, T/6	6 2 2 0 0 2	4 R	6 MAL STRESS,	ε σ, T/SQ FT		12
	.010	TES	ST NO.		1	2	3	
•	Z .020		WATER CONTENT	w.	18.8%	18.7%	%	%
	OSO. CE OBRATION.	INITIAL	YOID RATIO	e.				
	.030	=	SATURATION	S.	%	%	%	%
	2 ::: : : : : : : : : : : : : : : : : :		DRY DENSITY, LB/CU FT	γa	102	102		
	E	co	DID RATIO AFTER INSOLIDATION	e,				
	0 0.1 0.2 0.3 0.4 0.5	CO	HE FOR SO PERCENT PHSOLIDATION, MIN	tan				
****	Horiz. Deformation, Im.		WATER CONTENT	WI	19.1%	17.4%	%	%
	SHEAR STRENGTH PARAMETERS	FINAL	VOID RATIO	e,				
	ø' = <u>28</u> °	_	SATURATION	Sı	%	%	%	%
	TAN 6' = 0.52	1/3	ORMAL STRESS, 50 FT	σ	6.0	6.0		
			XIMUM SHEAR RESS, T/SO FT	Tmaz	3.11	3.06		
	e' z		TUAL TIME TO ILURE, MIN	1	2850	2850		
	CONTROLLED STRESS		TE OF STRAIN, IN./MIN		.0001	.0001		
	CONTROLLED STRAIN	1511	1855, 1/50 /1	Tast				
	<u></u>		γ, w+3)		3.0	IN, SQUARE	. 5	IN, THICK
:	CLASSIFICATION Clayey SAND (SC	<u>) </u>			I			
	ll 35 Pl 18		P1 17				G,	2
:	REMARKS		PROJECT [LER	KS DAM			
:			AREA					90%
			BORING NO. Quarr		SA	MPLE NO X	-15011	
			DEPTH EL	J	DA	16 \$ EF	7	1
·	ENG FORM 2002 (EM LUIS 2 1004) INCOME	, <u>-</u> ,				EST REPO	·	, PLATE IX-3

2092 (EM 1110-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

2092 (EM 1110-2 1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

Plate HBLATE IX-3

1 JUN 45 2092 (EM 1110-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

	SHEAR STRESS, T, T/SQ FT	2-				7		SHEAR STRENGTH, 1, T/SQ FT	6 4 2			4 NOR	6 MAL STRESS,	8 o, T/SQ FT	10	12
-		,01	Ž				:		TEST	МО			1	2	3	
	<u>z</u>			V.	②	== ==	 -::::::			WATER CONTENT		w,	12.6%	2.1%	%	%
	VERTICAL DEFORMATION, IN.	.oz	711	Ø	X.X	r c) : : : : : : :		INITIAL	VOID RATIO		e,				
	SEFORW SEFORW	23	<u>.:</u>	.:.	<u></u>	`x-:	, <u>-</u>			SATURATION DRY DENSITY,		S,	%	%	% 	%
	נונאו		-:;;			:-	•			LB/CU FT		Υd	107	108		
	X	c	0		.2 (3 .	040	•	TIME	FOR 30 PERCENT		e, دءه				
		,	•			ATION		ر.	(0)	WATER CONTENT			13.7%	13.9%	%	%
							•		FINAL	VOID RATIO		eı				
		SHEAR	STREN	GTH P	ARAMI	ETERS		i	¥	SATURATION		Sı	%	%	%	%
		ø' :	·	34					NO#	MAL STRESS, O FT		σ	6.0	6.0		
Ì	1	ian \$:	=	_0,0		-				IMUM SHEAR SS, T/SQ FT	7,	mas	3.90	4.04		
		ć ':	=	0.0	0) ਜ'			UAL TIME TO URE, MIN		ŧį	4 3 0 0	4400		
	co	ONTROLLE	D STRE	55					RATE	OF STRAIN, IN /MIN			.0001	.0001		
ļ	<u>;</u> «	ONTROUE	D STPAI	11		 -			STRE	MATE SHEAR SS, T/SQ FT	,	ult		<u></u>		
-	TYPE OF S					old				%γ, w)			3.0	IN SQUARE	. 5	IN THICK
-	CLASSIFIC		<u>C</u>		 -		ND	_(5د' ا							
+		24			PL 	/5			-	PI 9					G,	.00
	REMARKS								- [PROJECT	υI	ŁRK	S DAM			90%
									_	AREA						
1									_	BORING NO Test	, f.	.//	SA	MPLE NO X	-15017	С
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APPENDIX C

SWDED - GL REPORT NO. 11332

DATED 13 JULY 1972

APPENDIX V

SUDED-GL REPORT NO. 11332 DATED 13 JULY 1972

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Results of Tests of Shale Material Spillway Quarry Dierks Dam - Tulsa District

1. Reference:

- a. Test Request TU-FM-72-20 dated 14 February 1972, and Supplement No. 1 thereto dated 22 February 1972.
 - b. SWDED-GL Report No. 11227 dated 29 September 1971.
- 2. Two samples consisting of three bags each, marked Quarry 1 and Quarry 2, were received 2 February 1972, and were assigned SWD Nos. X-15623 and X-15624. Four bags (two of each sample) were combined for testing and the remaining two bags saved in their "as-received" condition for subsequent X-ray diffraction, slaking tests, accelerated expansion tests and megascopic examination.
- 3. The four composited samples were crushed to pass the No. 10 screen after an examination of the materials indicated that individual particle sizes were much smaller.
- 4. Atterberg limits tests were conducted on the minus? fraction of the as-crushed material, on the material after 10 minutes of blenderizing, and after 30 minutes of blenderizing. Liquid limits were 25, 33 and 35, respectively. Gradation curves corresponding to each treatment are shown on Plate 1. A standard compaction test performed on the as-crushed material gave data shown on Plate 2.
- 5. Q triaxial specimens were remolded to densities near 90 and 95% of maximum density from the compaction test, at water contents of optimum and optimum -3%. Test data and results are shown on Plates 3 through 6.
- 6. R triaxial specimens were remolded to densities near 95 and 100% of maximum density from the compaction test, at water contents of optimum and optimum -3%. Test data and results are shown on Plates 7 through 10.
- 7. S direct shear specimens were remolded to densities near 90 and 95% of maximum density, at water contents of optimum and optimum -3%. Two specimens at each remolded condition were tested under normal stress of 8 T/sq ft, and strength envelopes drawn between the average strength value and the origin. Test data and results are shown on Plates 11 through 14.
- 8. In addition to the tests of the bag samples received 2 February 1972, R tests were performed on SWD Sample X-15011, received 20 August 1971, and previously tested for Q and S strengths (SWDED-GL Report 11227, 29 September 1971). Specimens were remolded to near 95% and 100% maximum density, at

Results of Tests of Shale Material Spillway Quarry Dierks Dam - Tulsa District (Lont'd)

water contents of about optimum and optimum -3%. Test data and results are shown on Plates 15 through 18. Strength values agree closely with those obtained in tests of the new material.

- 9. The two bag samples which were saved in the "as-received" condition were combined and the pieces subjected to megascopic examination. (See Petrographic Report). In general, three materials appeared present: shale, an intermediate material initially designated silty shale, and shaly sandstone. Typical specimens of the shale and sandstone were submitted to Dr. A. J. Ehlmann, Texas Christian University, for X-ray analysis; his report is included at the conclusion of the text. The X-ray diffractograms are inclosed in an envelope inside the back cover of the originals of this report.
- 10. Specimens of each of the three materials were subjected to five cycles of slaking in distilled water and oven-drying. Photographs were taken before the tests were begun, at the end of the first slaking (before oven-drying) and after the third and fifth cycles; the photographs are found on Plate 20. The shale specimens began to disintegrate within minutes of the first immersion in water; the silty shale was affected to varying degrees; and the shaly sandstone was affected almost not at all. Atterberg limits tests were performed on a portion of the shale material after the slaking-drying test: "quid limit 41, plastic limit 21, plasticity index 20.

SWD Form 600-B Reinstated 4 May 59	PETROGRAPHIC REPORT	Corps of SWD Testing Dallas,	
Report No. 11332	Project Dierks Dam	Date 13 Jul 72	initial GHII
Sample No.	Source See paragraph 2	below	

- 1. References. Reference is made to Tulsa District Test Request No. TU-TM-72-20 dated 14 February 1972 and to Supplement No. 1 dated 22 February 1972, requesting petrographic analysis and accelerated expansion tests on the sample described below.
- 2. Samples. Samples consisted of two bags of chunk rock from the spillway quarry selected at random from the six bags submitted. Chunk size ranged up to 11" x 6" x 5". The samples for petrographic examination, X-ray diffraction, accelerated expansion tests and slaking tests were selected from this portion of the total sample.
- 3. Description. The material from each bag was examined separately and divided into three types as noted below:

		Percent	
Type	Bag 1	Bag 2	Total
Shale	26	0	14
Silty Shale	7	42	24
Sandstone	67	57	62

- a. The shale was black, soft and fissile with occasional thin stringers of very fine-grained sandstone. Some iron staining was noted. Simple staining tests with benzidine indicated the presence of a small percent of swelling-type clays although X-ray diffraction did not show this. This material broke down readily in both the accelerated expansion test (Plate 19) and in the slaking test (Plate 20).
- b. The silty shale was dark gray and moderately hard. Thin, randomly oriented, carbonaceous inclusions up to 1-1/2" in diameter were noted throughout the rock. During the slaking test some of the particles broke down during the first cycle; others retained their general outward shape, but cracked on random planes during the test period (Plate 20). The only breakdown that occurred during the accelerated expansion test was random cracking in two particles and slight spalling in two particles (Plate 19). See below for a discussion of the latter test. It was found that after a short period of soaking in water, this material could be crumbled between the fingers, and with a little effort and the addition of more water, could be completely broken down. A small sample processed in this manner and sieved over a No. 325 (0.044 mm) sieve resulted in approximately 70 percent passing the sieve. The 30 percent retained consisted predominantly of quartz with a trace of chert. Twenty percent of the material passing was clay minerals with the remaining 80 percent consisting predominantly of quartz with small percents of miscellaneous minerals. Staining tests indicated a small percent of the clay minerals to be of the swelling type.

- c. The sandstone was gray, moderately hard, highly argillaceous, and fine-grained. The sand grains were subrounded to subangular, and consisted predominantly of quartz with a trace of chert and quartzite. Some of the surfaces of the cock had thin, iron-stained, shale coatings with a trace of mica. Large chunks of the rock were difficult to break with a hammer, but it was possible to break off small pieces with the fingers. The rock was only slightly affected by either slaking or accelerated expansion tests (Plates 19 and 20).
- 4. Accelerated Expansion Tests. These tests were performed in accordance with the procedures in CRD-C 148-69. Photographs, taken at various intervals during the test, are shown on Plate 19. Breakdown of the shale began within five minutes after submerging the sample, and continued through 24 hours. No obvious additional breakdown occurred after that time. The silty shale showed no signs of breakdown during the first two hours, after which slight cracks appeared in two particles. At 24 hours, two particles had cracks through the entire particle, and two other particles began slight spalling. Only slight additional breakdown occurred during the remainder of the test. No evidence of any breakdown of the sandstone occurred other than small deposits of sand grains around each particle. The percent loss (passing the 3/4" sieve) of each type material at the end of testing is shown below:

Rock Type	Loss, %
Shale	37.7
Silty Shale	0.9
Sandstone	0.6



TEXAS CHRISTIAN UNIVERSITY

Fort Worth, Texas 76129

Department of Geology

March 30, 1972

Director
Southwestern Division Laboratory
U. S. Army Engineer Div. Southwestern
Corps of Engineers
4815 Cass Street
Dallas, Texas 75235

Dear Sirs:

The X-ray analysis of the two samples from Dierks Dam indicated:

1. Shaley sample:
 Illite ------50% estimated
 Kaolinite -----20% "
 Quartz -----20% "
 Chlorite -----10% "

The sample appears to be a non-expanding black shale.

2. Sandy sample:

The sample appears to be a sandstone of graywacke type differing from the type only in having no feldspar with the quartz.

If I can be of additional help on interpretation, do not hesitate to call me.

Very truly yours,

A. J. Ehlmann

Professor of Geology

AJE:vf

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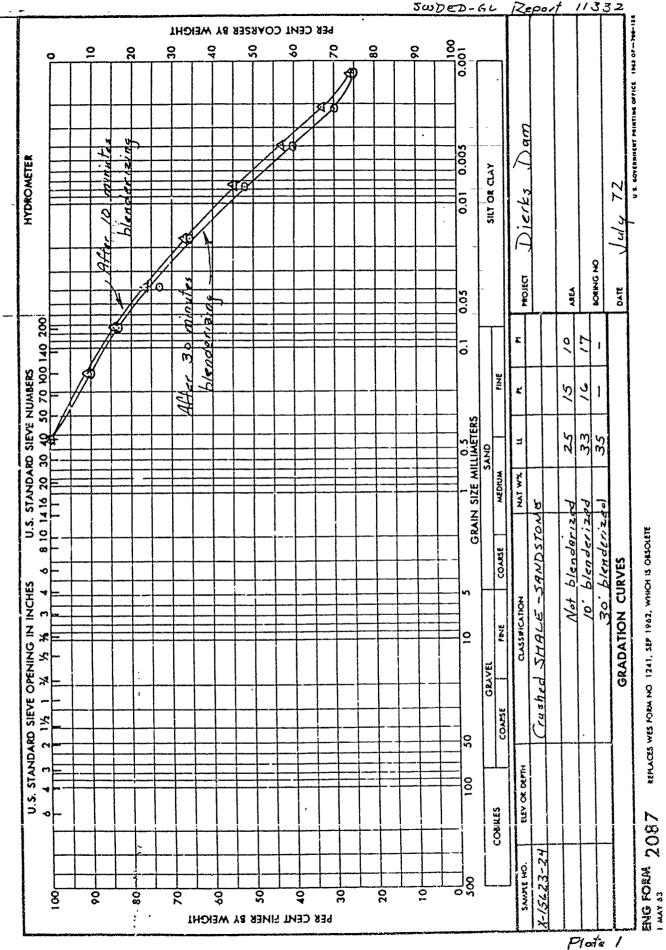
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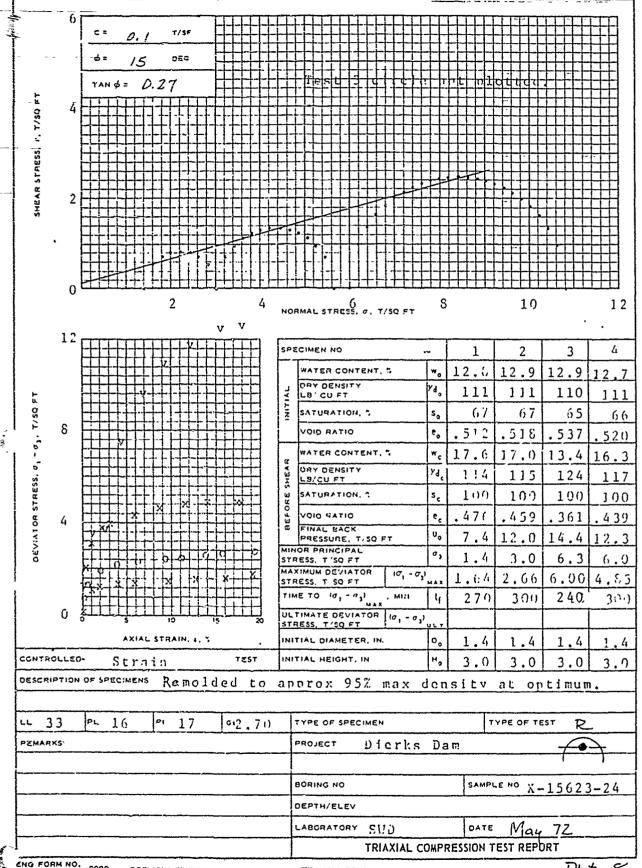
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LL 33	Pt 16 Pt 17 92.70	TYPE OF SPECIMEN ROMO	
REMARKS:		PROJECT Dierks	Dam
		BORING NO	SAMPLE NO X-15623-24
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		LABORATORY SWD	DATE May 72
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		HORIZ. DEFORMATION, IN		WATER CONTENT	w,	15.4%	14.6%	%	%
•	SHEAR	STRENGTH PARAMETERS	FINAL	VOID RATIO	e,				
	,	= 31°		SATURATION	Sr	%	%	%	%
	φ =			RMAL STRESS, GQ FT	σ	8.0	8.0		
	TAN 6' =			XIMUM SHEAR ESS, T/SQ FT	MAX	4.96	4.63		
	c' =	<u>0</u>		TUAL TIME TO LURE, MIN	tr	3250	5900		
	CONTROLLE	D STRESS	RAT	E OF STRAIN, IN /MIN		.0001	,0001	_	
	CONTROLLE) STRAIN	ULT	TMAYE SHEAR ESS, T/SQ FT	ult				
	TYPE OF SPECIMEN	Remolded to app at optimum - 3%	ro	х 90% max de	ns	3.0	N. SQUARE	. 5	IN. THICK
	CLASSIFICATION	CL							
	u 33	PL 16		PI 17				G. 2	. 70
	REMARKS			PROJECT Di	erk	s Dam		\bigcirc	_
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				BORING NO. DEPTH					
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1 JUN 43 2092 (EM 11:0-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

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PLATE IX-3

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					NOR	AAL STRESS,	σ, T/SQ FT		
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	<u>z</u>			WATER CONTENT	w.	12.6%	12.0%	%	%
	z .02			VOID RATIO	e.	.598	.597		
	VIII VIII VIII VIII VIII VIII VIII VII		MINAL						
	P. C.		Z	SATURATION	s.	56%	56%	%	%
	GF C			DAY DELICITY	├	100	105		
	VERTICAL DEFORMATION, IN			DRY DENSITY, LB/CU FT	74	105	105		
	TIC.			D RATIO AFTER	e.				
	> E		COI	NSOLIDATION	<u> ''</u>			ļ	
	(0 0.1 0.2 0.3 0.4 0.5	TIM	E FOR 50 PERCENT NSOLIDATION, MIN	tsn				
-			H		 	12 5	1 / 1		
		HORIZ. DEFORMATION, IN.		WATER CONTENT	wı	13.5%	1 1 %	%	%
			FINAL	YOID RATIO	eı				
	SHEAR	STRENGTH PARAMETERS	2						
				SATURATION	Sı	%	%	%	%
	ø'	= 31	NO	RMAL STRESS,	 	8.0	8.0		
		4.	1/5	Q FT	0	0,.,	0.0		
	TAN φ' :	= 0,60		XIMUM SHEAR	mas	4.72	4.00		
		= 0.0 T/SQ FT	-	233, 1/30/1	·				
	•	=		UAL TIME TO LURE, MIN	t,	2900	2 80 0		
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	CONTROLL	ED STRESS	RAT	E OF STRAIN, IN /MIN				_	
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	- COMIXONE	O SIKAIN	STR	ESS, T/SO FT		-	L		
	TYPE OF SPECIME	н				3.0	N, SQUARE	. 5	IN. THICK
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1 JUN 65 2092 (EM 1110-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCIENT)

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PLATE IX-3 Plate 12

		WATER CONTENT	1 2 w. 9.6% 9.7% e317 .517	3 % %
	VERTICAL DEFORMAL	DRY DENSITY, LB/CU FT COID RATIO AFTER CONSOLIDATION IME FOR 50 PERCENT	S. 50% 50% y _d 111 111 e.	% %
Sagge of the sagge	HORIZ. DEFORMATION, IN.	WATER CONTENT	w, 15.4% 15.3%	% %
	o' = 32	SATURATION	S ₁ % %	% %
	TAN 8' = .62	AAXIMUM SHEAR Tme	F (17 1 1)	
	c' = 0 1/50 FI	CUAL THE TO	t, 5900 2900	
		ATE OF STRAIN, IN./MIN	.0001 .0001	
		ILTIMATE SHEAR Tul		
	TYPE OF SPECIMEN Remolded to app density at optim	rox 95% maxim	um 3.0 IN. SQUARE	.5 IN. THICK
	LL 33 PL 16	PI 17		G, 2.70
	REMARKS	PROJECT Dier	ks vam	
	-	- AREA		,
		BORING NO.	SAMPLE NO. X	-15623-24
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1 JUN 45 2092. (EM 1110-2-1906) PREVIOUS EDITIONS ARE OBSOLETE (TRANSLUCENT)

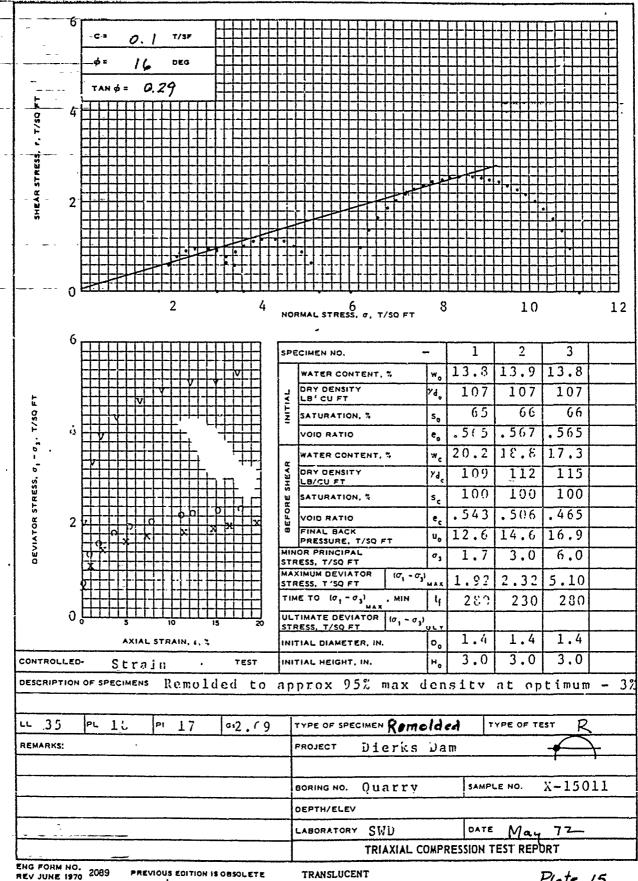
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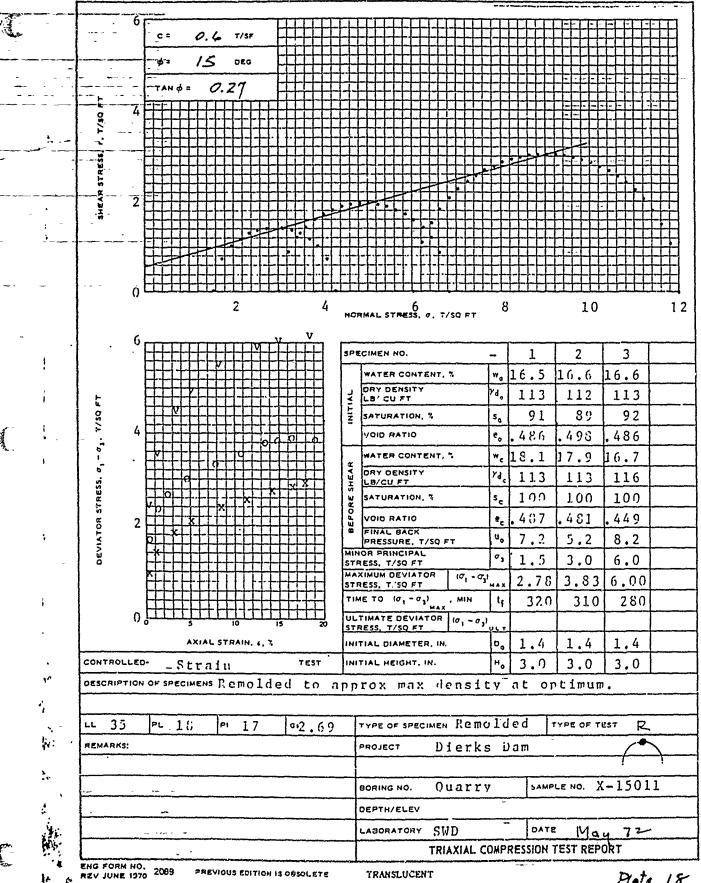
PLATE IX-3 Plate 14



PREVIOUS EDITION IS OBSOLETE

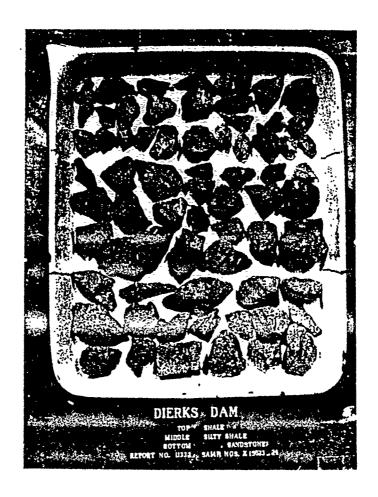
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Plates 16 + 17 nov available

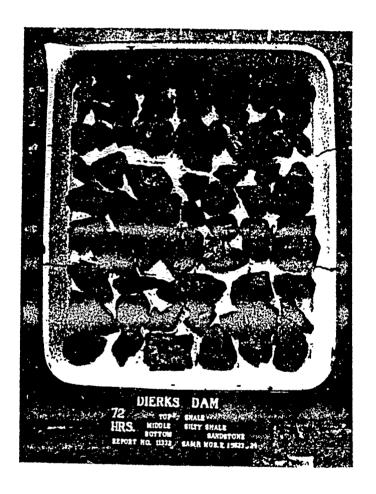


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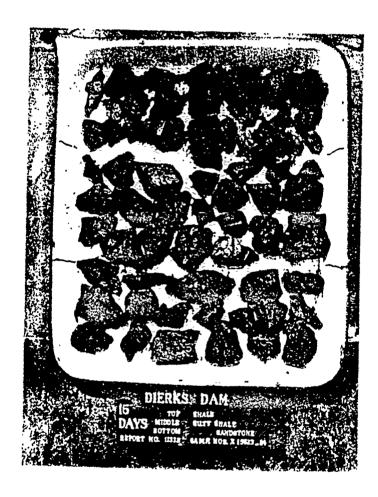
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Accelerated Expansion Tests



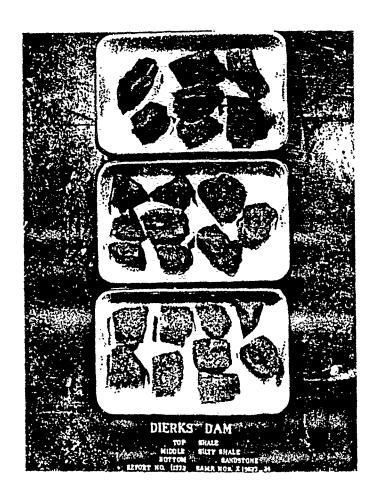
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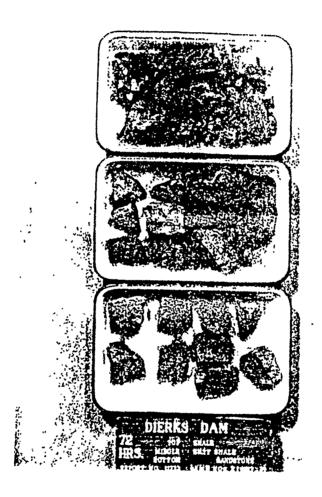
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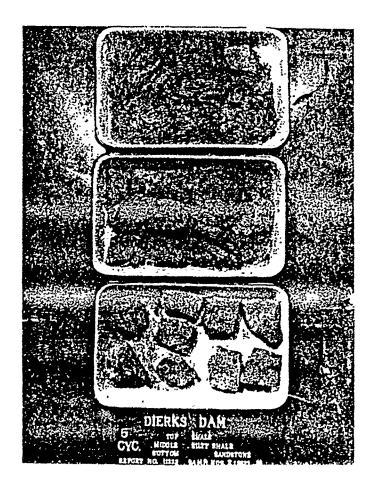
Slaking Tests



Slaking Tests



Slaking Tests



Slaking Tests

APPENDIX D

FOUNDATION MATERIALS

SECTION 1. UNCONFINED COMPRESSION TESTS

(TRANSLUCENT)

C 8927

PLATE XI-2

Ø125 Sketch of specimen after failure TEST TYPE (Check one) ▼ Controlled-stress 8,600 lbs/min. Controlled-strain AXIAL STRAIN, e , \$ ☑ Vodisturbed Type of specimen Test No. Test No. Test No. Test No. ☐ Remolded Water content 5. ... ۰۰ Void ratio Sį Saturation 7_d Dry density, lo/cu ft Time to failure, min Unconfined compressive strength, tons/sq ft Undrained shear strength, tons/sq ft Sensitivity ratio Classification SANDSIONE ᄔ PL Specimen Specimen Diam in. Project __ Diarks Remarks Follmoatton Material _ Sample No. __M-3918 Boring No. _ Date 901 64 Depth, 🚉 18.1-190 UNCONFINED COMPRESSION TEST REPORT ENG PORM : 3659 PLATE NO. G PLATE XI-2(3) (TRAHSLUCENT) c 8927

SATIGI Report No 8992

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		Sketch of specimen after failure	0						-
-	-	TEST TYPE (Check one) Controlled-stress 9,300 lbs/min.				3.0 4.D L 378418, e		7.0	-
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***	;	Yoid ratio		e ₀		-			
		Saturation		S _o	5	\$	*	*	
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SWDGI-Renort No. 8992

SWDCL Report-Ro-BS92 TONS/SQ Y Sketch-of-specimen after failure TEST TYPE . - --(.Check one) X - Controlled-stress 1,500 lbs/min. Controlled-strain AXIAL STRAIN, є , \$ ∀ Undisturbed Type of specimen Test No. Test No. Test No. Test No. ☐ Remolded ٧٥ Water content 4.2 Initial Void ratio Saturation $\gamma_{\mathbf{d}}$ Dry density, lb/cu ft 144 ţ. Time to failure, min Unconfined compressive strength, tons/sq ft Undrained shear strength, tons/sq ft 16.7 Sensitivity ratio Classification SANDSTONE. PL. Specimen Spec imen Diam Height in. in. Project Dierks Dam Remarks Area Foundetion Material Boring No. 602C-17 - Sample No. -M-3104 DCT 84 11.4-12.3 Date UNCONFINED COMPRESSION TEST REPORT PLATE NO. 2 ENG FORM 3659 PLATE XI-E (TRANSLUCENT)

_(TRANSLUCENT)

C 8927

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•	Classification SHALE												
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	Specimen cm Specimen cm				<u></u> -			====		====			=
	Diam 4.0 in. Height 7.5 in.					Project <u>Diecks Dom</u>							
	Remarks SLIALE, black; contains gray,												
•	hard, well comented fine grained				Area Foundation Material								
	Sandstone bands from hairling to 2" Boring Ro. 190 Sample No.							ple No&	1-11,767	:			
2	size at 16 to 3"internals Bodding				D	epth, 🖭		70	0	Dat	e SEP	65	
,	die is about 60°				UNCONFINED COMPRESSION TEST REPORT								
	ENĆ I	1 4 3459 (THANKS TOWNS	Dr. Ares - Cr. o.										

c 8 9 2 7 Plate No. J

APPENDIX D

FOUNDATION MATERIALS

SECTION 2. Q TRIAXIAL TESTS

SWDGL_Report Mr. 8992 Hormal Stress, o , tons/sq ft (2) ④ Test No. Water content 6.9 165 Void ratio 508 485 480 523 Saturation so 90 89 Dry density lb/cu ft 110.1 020 Water content Saturation \$ \$ Consolidation press., tons/sq ft Void ratio ec Water content Axial Strain, \$ Void ratio Major principal Shear Values 68 stress, tone/sq ft himor principal 3.05 4.57 8.99 Ø = <u>8.1</u> stress, tons/sq ft 3.0 6.0 0.5 $tan \not = \underline{.142}$ Time to failure, min _tons/sq ft Initial diameter, om in 1.4 1.4 1.4 Initial height, Ho, om in 29 3.0 3.0 Type Test_ Method of Saturation . Controlled Stress x Controlled Strain Type of specimen UNDISTUPBED Rate of strain in./min 0 01 Classification Silty SAND (SM) Dto G 2.66 NP PL NP NP PI A† 15% strain Diarks Dam Foundation Material Boring No. GD2C-II Sample No. M - 25014 Depth Date OCT 6 4 5.0 - 7.0 TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089

PREVIOUS EDITIONS ARE OBSOLETE.

(TRANSLUCENT)

PLATE NO. 8 0 3426

APPENDIX D

FOUNDATION MATERIALS

SECTION 3. R TRIAXIAL TESTS

SWIG Amount No 8992 Mormal Stress, o , tons/sq ft ① 3 2 Test No. Water content 119 17.9 \$ <u>16.3 %</u> Void ratio 523 498 Saturation 87 Pore Dry density lb/cu ft 7a 109.0 110.9 Water content Induced Saturation <u>100 ^{\$}</u> 100 Consolidation press., tons/sq ٥c 1.55 3 05 5.95 Void ratio 441 Water content ٧f 6.6 Axial Strain, % Void ratio Shear Values ress, tons/sq ft Minor principal Ø = 20.2 stress, tons/sq ft 3.05 5.95 $tan \phi = __367$ Time to failure, min 230 235 105 c = 0.5 tons/sq ft Initial diameter, emin 1.4 1.4 Initial height, Ho, omin 3.0 3.0 Type Test Method of Saturation Back Pressure Controlled Stress Controlled Strain Type of specimen in./min UNDISTURSED Rate of strain (lenimon)500 o Classification Silty SAND (3M) [₿]19 G PL PI 2.66 NP NP NP Remarks At 15% Strain Project Dierks Dam Foundation Material Area Sample No. M- 2901 Boring No. 6DZC-11 Depth Date TOTRA . 5.0 - 7.0 , TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089

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PREVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

PLATE NO. 9

1 1424

APPENDIX D

FOUNDATION MATERIALS

SECTION 4. DIRECT SHEAR TESTS

SWDGL Report No 8992 strength, Normal Stress, o , T/sq ft ➂ Test No. Water content Void ratio Saturation Dry density 1b/cu ft Void ratio after consolidation Time for 50% < 1 consolidation, min Water content Void ratio Horiz Deformation, in. Saturation Actual time to failure, min. 1900 3/00. Shear Values Normal stress T/sq ft d' - 32.6 Maximum shear strength, T/sq ft tan 0' - 638__ c' = 0.1 Test Type (Check One) Controlled, strain Controlled, stress Type of Spectmen UNDISTURBED in. Square in. Thickness 30 Classification silty sand (sm) Pemarks Project Bate of shoot Tost 1 000009 14/200 Foundation Material Area 0 0001 IN/MY? Boring No. 602C-11 Sample No 0000/ 11/HIA Depth Date OCT6 4 37 5.0 - 7.0 DIRECT SHEAR TEST REPORT

ENG FORM 2092

PREVIOUS EDITIONS ARE ORSOLETE. (TRANSLUCENT)

PLITE NO 11 C 3431

SIVINGE Report NO 9125 Mormal Stress, o , T/sq ft .030 (z)Test No. ğ- -Water content ٧, 02.0 Void ratio 250 263 So Saturation 91 % Dry density lb/cu ft 1368 Void ratio after consolidation Time for 50% ^t50 consolidation, min Water content .010 Void ratio Horiz Deformation, in. 5 Saturation 9, Actual time to failure, min. 900 200 Shear Values Normal stress σ T/sq ft d - 34.3° Maximum shear strength, T/sq fi tan 0' = .681 ۲ c' = 0.3 Test Type (Check One) Controlled, stress Controlled, strain, 0.00 01 in /min. Type of Specimen UNDISTURBEL in. Square in. Thickness SHALE Classification LL 36 PL 20 DJU G ΡI 16 2.74 Remarks Project Diarke Dam Area folindation control Sample No. M-5070 Boring No. 6 C-13A Depth Date DEC 6 DIRECT SHEAR TEST REPORT C 3437 2072

•:.

SWOGL REPORT MOSIZE. Normal Stress, o , T/sq ft ① 3 ③ Test No. Water content ٧, 18.1 Change, 005 Void ratio s_o Saturation <u>80</u>3 Intchness 010 Dry density 7_d lb/cu ft 194.8 Void ratio after consolidation Time for 50% t₅₀ consolidation, min Water content 0.2 Void ratio Horiz Deformation, in. s, Saturation Actual time to failure, min. 700 Shear Values Normal stress σ T/sq ft d' = 200° Maximum shear strength, T/sq ft tan Ø' = __364 0.11 c' = _0.2 Test Type (Check One) Controlled, strain, 0.00 01 in./mis, Controlled, stress Type of Specimen UNDISTITIET. in. Square 3.0 in. Thickness Classification CLAY (CH) (weathered shale) PL 24 54 PI 30 ₱30 G 2.77 Remarks Project Dierke Dem Area ל חווחל יווים מישוביים Sample No. M-BOTE Boring No. Depth Date Di 1.4 # 25-35 DIRÈCT SHEAR TEST REPORT

ENG FORM 2092

PREVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

PLATE NO 3

C 3437

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APPENDIX E

COMPACTED MATERIALS

SECTION 1. Q TRIAXIAL TESTS

SWDGL Report 9467 0 T) .201 10.0 NORMAL STRESS, σ , TONS/SQ FT (I)2 3 TEST NO ④ WATER CONTENT 11.6 % 115 % 11.6 % 11.5 % VOID RATIO 426 +24 410 431 SATURATION 72 % 75 % 73 % 72 % DRY DENSITY LB/CU FT 116.9 117.0 118.2 116.3 WATER CONTENT ٧, ٧, SATURATION % CONSOLIDATION PRESS , TONS'SO FT VOID RATIO WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO SHEAR VALUES MAJOR PRINCIPAL STRESS, TONS/SQ FT 4.49 4.29 14.65 21.62 MINOP PRINCIPAL 30.9 STRESS, TONS/SQ FT 0.5 15 3.0 6.0 600 TIME TO FAILURE MIN 9 8 12 13 0:8 JONS/SQ FT INITIAL DIAMETER OM IN 1.4 1.4 1.4 1.4 INITIAL HEIGHT, Ho, CM / N 3.0 3.0 3.0 3.0 9 TYPE TEST . METHOD OF SATURATION CONTROLLED STRAIN CONTROLLED STRESS TYPE OF SPECIMEN **विकार्गतिकार्गति** . 015 IN /ÅIN RATE OF STRAIN CLASSIFICATION AY, sandy (CL-ML) G, 2.67 PROJECT Dierks Dam REMARKS. AREA BOTTOW Material $(8, \omega)$ SAMPLE NO BORING NO. 81.200 DEPTH DATE NOV 65 0.0-3.0 TRIAXIAL COMPRESSION TEST REPORT ENG FORM GPO 1944 OF-714-729 2089 PREVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

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SWDGL Report 9467 P. W. -20.0 10.0 NORMAL STRESS, σ , TONS/SQ FT -- \bigcirc (2) (3) (4)TEST NO. WATER CONTENT 14.9% 14.3 % 4.6% 14.6 % VOLUMETRIC STRAIN, VOID RATIO 505 .514 .501 .507 SATURATION 77 % 76 % 77 % 77 % DRY DENSITY 110.1 111.1 LB/CU FT 110.8 110.6 WATER CONTENT 5, SATURATION % CONSOLIDATION MESS , TONS/SQ FT OITAR GIOY 20 10 ۳, WATER CONTENT AXIAL STRAIN, % VOID RATIO MAJOR PRINCIPAL SHEAR VALUES 0.92 5.94 STRESS, TONS/SQ FT 647 19.40 MINOR PRINCIPAL STRESS, TONS/SQ FT s = . 20,0 _ . 0.5 1.5 3.0 6.0 TAN . = 277 .. TIME TO FAILURE, MIN 22 21 17 25 ·= .0.4 TONS/SQ FT INITIAL DIAMETER, CM IN 14 14 1.4 1.4 INITIAL HEIGHT, Ha, SEM IN 3.0 30 3.0 30 METHOD OF SATURATION CONTROCLED STRAIN CONTROLLED STRESS Remolderied TYPE OF SPECIMEN RATE OF STRAIN .02 IN./AIN CLASSIFICATION CLAY, Sandy (CL-ML) п 21 15 6 267 PROJECT Dierks Dam ŘĚMAŘKS. AREA BOTTOW Material SAMPLE NO. M - 12,496 BORING NO. 84-200 DEPTH 65 DATE NOV 0.0 . 3.0 TRIAXIAL COMPRESSION TEST REPORT

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2089 PREVIOUS EDITIONS ARE OSSOLETE

(TRANSLUCENT)

670 1964 07-714-729 Plate No. 16 \$1- . . . SWDGL Report 9467 TONS/50 2.5 15.0 5.0 10.0 NORMAL STRESS, a, TONS/SQ FT---(1) (2) 3 4 TEST NO WATER CONTENT 11.7 % 11.8 % 11.6 % 118 % VOLUMETRIC STRAIN, VOID RATIO .500 513 .506 .514 S SATURATION 61 62 % 61 62 % DRY DENSITY 110.2 110.7 LB/CU FT 111.1 110.1 WATER CONTENT TEST s, SATURATION ٧. CONSOLIDATION MESS , TONS/SO FT • , VOID RATIO WATER CONTENT AXIAL STRAIN, % •, SHEAR VALUES MAJOR PRINCIPAL STRESS, TONS/SO FT 2.96 6 12 7.81 20 84 ٠̈₃ MINOR PRINCIPAL 0.5 1.5 30 STRESS, TONS/SQ FT 6.0 TAN O ... GRS TIME TO FAILURE, MIN 15 13 18 28 TONS/SQ FT INITIAL DIAMETER, EN IN 14 1.4 14 1.4 INITIAL HEIGHT, Ho, CM IN 30 3.0 3.0 3.0 TYPE TEST -ONTROLLED STRAIN CONTROLLED STRESS METHOD OF SATURATION molded TYPE OF SPECIMEN RATE OF STRAIN .015 IN /MIN CLASSIFICATION CLAY, sandy (CL-ML) 21 6 2.67 Dierks Dom PROJECT REMARKS. AREA BOTTOW Material (958,W) SAMPLE NO. BORING NO. 8A-200 M-12,490 DEPTH DATE NOV 65 0.0-3.0 TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089 PREVIOUS EDITIONS ARE OISOLETE

(TRANSLUCENT)

GPO 1944 07-714-725

SWOOL Report 9467 (1) **②** SHEAR STRESS, T, TONS/50 FT 15.0 NORMAL STRESS, o, TONS/SQ FT 4 (1) @ 3 TEST NO WATER CONTENT 8.7 % 8.6 % 8.7 % 8.5 % VOID RATIO 505 504 505 505 SATURATION 45 % 46 % 45 % 46 % DRY DENSITY 110.7 110.7 110.7 110.8 WATER CONTENT \$ SATURATION % ٠, % CONSOLIDATION PRESS , TONS/SQ FT VOID RATIO ٠, WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO MAJOR PRINCIPAL STRESS, TONS/SQ FT SHEAR VALUES 3.19 6.74 21.98 12.26 MINOR PRINCIPAL STRESS, TONS/SQ FT 0.5 1.5 3.0 6.0 TAN 0= :654_ TIME TO FAILURE, MIN 8 16 33 14 c=_ 0.4 _._TONS/59 FT INITIAL DIAMETER, CM IN 1.4 1.4 1.4 INITIAL HEICHT, Ho, MA I N 3.0 3.0 3.0 3.0 METHOD OF SATURATION CONTROLLED STRESS ONTROLLED STRAIN TYPE OF SPECIMEN Remolde ! . 0/5 IN /MIN RATE OF STRAIN **CLASSIFICATION** PROJECT Dierks REMARKS_ AREA BOTTOW Material M-12,496 BORING NO. SAMPLE NO 8A.200 DEPTH DATE NOV 65 0.0 - 3.0 TRIAXIAL COMPRESSION TEST REPORT

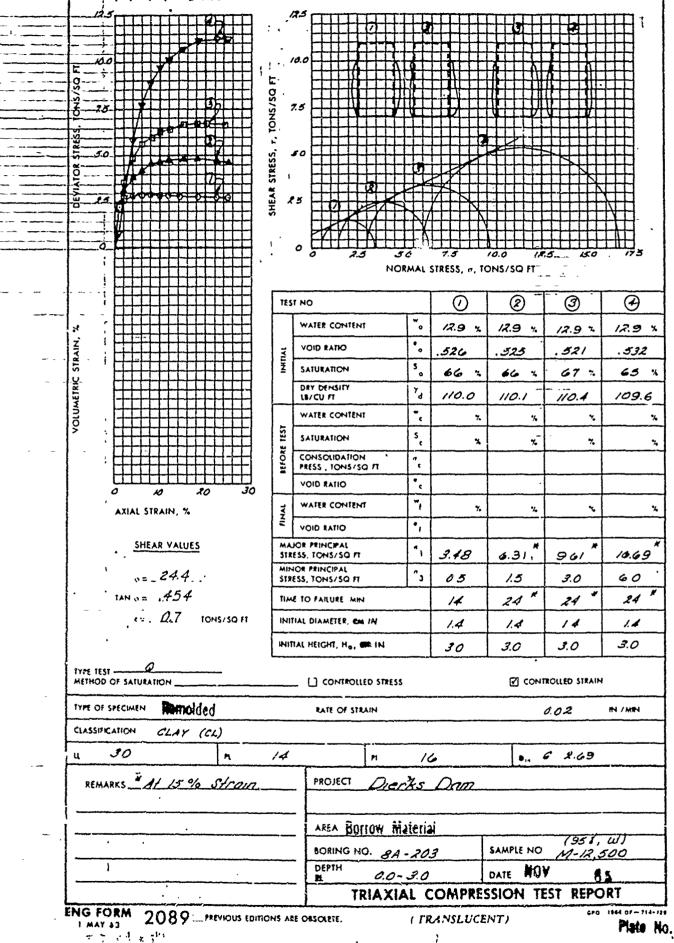
15.0 50 70.0 2.5 · NORMAL STRESS, &, TONS/SQ FT --- (3) $\langle \rangle$ Θ TEST NO ۳. WATER CONTENT 12.8 % 12.8 % 12.8 % 12.8 ζ. VOID RATIO 447 450 442 433 Ŝ. SATURATION 78 % 77 77 . 80 % DRY DENSITY 7 117.2 LB/CU FT 116.0 114.5 115.8 WATER CONTENT 3, SATURATION ٧. ۲ CONSOLIDATION PRESS , TONS/SQ FT • [VOID RATIO WATER CONTENT ٧, AXIAL STRAIN, % OITAR DIOY MAJOR PRINCIPAL STRESS, TONS/SQ FT SHEAR VALUES 5.09 835 12.47 .18.73 ·=_25.2 · 3.0 · STRESS, TONS/SQ FT 05 1.5 6.0 14. = 470 TIME TO FAILURE, MIN 7 23 25 20 = _12__10NS/SO FT INITIAL DIAMETER, SM IN 1.4 1.1 1.4 1.4 MITTAL HEIGHT, Ha, 👊 IN 3.0 3.0 3.0 3.0 0 TYPE TEST -METHOD OF SATURATION CONTROLLED STRESS CONTROLLED STRAIN heblomen TYPE OF SPECIMEN RATE OF STRAIN IN / MIN 0.02 CLASSIFICATION CLAY. (CL) 30 14 u s. 6 2.69 16 REMARKS # At 15% Strain PROJECT Dierks Dam AREA BOTTOW Material (1, W) SORING NO. SAMPLE NO. 81-203 M-12,500 DEPTH 65 00-30 DATE MOA TRIAXIAL COMPRESSION TEST REPORT ENG FORM 2089 MEVIOUS EDITIONS ARE OSSOLETE. 4PQ 1964 0P-714-729 (TRANSI.UCENT) E O YAM I

10 (4)

STUNK HAMAN 5467

SWDGL_Report 240: r, TONS/SQ 10.0 NORMAL STRESS, o, TONS/SQ FT TEST NO 0 2 3 4 WATER CONTENT 15.8 % 15.9 % 15.9 % 15.7 VOID RATIO .525 .522 .515 :528 5 SATURATION 81 82 × 83 % 80 DRY DENSITY LB/CU FT 110.1 110.3 1109 1099 WATER CONTENT \$ SATURATION % CONSOLIDATION ٠, PRESS, TONS/SQ FT YOID RATIO 10 ٦, WATER CONTENT AXIAE STRAIN, % VOID RATIO SHEAR VALUES MAJOR PRINCIPAL ٥, 4.20 * 6.92 × 2.57 * STRESS, TONS/SQ FT 1078 MINOR PRINCIPAL ·=_//.4_ ... STRESS, TONS/5Q FT 0.5 1.5 3.0 60 TAN 0= .. 202 ... 29 ***** 29 * 29 * TIME TO PARURE, MIN 29 c= O.T . TONS SO FT INITIAL DIAMETER, COL IN 1.4 1.4 1.4 1.4 INITIAL HEIGHT, Ho, 🗪 IN 3.0 30 30 3.0 CONTROLLED STRESS T CONTROLLED STRAIN TYPE OF SPECIMENT REMOVED RATE OF STRAIN 0.015 IN./MIN CLASSIFICATION CLAY (CL) 30 11 M 14. . 5 2.69 REMARKS At 15 % Strain PROJECT Dierks Dam AREA BOTTON MATERIAL SAMPLE NO. M-12, 500 BORING NO. 84-203 DEPTH 0.0 - 3.0 DATE NOV 65 TRIAXIAL COMPRESSION TEST REPORT

Plate No >0



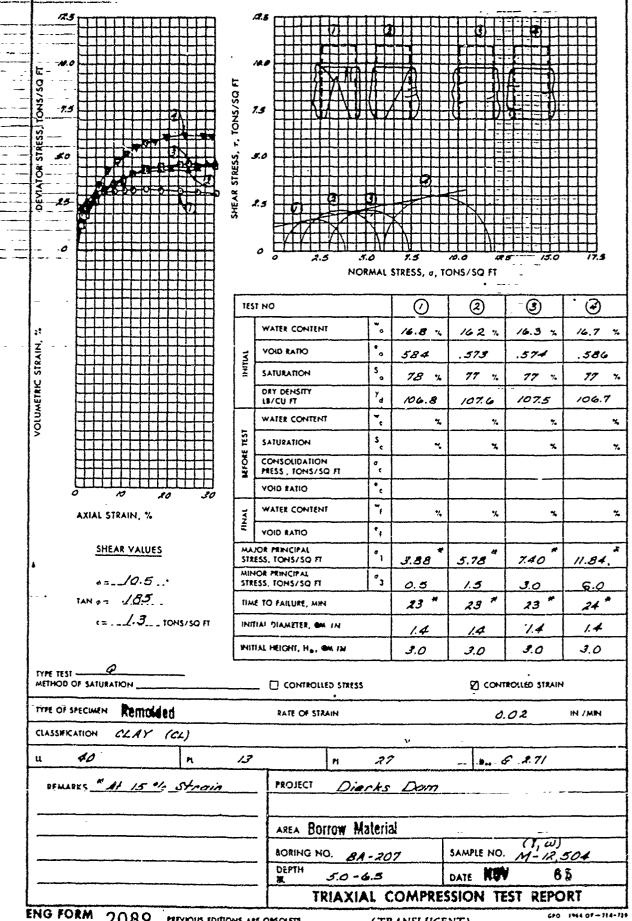
37 --

Plate No.21

たいしょ SWDGL Report 9467 10.0 NORMAL STRESS, #, TONS/SQ FT (I)(2) (3) **(4)** TEST NO WATER CONTENT 10.2 % 10.2 %. 10.1 % 10.3 % STRAIN, VOID RATIO .535 .527 531 540 SATURATION 51 % 52 % *52 %* 51 OLUMETRIC DRY DENSITY 109.4 109.7 110.0 109.1 WATER CONTENT 5, SATURATION % CONSOLIDATION PRESS , TONS/SQ IT VOID RATIO ۳, WATER CONTENT % AXIAL STRAIN, % VOID BATIO SHEAR VALUES MAJOR PRINCIPAL 20.83 8.16 * 4.50 13 19 STRESS, TONS/SQ FT MINOR PRINCIPAL STRESS, TONS/SQ FT v= . 29.8 . · 0.5 1.5 3.0 6.0 TAN 0= . 573_ -45 * 50* TIME TO FARURE, MIN 39 9 €=_. 0:9. _TONS/SQ FT INITIAL DIAMETER, OR IN 1.4 1.4 14 1.4 INITIAL HEIGHT, Ho, 🗪 IN 3.0 3.0 3.0 3.0 TYPE TEST -METHOD OF SATURATION O CONTROLLED STRAIN CONTROLLED STRESS TYPE OF SPECIMEN Remolded RATE OF STRAIN IN /MIN 001 CLASSIFICATION CLAY (CL) 30 14 16 2.49 " At 15 % Strain PROJECT Dierks Dom REMARKS AREA BOSTOW Material SAMPLE NO. M-12,500 BORING NO. 8A - 203 DEPTH DATE NOV 0.0-3.0 TRIAXIAL COMPRESSION TEST REPORT

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SWOGL Report 9467 أرجو لأهبن r, TONS/SQ 0.5 NORMAL STRESS, a, TONS/SQ FT TEST NO. 0 **②** -(3) 4 WATER CONTENT 19.9 % 19.5 % 19.4 % 19.7 % VOID RATIO 678 .675 676 :071 SATURATION 79 78 % 78 × 80 VOLUMETRIC DRY DENSITY 101.2 100.8 101.0 101.0 WATER CONTENT SATURATION % ٧, CONSOLIDATION PRESS, TONS/SQ FT VOID RATIO ۳, WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO MAJOR PRINCIPAL SHEAR VALUES 481 STRESS, TONS/SQ FT 1.88 3.10 8.01 MINOR PRINCIPAL o= _3./ ... STRESS, TONS/SQ FT 0.5 1.5 3.0 6.0 TAN +=: _054 TIME TO FARURE, MIN 24 23 25 24 (=. 0.6__ TONS/SO PT INITIAL DIAMETER, COL IN 1.4 1.4 1.4 1.4 INITIAL HEIGHT, Ha. WA IN 3.0 3.0 3.0 3.0 TYPE TEST . T CONTROLLED STRAIN METHOD OF SATURATION COMPROLLED STRESS TYPE OF SPECIMEN Remoided RATE OF STRAIN IN /MIN 0.02 CLAY (CL) CLASSIFICATION 10 13 27 6 2.71 REMARKS * At 15 % Strain PROJECT Qiacás_ AREA SCITCH Malerial (958, W+3) BORING NO. SAMPLE NO. 84-207 DEFTH 5.0-6.5 DATE NO TRIAXIAL COMPRESSION TEST REPORT ENG FORM GPO 1944 09-714-729 2089 MEVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

Plate No. 24

SWDGL Report 9467 2.5 NORMAL STRESS, a, TONS/SQ FT \bigcirc 2 4 TEST NO. · -(3) WATER CONTENT 16.0 % -16.7 % 16.2 % 16.6 % ٠, VOID RATIO 670 .659 .667 679 SATURATION 67 % 65 % 68 % 66 DRY DENSITY LB/CU FT 101.3 102.0 100.7 101.5 WATER CONTENT TES1 SATURATION ٧. ٧. ٠, CONSOLIDATION PRESS , TONS/SO FT VOID RATIO WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO SHEAR VALUES STRESS, TONS/SQ FT 2.88 7.74 15.09 MINOR PRINCIPAL ·= 22.3 · STRESS, TONS/SQ FT 0.5 6.0 1.5 3.0 TAN 0 = . 410 _ TIME TO FAILURE, MIN 23 23 22 23 Q.6. TONS/SO FT INDIAL DIAMETER, ON IN 14 1.4 14 14 INITIAL HEIGHT, Ho, CM IN 5.0 30 3.0 3.0 METHOD OF SATURATION T CONTROLLED STRAIN CONTROLLED STRESS TYPE OF SPECIMEN IN /MIH Remolded RATE OF STRAIN . 002 CLASSIFICATION CLAY (CL) 40 6 2.71 13 . 27 REMARKS # At 15 % Strain PROJECT Dierks Dam AREA BOTTOW MAL

> BORING NO. DEPTH

5.0-65

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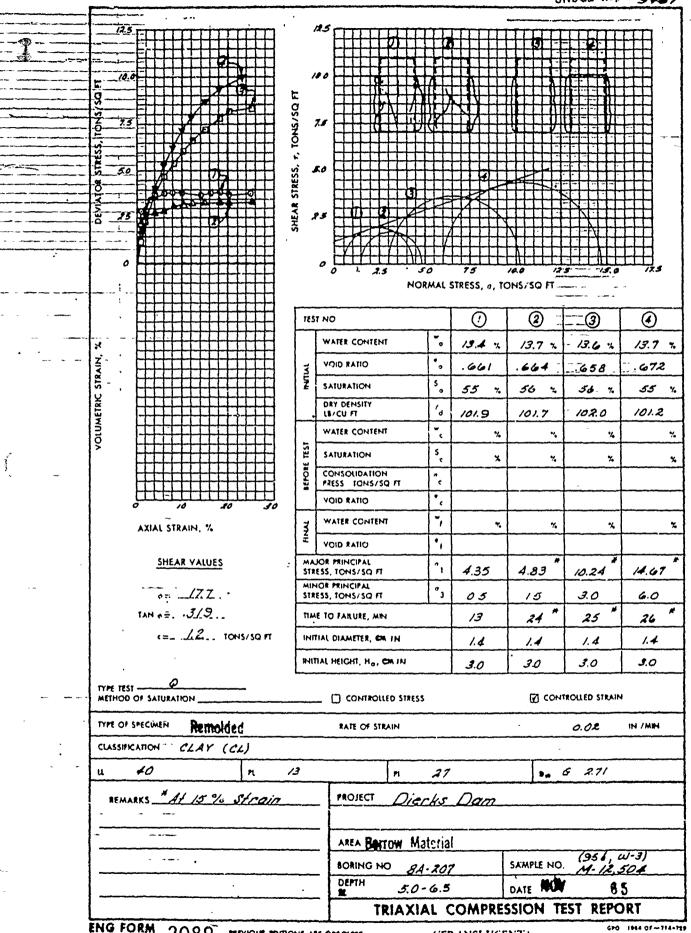
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SAMPLE NO.

DATE TRIAXIAL COMPRESSION TEST REPORT

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(TRANSLUCENT)

Plate No. 26

SWDGL Report 9469 12.5 2.5 5.0 7.5 10.0 17.5 0 Normal Stress, o , tons/sq ft Test No. z Water content 11.2 \$ 11.4% 11.2 \$ Void ratio 450 154 452 67 \$ 66 \$ Saturation s, Dry density lb/cu ft ď 115.4 115.1 115.3 Water content \$ Saturation Consolidation press., tons/sq ft σc Void ratio ec 20 10 Water content ٧ſ Axial Strain, \$ Void ratio Major principal Shear Values tress, tons/sq ft σι 2.98 Minor principal 0 = 25.4_° stress, tons/sq ft 0.5 3.C 6.0 tan # = .475 Time to failure, min 18 27 c = 0.6 tons/sq ft Initial diameter, amin. 1.4 1,4 Initial height, Ho, was. 3.0 3.0 3.0 30 Type Test Controlled Strain Method of Saturation Confrolled Stress Type of specimen in./min Rate of strain Remolded 0.02 (Nomina Classification sandy (ML) ΡI Remarks At 15 % strain Project DIERKS DAM Composite of samples Borrow Material Area Sample No. M-12 50 8 C *** Boring No. 60C Depth 88 Date FEB TRIAXIAL COMPRESSION TEST REPORT ENG FORM 2089

1 9428

SWDGL Report 9469 Stress, Normal Stress, o , tons/sq ft Ø 0 3 **4** Test No. Water content Volumetric Strain 140 13.8 \$ Void ratio e_o Saturation So Dry density lb/cu ft 7d Water content \$ ٧c Saturation % σ̄c Consolidation press., tons/sq Void ratio ec Water content Υf Axial Strain, \$ Void ratio Major principal Shear Values stress, tons/sq ft Minor principal 201 0 = <u>14.5</u> stress, tons/sq ft tan \$ = .258 Time to failure, min c = _0.5 __tons/sq ft Initial diameter, -14 Initial height, Ho, 3.0 3.0 a Type Test_ Method of Saturation _ Controlled Stress Controlled Strain Type of specimen Remolded Rate of strain in./min 0.02 Classification T. sandy (ML) D G 2.68 PL LL ΡI Remarks At 15 % strain Project Dierks Dam Area Borrow Material 7958,W+3) Sample No. 17-12,508 C

ENG FORM 2049

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Boring No. 6DC-165

86

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TRIAXIAL COMPRESSION TEST REPORT

SWDGL Report 9812 SHEAR STRESS, +, TONS/SQ FT 0 5.0 7.5 10.0 NORMAL STRESS, o, TONS/SQ FT ① 2 ④ TEST NO 3 WATER CONTENT 14.7% 14.8% YOLUMETRIC STRAIN, VOID RATIO 480 SATURATION 82 % 83% 82 % DRY DENSITY 112.9 113.1 13.2 WATER CONTENT SATURATION ٧. ٧, CONSOLIDATION PRESS., TONS/SQ FT VOID RATIO WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO MAJOR PRINCIPAL STRESS, TONS/SQ FT SHEAR VALUES 3,88 3.87 MINOR PRINCIPAL 0=_14.L_ .. STRESS TONS/SQ FT 0,5 3.0 6.0 TAN A= .251 TIME TO FAILURE, MIN 23 20 22 (=._/.2___10NS/SQ FT INITIAL DIAMETER, CM IN 1.4 HITIAL HEIGHT, Ho, GHE IT 3.0 3.0 30 3.0 Q TYPE TEST . CONTROLLED STRAIN METHOD OF SATURATION CONTROLLED STRESS IN /MIN TYPE OF SPECIMEN Remolded RATE OF STRAIN 0.02 CLASSIFICATION SAND, clayey (Sc) 31 13 G = 2.68 18 PROJECT Dierks Dam REMARKS. AREA BULLOW Militerial (r,w) SAMPLE NO. M-17.973 BORING NO. 104-20-169 DEPTH DATE JAL 1.0 - 3,3 TRIAXIAL COMPRESSION TEST REPORT 6FO 1944 OF - 714-729 ENG FORM

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Plate No. 3

	Shugt report 4813									
	100 100 100 100 100 100 100 100 100 100	Shear Stress, v. tons/sq ft.	0 25 5.				5	17.5		
-			Norm	al S	tress, σ	tons/sq	ft -	-		
		Test	No.		0	2	3	4		
	a	! . ├─	ater content	w _o	119 %	11.9 \$	118 \$	11.8 \$		
	Strain	1 #		e _o	551	.564	548	.561		
	H H H H H H H H H H H H H H H H H H H	1 44 1	aturation ry density	S _o	58 \$	57 \$	58 ^{\$}	56 7		
	Volumetric	1	b/cu ft	₹d	107.8	1071	1081	107.1		
		ا ۋا		₩ _C	*	*	\$			
<i>‡</i>		ا با	aturation onsolidation	s _c	*	*	*			
년 경 기		1 2 12	ress., tons/sq ft oid ratio							
	0 10 20 30		ater content	e _c	*	7	\$	**		
	Axial Strain, 🖇	inal A	oid ratio	e _f			-			
	Shear Values	Major principal stress, tons/sq ft			4.76	7.93	11.96	16.75		
	ÿ = <u>21.8</u> °	Minor	principal s, tons/sq ft	σ ₁	0.5	1.5	3.0	6.0		
	tan Ø =400	Time to failure, min			7	21	23	24		
	c = <u>1.2</u> tons/sq ft	Initi	al diameter, om	n	14	1.4	1.4	1.4		
		Init	al height, Ho, c	min	3.0	3.0	30	3.0		
	Type Test Q Method of Saturation Controlled Stress V Controlled Str									
	Type of specimen Remulder, Rate of strain 0.02 in./m							in./min		
•	. Classification SAND, Clayey (SC)									
	LL 31 PL	PI 18			² 20 G = 268					
	Remarks Major principal stress	Project Dierks Dam								
	are based on man deviator str									
	at or below 15% strain.		Area Burrow Material (95 %, W-3)							
		Boring No. 10 A-2C-1G9 Sample No. M-17,473								
		E 1.0 - 3.3 Date JUL 1/3								
* ••			TRIAXIAL COMPRESSION TEST REPORT							

(TRANSLUCENT)

o 3426 Plate No. 4

SWDGL River 9812 r, TONS/SQ 7.5 NORMAL STRESS, o, TONS/SQ FT. \bigcirc (2) (3) **(4)** TEST NO WATER CONTENT 14.7 % 14.7 % STRAIN, • ູົ VOID RATIO 562 538 561 SATURATION 70 73 % DRY DENSITY 107.6 108.7 WATER CONTENT TEST **5** c SATURATION ٧. ٧, % % CONSOLIDATION PRESS , TONS/SQ FT VOID RATIO ٠, WATER CONTENT AXIAL STRAIN, % ٠, VOID RATIO SHEAR VALUES MAJOR PRINCIPAL "1 STRESS, TONS/SQ FT 2,55 5,30 12.17 7.69 MINOR PRINCIPAL °, 4=_15.8 _· STRESS, TONS/SO FT 0.5 6,0 30 TAN += . 283_ . TIME TO FAILURE, MIN 23 23 23 (=_ 0.6 __TONS/SQ FT INITIAL CHAMETER, ES in 1.4 INITIAL HEICHT, Ho, CALIN 3.0 3.0 3.0 3.0 TYPE TEST CONTROLLED STRAIN METHOD OF SATURATION CONTROLLED STRESS. TYPE OF SPECIMEN Remolder RATE OF STRAIN 0.02 pl./MN CLASSIFICATION SAND Clayey (SC) 13 18 ™ G = 2.68 31 PROJECT Dierns Dám REMARKS Major principal streetes are based on mar deviator stresses AREA BOSTOW Material at or below 15 % strain SAMPLE NO. (96 1.W) BORING NO. 104-2C-169 DEPTH DATE 3111 G: 1.0 - 3.3 TRIAXIAL COMPRESSION TEST REPORT GPO 1984 07 - 214-729 ENG FORM

SWDGL Report 9812 Normal Stress, o , tons/sq ft (I)(2) (\mathfrak{Z}) Test No. (4) Water content Strain, 177 \$ 17.6 \$ Void ratio 548 545 Saturation So Volumetric 87 86 85 Dry density lb/cu ft 7a 1081 1078 1084 107.4 Water content Saturation S Consolidation press., tons/sq ft o_c Void ratio ec Water content Axial Strain, & Void ratio Shear Values Major principal stress, tons/sq ft Minor principal 2.29 352 5.40 8.43 ø - 32 15 stress, tons/sq ft 0.5 3.0 60 tan \$ = ___055 Time to failure, min 24 23 23 c = 0.8 tons/sq ft Initial diameter, camin 1.4 14 14 Initial height, Ho, mem 3.0 30 30 Type Test Method of Saturation Controlled Stress V Controlled Strain Type of specimen Remorted Rate of strain in./min 0.02 Classification clayey (SC) SAND 31 PL Exa G = 2.68 Remarks Major principal stresses are Project Dierks bused on max deviator stresses at or Area Rorrow Material helow 15 % Strain. (95 1.W+3) Boring No. 10A-2C-169 M-17.973 Sample No. JUL 66 TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089

Plate No K

-SWUGL-KEPOTT 9891 Normal Stress, o, T/sq ft **②** 3 **(4)** \bigcirc Test No. 10.2 "% Water content 10.1 10.2 % 10.1 \$ ¥o Void ratio e_o .416 422 407 426 Saturation So 65 65 67 63 Dry density, $\gamma_{\rm d}$ 117.7 117.2 118.5 1169 lb/cu ft Water content Void ratio ec Sc Saturation \$ Final back presuo sure, T/sq ft Axial Strain, \$ Water content ≰, É Void ratio Shear Strength Parameters er Minor principal σ₃ · =. 366 0.5 1.5 30 6.0 stress, T/sq ft Max deviator stress, T/sq ft (01-03) max tan * = .743 2.50 5.82 11.83 18 78 c = 0.3 T/sq rt Time to failure, min 6 4 Rate of strain, percent/min 0.4 0.6 04 0.3 Method of saturation _ Ult deviator stress. T/sq ft (01-03)ult Controlled stress Initial diameter, in. 14 1.4. 1.4 1.4 Controlled strain Initial height, in. 30 3.0 3.0 3.0 Type of test Q Type of specimen Remolded Classification SAND, silty (SM) NP -NP MP 2.67 Project DIERKS DAM Remarks _ Area Borrow Material Boring No. TP-488 Sample No. M-18548 Depth Date OUT в 0.0-80 TRIAXIAL COMPRESSION TEST REPORT

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SWUGL REPORT STUWE 30 10 20 0 H s - 15 10 Normal Stress, o, T/sq ft (2) (3) (4)(I) Test No. Water content 10.2 \$ 10.0% ¥o 10.24 -10:3-% Void ratio e_o .49/ 492 491 489 Saturation so 55 \$ 55 % 56 % 54 % Dry density, $\gamma_{\rm d}$ 111.8 111.7--///.8 111.9 lb/cu ft % Water content w.c \$ Shear Void ratio ec Saturation \$_ % 1.5 Final back pres-10 uo sure, T/sq ft Axial Strain, % Water content ٧ſ Void ratio Thear Strength Parameters er Minor principal = <u>34.2</u> ° 0.5 1.5 3.0 6.0 stress, T/sq ft Max deviator stress. T/sq ft (01-03) max 1.79 4.81 15.89 tan • = _,680 8.45 Time to failure, min tr · c = ______ T/sq ft 6 5 9 11 Rate of strain, percent/min 0.3 0.6 0.7 0.6 Method of saturation _ Ult deviator stress. T/sq ft (01-03) ult Controlled stress Initial dismeter, in. Do 1.4 1.4 1.4 1.4 Controlled strain Initial height, in. H_o 3.0 3.0 3.0 3.0 Type of test Q Remnided Type of specimen Classification SAND, silty (SM) FL NP LL NP 2.67 NP Project DIERKS DAM Remarks Area Borrow Material Sample No. M-18,548 Boring No. TP-488 Depth บเ. Date 0.0-8.0 TRIAXIAL COMPRESSION TEST REPORT 10

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TRIAXIAL COMPRESSION TEST REPORT

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SWDGL Report 9891 Normal Stress, . o, T/sq ft 0 € Test No. 9.7 \$ Water content ¥0 9.6 9.6 \$ 98% e_o Void ratio 488 486 498 487 Saturation So 53 % 54 % 53 52 Dry density, 7_d 112.9 113.0 7/30 112.0 lb/cu ft Water content * % Void ratio ec Saturation \$ Final back presno. sure, T/sq ft Axial Strain, \$ % Water content ٧ſ Shear Strength Parameters Void ratio ¢ſ Minor principal stress, T/sq ft • = 309 ° 0.5 1.5 3.0 6.0 Max deviator stress. T/sq ft (01-03)max 4.05 tan • = 599 7.27 10.34 15.65 Time to failure, min 4 13 21 25 c = 09 T/sq ft Rate of strain, percent/min 0.6 0.3 0.6 0.6 Method of saturation _ Ult deviator stress, T/sq ft (01-03)ult Controlled stress Initial diameter, in. p_{o} 1.4 1.4 1.4 1.4 Controlled strain Initial height, in. 3.0 3.0 3.0 3.0 Type of test o Type of specimen Remolded Composite of minus No. 10 fractions of SWD Sample M-18564, -18565, and -18567. Classification CLAY, sandy (CL) PL 12 Gs 2.69 25 PI 13 Project Remarks Major principal stresses DIERKS DAM are based on maximum Area deviator stresses at or Borrow Material Sample No. M-18,564-C below 15% strain. Boring No. TP-491 & TP-492 Depth 13. 13. Date 2.0 -10.1 TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089 (EM 1110-2-1902)

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1080 hoose 1904 Normal Stress, o, T/sq ft (I)0 3 4 Test No. 15.6 \$ 15.4% 15.5% Water content 15.5% W_O Void ratio e_o .488 48.7 486 490 So 86 \$ 85 \$ Saturation 85 \$ 86 % Dry density, 7_d 772.7 112.9 112.79 773.0 1b/cu ft Water content w_c \$ % Void ratio ec \$ Saturation \$ 4 Sc Final back presuo sure, T/sq ft Axial Strain, \$ Water content 4 ٠ 🐒 \$ ٧ſ Shear Strength Parameters Void ratio er Minor principal stress, T/sq ft 0.5 1.5 3.0 6.0 • = _5A Max deviator stress, T/sq ft (01-03) max tan • = _,094 1.68 2.01 2.82 2.62 Time to failure, min c = 0.7 T/sq ft 24 21 23 22 Rate of strain, percent/min 0.6 0.7 0.6 0.7 Method of saturation _ Ult deviator stress. T/sq ft (01-03)ult Controlled stress Initial diameter, in. 1.4 14 1:4 1.4 Controlled strain Initial beight, in. 3.0 3.0 3.0 5.0 Type of test Q Type of specimen Remold Composite of minus No:10 fractions of SND Samples M-18 564 -18 565; and -18 567 Classification CLAY, sandy (CL) 15 12 13 2.69 Project DIEAKS DAM Remarks Major principal stresses are based on maximum Airea deviator stresses as or Borrow Material Boring No. TP-491 + TP-495 Sample No. M-18,564 C below 15% strain Depth 2.0-10.1 Date OUT TRIAXIAL COMPRESSION TEST REPORT

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Plate No 16

APPENDIX E

COMPACTED MATERIALS

SECTION 2. R TRIAXIAL TESTS

15 NORMAL STRESS, o, TONS/SQ FT TEST NO 0 ③ WATER CONTENT 11.8 % 11.9 , VOID RATIO 430 438 SATURATION DRY DENSITY 7 LB/CU FT 115.8 б WATER CONTEST 16.64 5, SATURATION 100% 100% CONSOLIDATION PRESS , TONS/SQ FT 2.95 6.00 VOID RATIO 439 445 444 ۳, WATER CONTENT AY! STRAIN, % 16.4% 16.6% VOID RATIO At ٠, 439 445 444 MAJOR PRINCIPAL STRESS, TONS/SQ FT AR VALUES 19.26 35.33 MINOR PRINCIPAL o.. 38.6 STRESS, TONS/SQ FT 6.00 TAN n= .796 TIME TO FAILURE, MIN 435 265 460 c= 2.2 TONS/SO FT INITIAL DIAMETER, CHIN. 14 INITIAL HEIGHT, Ho, XIMIN. 3.0 3.0 METHOD OF SATURATION BACK DIESSLIFE __ [] CONTROLLED STRESS CONTROLLED STRAIN TYPE OF SPECIMEN Remolded RATE OF STRAIN IN /MIN 0.001 (Nominal) CLASSIFICATION AV, sandy (CL-ML G 2.67 21 6 MOJEC! Dierks Dam REMARKS_ AREA (8, W) SAMPLE NO. My. BORING NO BA-200 12.496 DEPTH DATE NOV 85 0.0-3.0 TRIAXIAL COMPPESSION TEST REPORT ENG FORM GPO 1944 07-714-728 2089^{-} previous editions are obsolete (TRANSLUCENT)

SWOGL Report 9467 Stricte. The NORMAL STRESS, σ, TONS/SQ FT" TEST NO 2 WATER CONTENT 15.0 % 151 VOID RATIO 492 SATURATION 82 % 79 DRY DENSITY 111.7 WATER CONTENT 1251 SATURATION CONSOLIDATION PRESS, TONS/SQ FT 6.05 VOID RATIO WATER CONTENT 19.7 % AXIAL STRAIN, % VOID RATIO At MAJOR PRINCIPAL SHEAR YALVES STRESS, TONS/SQ FT 7.28 MINOR PRINCIPAL , 29.0 . STRESS, TONS/SQ FT 1.55 6.05 TAN .. = 554 TIME TO FAILURE MIN 440 3.90 1 0.6 _ TONS/SQ FT INITIAL HEIGHT Ho. SMINL METHOD OF SATURATION Back prossure [] CONTROLLED STRESS - 🛛 CONTROLLED STRAIN TYPE OF SPECIMEN Remolded RATE OF STRAIN M /MM See romarks CLASSIFICATION CLAY sandy (CL-ML) n # G 2.67 PROJECT Diecks Dam AREA SAMPLE NO. M-12, 496 MORING NO. 84-200 a Dalin Imia DEPTH YUN DATE TRIAXIAL COMPRESSION TEST REPORT ENG FORM 90 1944 0F-714-729 2089 PREVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

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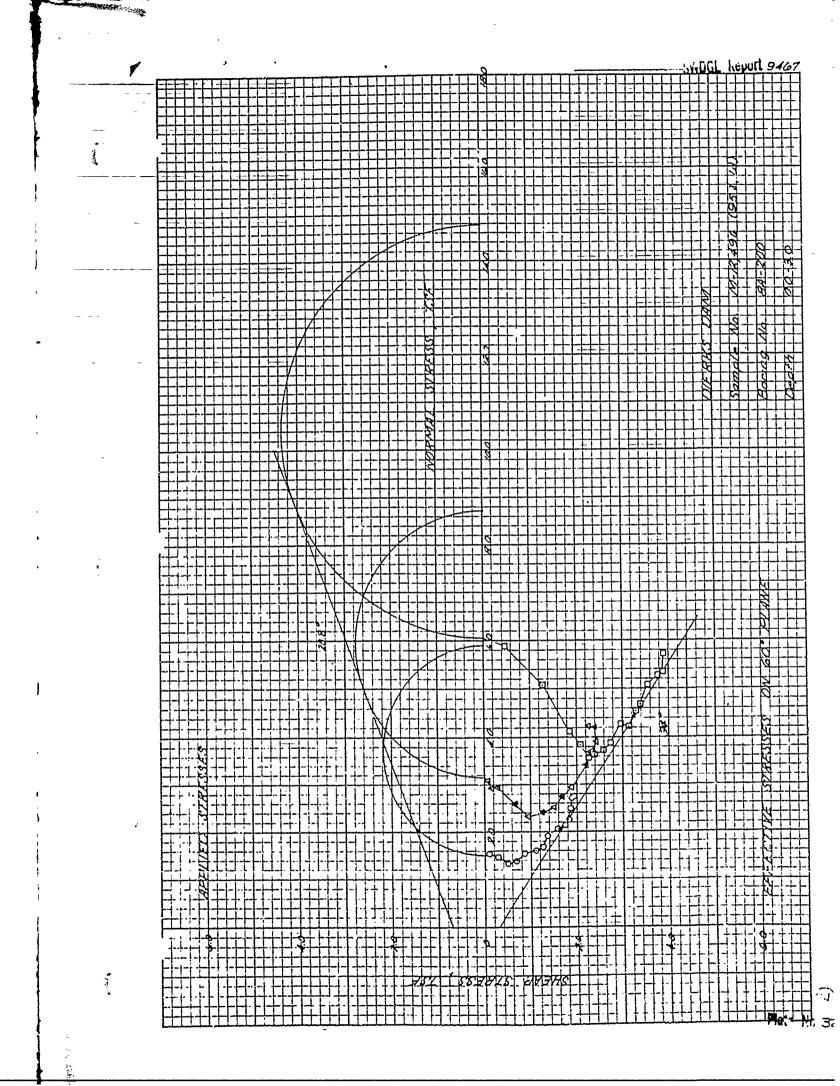
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WITCL Report 9467 NORMAL STRESS, a, TONS/SQ FT (2) TEST NO $\langle \gamma \rangle$ · (3) WATER CONTENT 11.6 % Pressure 11.8 % 11.8 % VOID BATIO .504 498 .512 SATURATION 61 % 62 % 63 % Pore DRY DENSITY 110.8 111.4 110.3 WATER CONTENT 18.6 % 195 % 18.4 % Induced SATURATION 100 % 100 % 100 % CONSOLIDATION PRESS , TONS/SU FT 1.50 3.15 6.05 VOID RATIO .496 .520 .492 WATER CONTENT 18.6 % 19.5 % 18.4 % AXIAL STRAIN, % VOID RATIO (At Failure 196 520 .492 MAJOR PRINCIPAL STRESS TONS/SQ FT SHEAR VALUES 5.89 8.70 14.77 MINOR PRINCIPAL 0= 20.8 · STRESS, TONS/SQ FT 1.50 3.1.5 6.05 TAN es .38Q TIME TO FARURE, MIN 415 300 360 2.7 TONS/50 FT INITIAL DIAMETER SMINI 1.4 1.4 14 INITIAL HEIGHT, Ho, CM IN 30 3.0 3.0 TYPE TEST . METHOD OF SATURATION BOCK PRESSURE CONTROLLED STRESS CONTROLLED STRAIN TYPE OF SPECIMEN RATE OF STRAIN Remolded 0.001 IN /MIN CLASSIFICATION CLAY, sondy (CL-ML) 21 15 6 on 6 2.57 REMARKS # At 15 % STroid PROJECT Diecks Dan AREA Borrow Malorial (951, W) M-12, 494 BORING NO SAMPLE NO 81-200 DEPTH 65 DATE 0.0 - 3.0 TRIAXIAL COMPRESSION TEST REPORT ENG FORM 2089 MEVIOUS EDITIONS ARE DESCRETE. 470 1344 OF-714-719

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UTITUL NEWLY 9457 r. TONS/SQ NORMAL STRESE, a, TONS/SQ FT-(3) 0 2 TEST NO ٧, WATER CONTENT 3.0 4 4 9/ 3 OITAR DIGY 3. SATURATION 48 % DRY DENSITY 18/CU FT Ó WATER CONTENT 1651 \$, SATURATION 100" PRESS, TONS/SQ FT OITAR GIOV WATER CONTENT ٧, AXIAL STRAIN, % VOID RATIO Feilure SHEAR VALUES STRESS TONS/SO FT MINOR PPINCIPAL 0= 15.0 STRESS, TONS/SQ FT 1.60 3.05 6.00 286 TIME TO FAILURE, MIN 460 0.8 TONS/SQ FT INITIAL DIAMETER CHIN. INITIAL HEIGHT, Ha, CORIN. 3.0 3.4 TYPE TEST METHOD OF SATURATION BACK DECESSURE CONTROLLED STRAIN COMPOULD STRESS TYPE OF SPECIMEN Remolded RATE OF STRAIN IN /MIN remacks CLASSIF'S ATION CLAY sondy (CL-ML) PROJECT Dierks Dam AREÁ 7958, W-2) BORING NO SAMPLE NO. M-12.496 DEPTH DATE NOV TRIAXIAL COMPRESSION TEST REPORT

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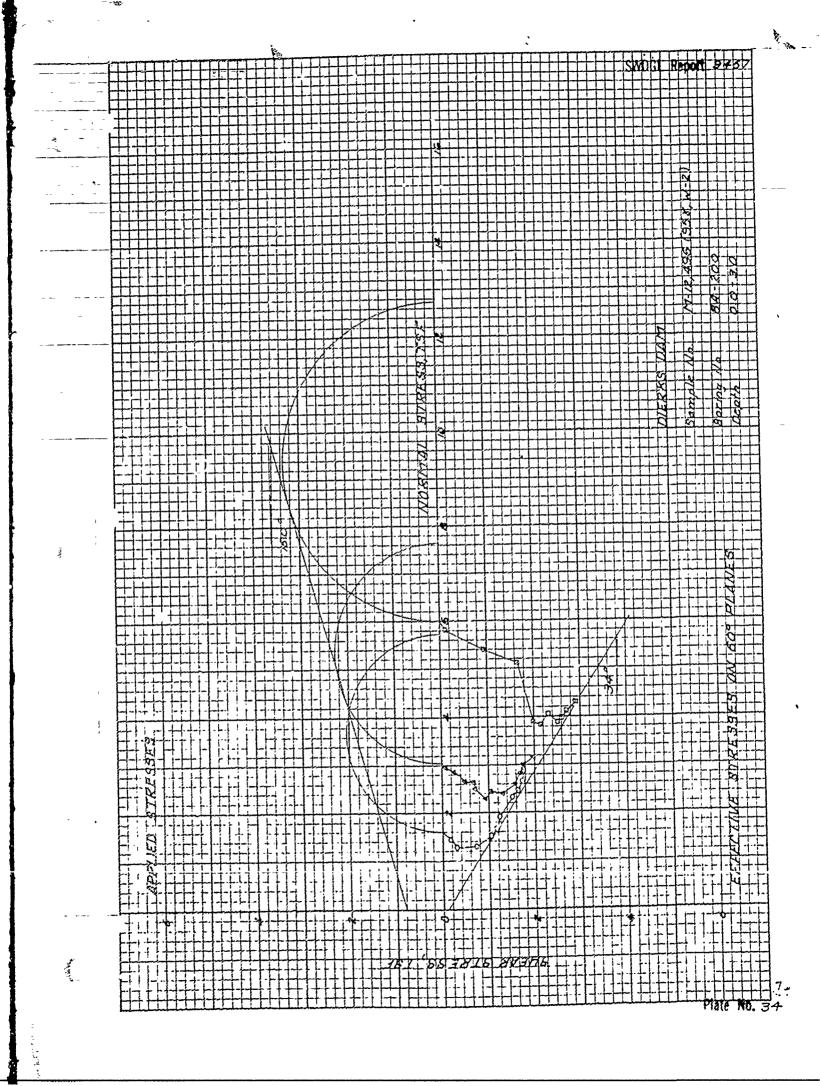
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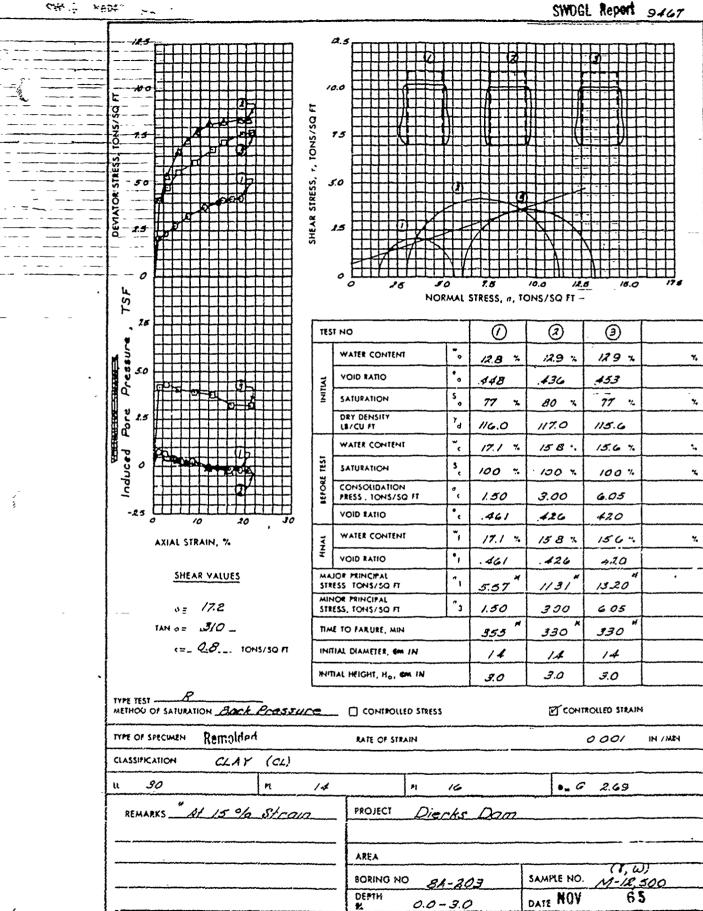
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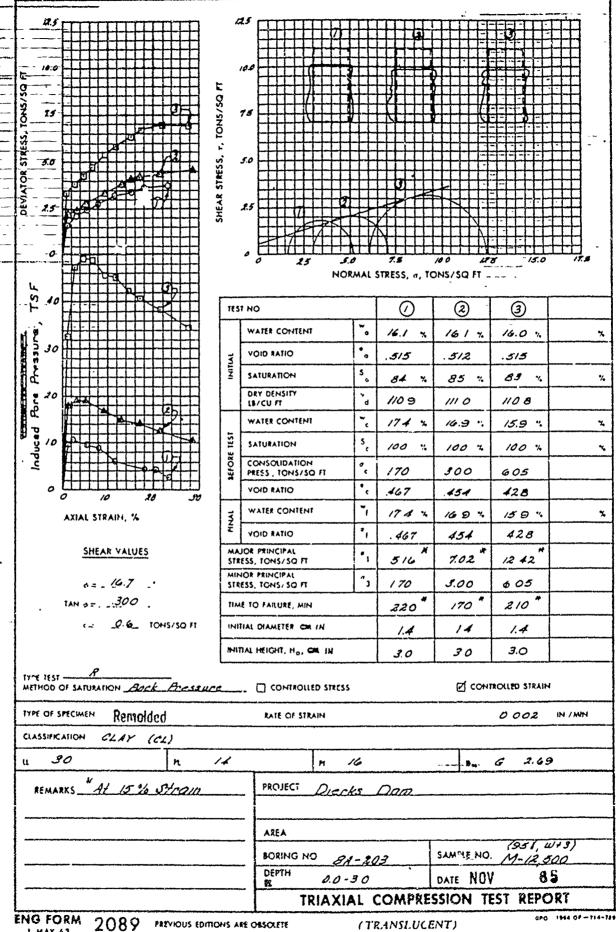
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TRIAXIAL COMPRESSION TEST REPORT

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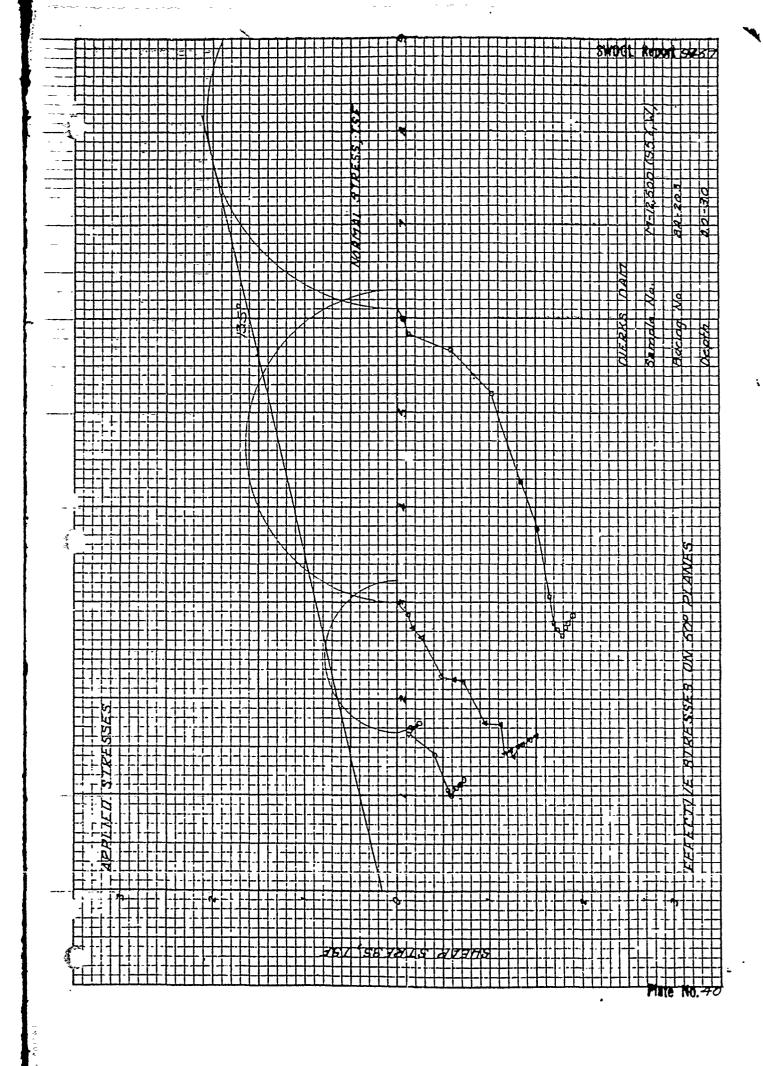
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SWDGL Report 9467 CHICA REPORT ð 75. NORMAL STRESS, a, TONS/SQ FT U TEST NO 3 0 2 WATER CONTENT 13.1 % VOID RATIO SATURATION DRY DENSITY LB/CU FT WATER CONTENT SATURATION CONSOLIDATION PRESS., TONS/SQ FT 3,00 6.10 00 VOID RATIO 575 497 505 WATER CONTENT 18.5% AXIAL STRAIN, % 18.8 % VOID RATIO At 497 505 SHEAR VALUES STRESS, TONS/SQ PT 6.30 MINOR PRINCIPAL <u>. . . /3.5</u> STRESS, TONS/SQ FT-3.00 TAN 3 - . 240 TIME TO FAILURE, MIN 335 245 ... O. 2 . TONS/SQ FT INITIAL DIAMETER, COM NJ. initial height, h_o, dairl METHOD OF SATURATION Brok pressure Controlled STRESS OCONTROLLED STRAIN Remolded TYPE OF SPECIMEN IN./MIN RATE OF STRAIN remorks CLASSIFICATION G 2.63 PROJECT REMARKS Dierks Dam Rate of strain: AREA (95 8,W) SAMPLE NO. M-12, 500 BORING NO. BA-203 DEPTH DATE NOV 0.0 - 5.0 TRIAXIAL COMPRESSION TEST REPORT **ENG FORM** 2089. PREVIOUS EDITIONS ARE OBSOLETE

(TRANSLUCENT)

Mate No. 33



SWDGL Report 9467 7, TONS/SQ SHEAR STRESS, NORMAL STRESS, &, TONS/SQ FT 0 0 3 TEST NO ٣. WATER CONTENT 10.2 % 10-2 3 VOID RATIO INITAL SATURATION DRY DENSITY LB/CU FT WATER CONTENT ced 5, SATURATION 100" CONSOLIDATION PRESS., TONS/SQ FT VOID RATIO WATER CONTENT AXIAL STRAIN, % VOID RATIO AT SHEAR VALUES MAJOR PRINCIPAL STRESS, TONS/SQ FT 3.34 MINOR PRINCIPAL 0= 13.0 _. STRESS, TONS/SQ FT 3.00 1.50 6.00 TAN 0 = . 230 TIME TO FARURE, MIN 195 € . 0.3 _ TONS/SO FT INITIAL DIAMETER, CON IN 1.4 1.4 1.4 MITIAL HEIGHT, H. CHIAL 3.0 3.0 (Z) CONTROLLED STRAIN METHOD OF SATURATION BOCK PRESSURE CONTROLLED STRESS TYPE OF SPECIMEN Remolded RATE OF STRAIN M,/MN See remarks CLASSIFICATION CLAY (CL) ≈ G 2.69 14 16 REMARKS # At 15 % strain PROJECT Dierks AREA 0.001 in/min (958, W-3) SAMPLE NO. 17-12,500 BORING NO. BA-203 0.0005 h./min. DATE NOV TRIAXIAL COMPRESSION TEST REPORT 4PQ 1864 0F-714-721 ENG FORM $2089_{\it c}$ previous editions are obsolete. (TRANSLUCENT)

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Plata No. 41

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SWDGL Report 9467 Shire. Here NORMAL STRESS, o, TONS/SQ FT" 3 TEST NO \bigcirc WATER CONTENT 16.4 % VOID RATIO SATURATION % 78 " DRY DENSITY LB/CU FT WATER CONTENT SATURATION CONSOLIDATION PRESS , TONS/SQ FT 2.95 5.90 VOID EATIO 604 WATER CONTENT 224" AXIAL STRAIN, % VOID RATIO At SHEAR VALUES STRESS, TONS/SQ FT MINOR PRINCIPAL \$=_*L7.2*____ STRESS, TONS/SO FT TAN 3 = .3/0 ... TIME TO FAILURE, MIN 320 c= . O.L __ .TONS/SQ FT INITIAL DIAMETER, CM INITIAL HEIGHT, Ha, CM 3.0 TYPE TEST K
METHOD OF SATURATION Back presisura Controlled Stress * 🛛 CONTROLLED STRAIN IN /MN TYPE OF SPECIMEN Remolded RATE OF STRAIN gee remacks CLASSIFICATION CLAY (CL) =G2.71 27 PROJECT Diarks Dam. REMARKS At 15% strain Rate of strain: 1 & 2, 0,001 in./min. AREA (W,8) SAMPLE NO 17-12,504 BORING NO. 84-207 0.0015 in /min DEPTH DATE NOV 65 5.0 - 7.5 TRIAXIAL COMPRESSION TEST REPORT GPO 1944 07-714-729 ENG FORM 2089- PREVIOUS EDMONS ARE OBSOLETE. (TRANSLUCENT)

Plate No.43

SWOG Report 2787 Semole No. 12 Society W. Bacing Wa Ba-207 APPENDED TO THE PROPERTY OF TH 十十 J.[___ Plate No.44

SWDGL Report 9467. S-1. West r. 15 NORMAL STRESS, o, TONS/SQ FT (2) (3) TEST NO (/)WATER CONTENT 20.1 % 20.2 % 20.0 % VOID RATIO 483 662 .682 SATURATION 80 % 82 % 79 % . % DRY DENSITY 100.6 101.7 100.6 US/CU FT WATER CONTENT 22.1 % 20.1 % 190 % SATURATION 100 % 100 % 100 PRESS , TONS/SO FT 1.65 2.95 6.10 VOID RATIO .598 .544 .516 WATER CONTENT 22.1 % 20.1 % 19.0 % AXIAL STRAIN, % VOID RATIO (At Failure) .598 544 516 SHEAR VALUES STRESS, TONS/SQ FT 2.87 6.00 10.31 MINOR PRINCIPAL e= __[4.6 ... · STRESS, TONS/SO FT 1.65 1.95 6.10 TAN 0= .260 435* TIME TO FAILURE, MIN 315 345 a.o. tons/soft INITIAL DIAMETER. MA IN 1.4 1.4 1.4 INITIAL HEIGHT, Ho, CM IN 3.0 3.0 3.0 METHOD OF SATURATION BOOK Prossure CONTROLLED STRESS TONTIOLLED STRAIN TYPE OF SPECIMEN Remolded RATE OF STRAIN 0.001 CLASSIFICATION CLAY (CL) 10 8- 6 271 ħ 15 27 At 15 % Strain PROJECT REMARKS_ Dierks Iran AREA (958, W+3) BORING NO. SAMPLE NO. M-12,504 81-207

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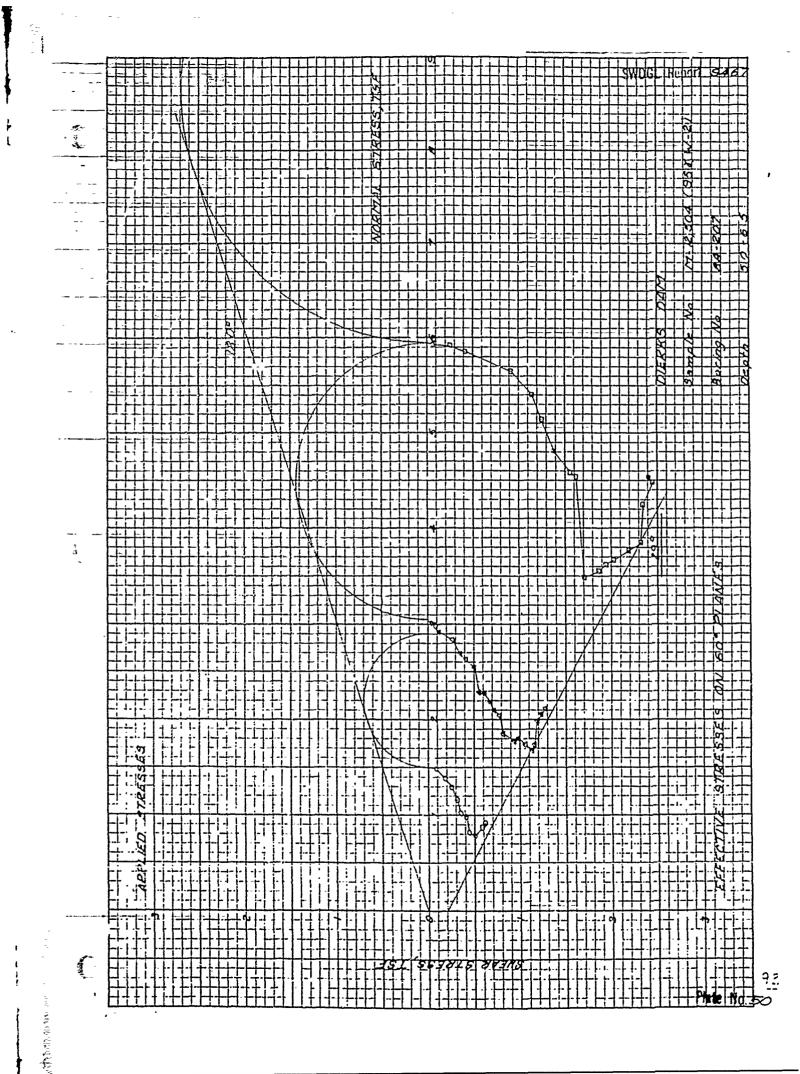
TATE HERE ILL SWDGL Report 9467 SHEAR STRESS, r, TONS/SQ NORMAL STRESS, a, TONS/SQ FT-(2) -_(3) TEST NO WATER CONTENT 16.4 % 16.5 % 16.0% •, VOID RATIO .676 .677 .654 SATURATION 66 × 66 % 66 % DRY DENSITY LB/CU FT 101.0 102.4 100.8 WATER CONTENT 22.3 % 19.4% 18.9 % SATURATION 100 % 100 % 100 % CONSOLIDATION 3.05 5.90 1.60 PRESS, TONS/SQ FT OID RATIO .605 .526 --. 515 WATER CONTENT 22.3 % 19.4 % 18.9 % AXIAL STRAIN, % VOID RATIO (At Failure 526 .513 .605 MAJOR PRINCIPAL SHEAR VALUES 9.73 2.85 5.63 STRESS, TONS/SO FT MINOR PRINCIPAL STRESS, TONS/SQ FT s= .. 13.6. . 3.05 5.90 1.60 335 TIME TO FARURE MIN 430 360 c= 0./ _ TONS/50 FT INITIAL DIAMETER CA IN 14 1.4 1.4 INITIAL HEIGHT, Ho, CA IN 3.0 3.0 3.0 TYPE TEST -METHOD OF SATURATION Back Pressure Controlled STRESS S CONTROLLED STRAIN tire of specimen Remolded RATE OF STRAIN 0.001 IN / MIN CLASSIFICATION CLRY (CL) 27 T.. 6 40 13 2.71 REMARKS At 15 % Stroin PROJECT Dierka Dom AREA (95 Y, W) SAMPLE NO BORING NO. M-12504 BA-207 DEPTH DATE NOV 65 5.0-6.5 TRIAXIAL COMPRESSION TEST REPORT ENG FORM 2089 MEVIOUS EDITIONS ARE OBSOLETE. (TRANSLUCENT)

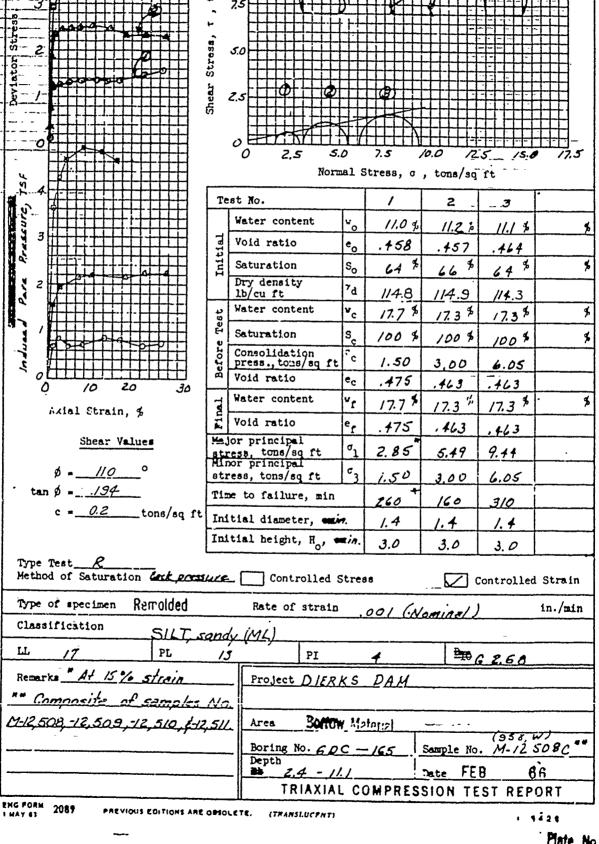
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WDGL Report 9467 田田 11.4 28 David Samoria A H 中节 1. 12 p 計 1-1-1 +: 1-1-1-22 STABILIST 5778 -- 1 . Т. j ++ 1 H + Į. -----1 7-Fiate 140, 48 でのでしても地方

C3" ' SWDGL Report 9457 10.0 SHEAR STRESS, +, TONS/*Q 7SF NORMAL STRESS, σ , TONS/SQ FT $\tilde{\ }$ 0 **(2)** 3 WATER CONTENT 142 % 140 % 142% VOID RATIO SATURATION ٧, 56 DRY DENSITY LB/CU FT 100.6 100.5 WATER CONTENT 947 17.6 TEST SATURATION 100 " 100 % CONSOLIDATION PRESS . TCNS/SQ FT 595 150 3.05 VOID BATIO 628 WATER CONTENT 9.4 % AXIAL STRAIN, % VOID RATIO 528 Eailuca MAJOR PRINCIPAL SHEAR VALUES STRESS, TONS/SQ FT 1134 MINOR PRINCIPAL a: 18.0 STRESS, TONS'SQ FT 5.95 TAN 0 .324 TIME TO FAILURE, MIN 395 375 310 . . O.Q TONS/SQ FT INITIAL DIAMETER, CHIN. 1.4 1.4 14 NITIAL PEIGHT, H_0 , EM IN. 3.0 3.0 3.0 TYPE TEST . METHOD OF SATURATION Mack pressure Controlled STRESS CONTROLLED STRAIN TYPE OF SPECIMEN REMOIDED ZATE OF STRAIN IN./MIN See remarks CLASSIFICATION \$ G 2.71 REMARKS At 15 % strain PROJECT Dierks AREA 0.001 la /mia. (958,W-Z) SAMPLE NO. 17-12, 504 BORING NO. 84-207 Test "3, 0.0015 in/mia DEPTH DATE NOV 5.0 - 6.5 4 TRIAXIAL COMPRESSION TEST REPORT GPO 1244 07-714-719 2089. MEVIOUS EDITIONS ARE CISSOLETE. (TRANSLUCENT) I MAY 63

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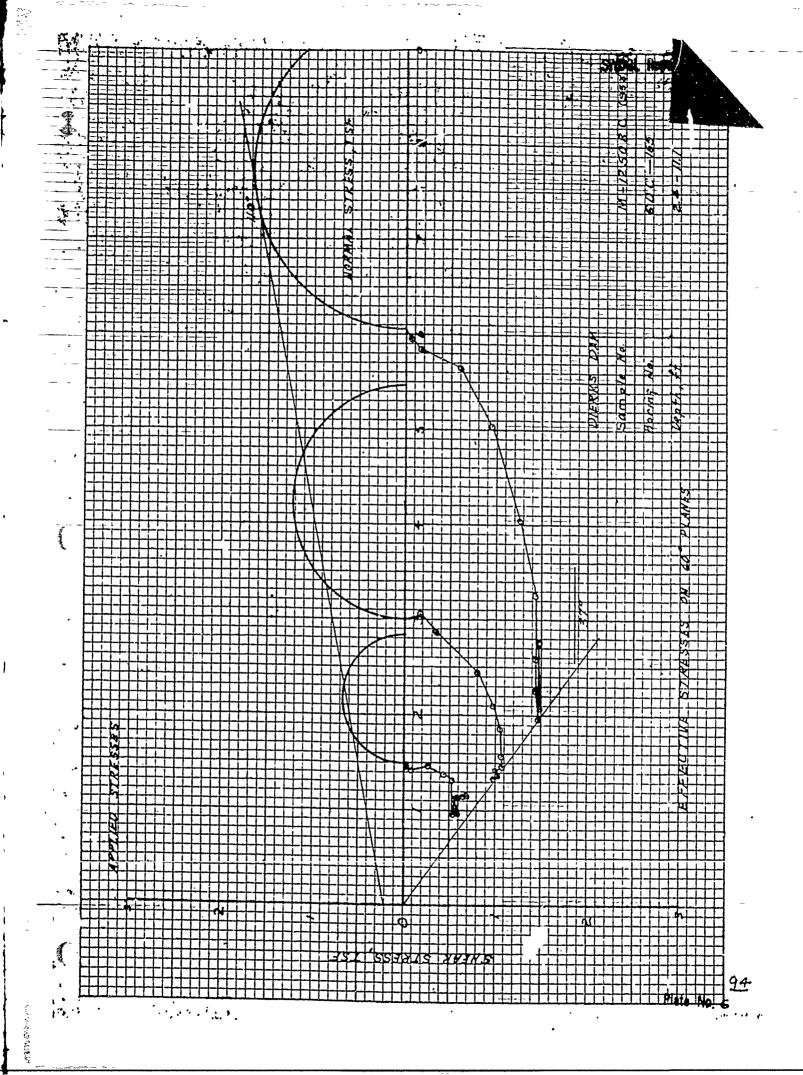




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SWDGL Report 9469 Stresa, 50 17.5 Normal Stress, σ , tons/sq ft 0 Test No. @ ③ Water content 13.9 % 14.3 % Void ratio 457 461 Saturation Dry density ď lb/cu ft 114.5 114.7 Water content 15.5 % 150 3 16.0 Saturation 100 \$ 100 100 \$ Consolidation press., tons/sq ft 150 295 6.00 Void ratio ec 430 402 407 10 Water content 160 3 150 " <u>15</u>5 3 Axial Strain, \$ Void ratio 430 407 402 Major principal Shear Values stress, tons/sq ft Minor principal 490 * 727 12.56 ø - <u>169</u> stress, tons/sq ft 150 295 6.00 tan 0 = 303 Time to failure, min 255 * 350 380 c = 0.6 tons/sq ft Initial diameter, 🗪 in 14 1.4 Initial height, Ho, mm/n 3.0 3.0 3.0 Method of Saturation Back prassure Controlled Stress Controlled Strain Type of specimen Remolded in./min Rate of strain 0015 (Nominal) Classification SILT, sondy (ML) 17 PL 13 **三** G 268 ΡI Remarks # At 15 % strain Project Dierks Composite of samples Area BOTTOW Material 12,500, 12,509, 12,510 € 12,511. (958, W+3) Sample No. /7-/2, 508 C

ENG FORM 2089

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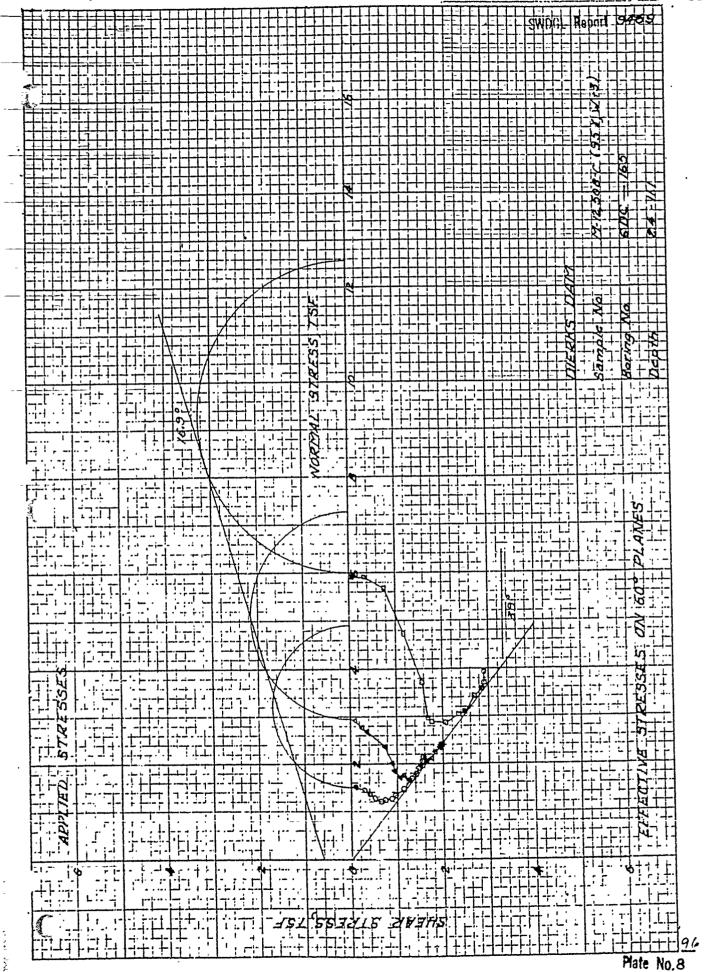
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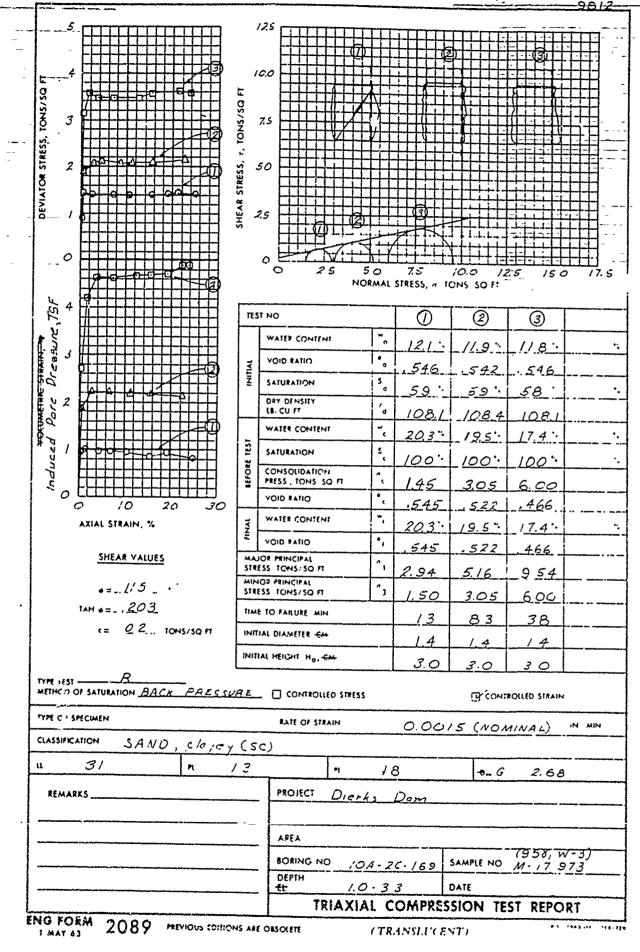
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Srace 75 9812 1 ------14. 1. (M/2) EX 621-W · L.· -11.6-; Nowal <u>m</u> - - - -1, 7.0 £ ··: ¦. ٠. 1. ļ 1:1 • : : DIEAKS. DAM 1 -BORING NO. .i ;. 17. 1 1 j'::: SAMPLE - 1 1 1 . r : 4.i 4 ! .i .! - | [_. i. :-: <u>.</u> . PLANES . . 1 1.1 . - - - - -. . · 00 • ; 390 ₹. I-1-; i 11:1 ŧ STRESSES 1111 ;--STARSSES 1-1-1 IJ 00-1 ! 1 ERFECTIVE r; ; ; . : : ., ! | 1-12 1 1: i . . . 1-71 , ; ; : ; ţ .Lj 1 9 \$\$.7¥15 WINS . | | 451 . . . ١.

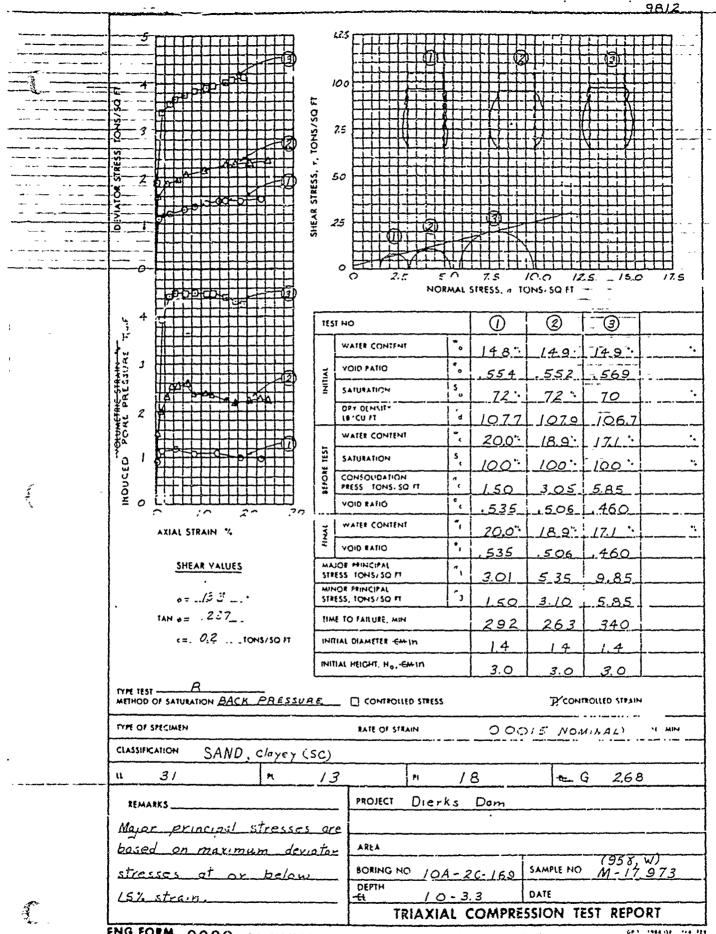
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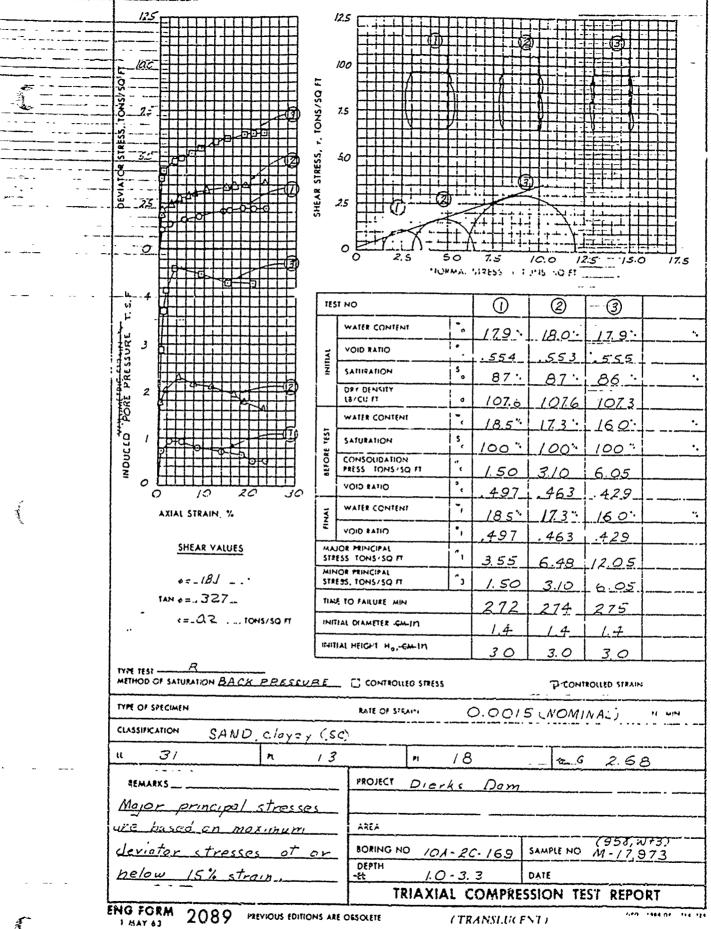
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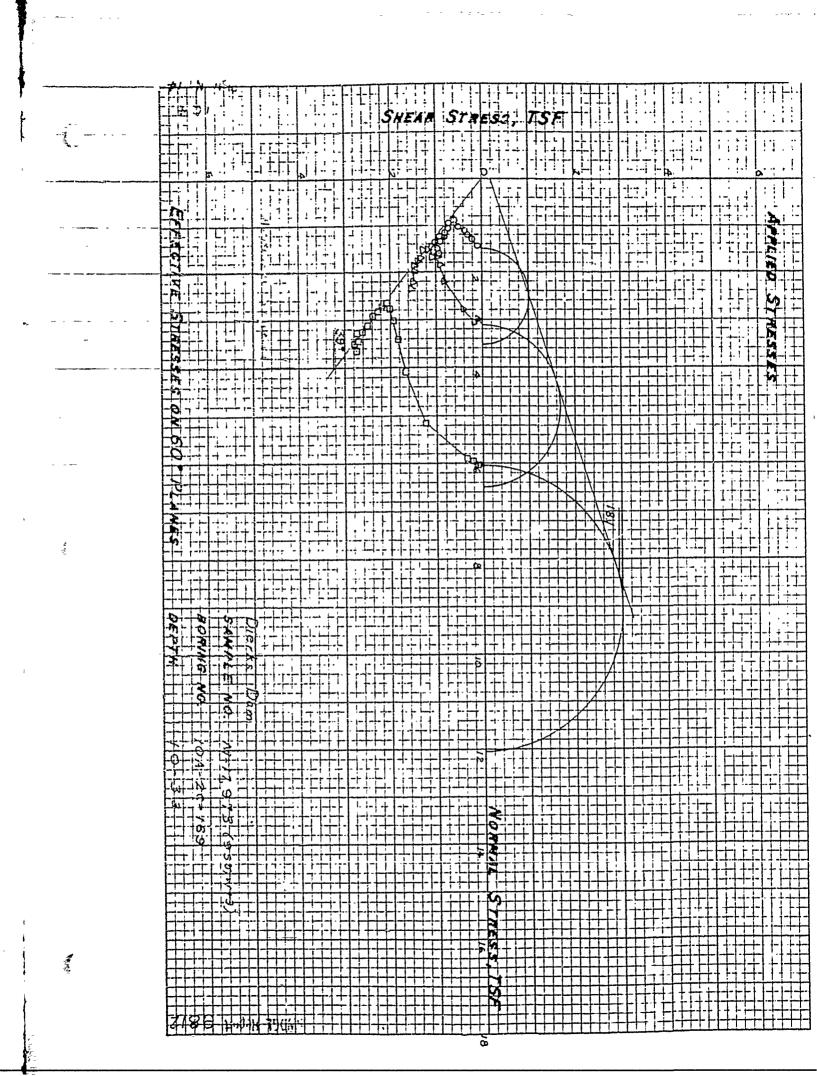
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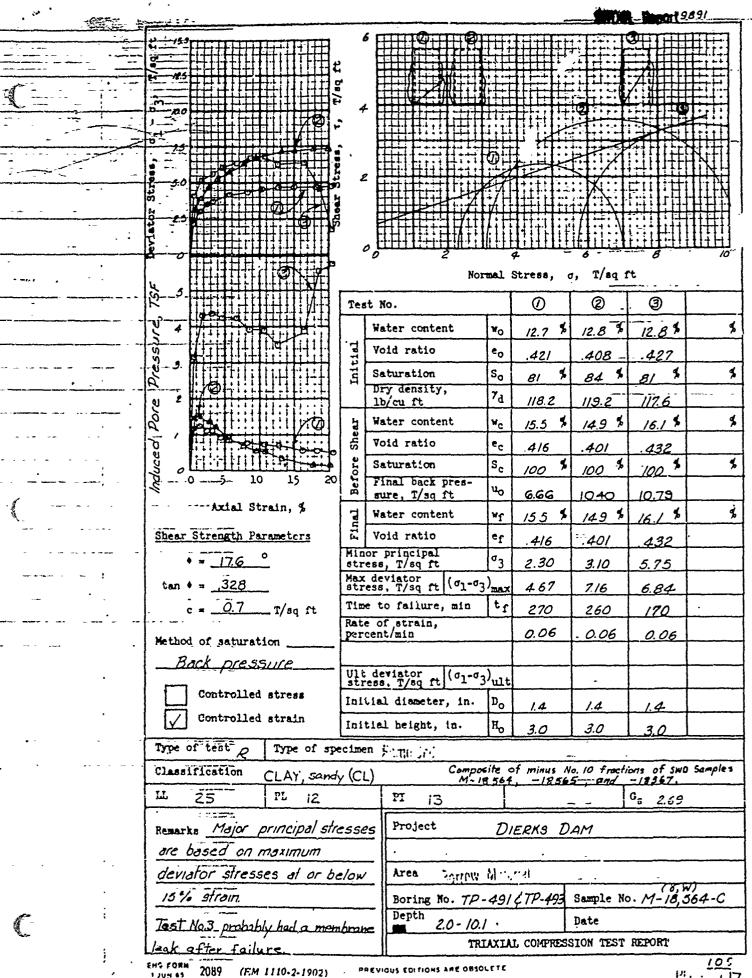
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SWDGL Report 9891 10 Normal Stress, o, T/sq ft (/)(2) 3 Test No. Water content w_o 10.1 10.2% 9.9% Void ratio eo .505 501 509 Saturation So 1 54 % 55 % 53 % Dry density, 7_d 111.6 2 111.9 111.3 1b/cu ft Sheer Water content ¥c 17.8 % 17.5% 15.5 % % Void ratio ec . 478 .472 416 Saturation sc \$ 100 \$ 100% 100% 0 Final back presuo 14.91 18.28 15.51 sure, T/sq ft Axial Strain, \$ Water content 17.8 % 17.5 \$ 15.5% ٧ſ Shear Strength Parameters Void ratio eſ .478 472 .416 Minor principal stress, T/sq ft • = <u>10</u>.3 σ_3 1.50 3.00 5.95 Max deviator stress, T/sq ft (01-03) max _.182 1.56 3.30 tan + = 2.00 tf Time to failure, min __ T/sq £t 196 53 43 Rate of strain, percent/min 0.06 0.06 0.05 Method of saturation Back Pressure Ult deviator stress, T/sq ft (01-03)ult Controlled stress Initial diameter, in. 1.4 1.4 1.4 Controlled strain Initial height, in. Но 3.0 3.0 3.0 Type of test R Type of specimen Ramalit Composite of minus No. 10 fractions of SWD samples
M-18564, -18565, and -18567 Classification CLAY sandy (CL) and -18567 25 PL 12 PI 13 2.69 Project DIERKS DAM Remarks Major principal stresses are based on maximum Romew Material Area deviator stresses at or (957, w-3) Sample No. M-18,564 -C below 15% strain Boring No. TP-491&TP-493 Depth Date Lit 2.0-10.1 TRIAXIAL COMPRESSION TEST REPORT

ENG FORM 2089 (EM 1110-2-1902)

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SNIMICE WO. TE 491 ++---WES -1-1 į 12 - -- : : TIL x 60° .9 0 13 TAG 38 ESS. Errective 5 All Pulled ++ ++ + + + + + **"**. |-|-|-!-!-;+ 102 451 'ss=#15 PF#15 : 1: ij Ł 1

SWINGL POINT 9891 0 Normal Stress, o, T/sq ft (3) 0 Test No. (I)12.5 \$ 12.9% Water content 12.9% Yo eo Void ratio 489 491 .500 4 Saturation So 70 \$ 69 **\$** 71 Dry density, 7_d 112.8 112.6 1/2.0 2 lb/cu ft ď Water content 17.7 \$ 17.0% 16.0% Void ratio 430 477 .457 ec \$ Saturation Sc 1005 100% 100% 10 Final back presu_o 12.28 10.72 13.70 sure, T/sq ft Axial Strain, % 17.7 16.0% % Water content 17.0% Shear Strength Parameters Void ratio .477 eſ 430 457 Minor principal 1.60 13.1 3.10 5.90 stress, T/sq ft Max deviator stress, T/sq ft $(\sigma_1 - \sigma_3)$ 1.93 4.26 tan * = _.233 2.48 Time to failure, min 228 188 246 $c = \frac{\sqrt{3}}{T/sq}$ ft Rate of atrain, percent/min 0.08 0.06 0.07 Method of saturation Back Pressure Ult deviator stress, T/sq ft (o1-o3) ult Controlled stress Initial diameter, in. 1.4 1.4 1.4 Controlled strain Initial height, in. 3.0 3.0 3.0 Type of test R Type of specimen Remole ! Classification Composite of minus No. 10 fractions of SWD Samples CLAY, sandy (CL) 18565 -/8567. 25 PL 13 2.69 12 PI Gs Project DIERKS DAM Remarks Major principal stresse are based on maximum Area Berroy, Material deviator stresses at (95 x, w) Sample No. M- 18,564-C Boring No. TP-491 / TP-493 below 15% strain Depth Date 44 2.0-10.1 TRIAXIAL COMPRESSION TEST REPORT 109

ENG FORM 2089- (EM 1110-2-1902)

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367.48 7. W. 18, 25, 64. 7. (9 T MIN R 1 11-1-1 09 WO S 1 37534 (V.C. S) 5 77774 4 4 1 H JSJ SSTUZG WERNG 7 ŢŢ 1,0 : Ļ

STILL KAPORT 9891 Normal Stress, o, T/sq ft (I)(Z) 3 Test No. Water content 16.05 15.9% 15.9 \$ ¥a Void ratio e_o 497 494 .499 So Saturation 87 \$ \$ 87 S 86 \$ Dry density, 7_d 112.2 112.4 112.0 1b/cu ft 14.1 % Water content 15.7 % 14.9% 4 V_C Void ratio 422 ec .400 .381 Saturation \$ Sc 100% 100% 100 \$ 15 Final back presսօ 7.13 8.92 8.87 sure, T/sq ft --- Axial Stre Water content 14.9% ď ٧ſ 15.7 \$ 14.1 \$ Shear Strength Parame Void ratio ef .422 .400 .381 Minor principal stress, T/sq ft σą 1.55 6.00 3.20 Max deviator stress, T/sq ft (01-03) max tan 4 = _.358 5.19 4.43 7.30 Time to failure, min c = 0.4 T/sq ft 216 368 332 Rate of strain, percent/min 0.07 0.05 0.05 Method of saturation Back Pressure Ult deviator stress. T/sq ft (01-03)ult Controlled stress Initial diameter, in. 1.4 1.4 1.4 Controlled strain Initial height, in. H_o 3.0 3.0 *3*.0 Type of test R Type of specimen fight 0.0. Classification Composite of minus No. 10 fractions of SWO Samples CLAY, sandy (CL) -18565 and -18567. 25 G₈ 2.69 PI Project DAM DIERKS Remarks Major principal stress are based on maximum Area Borrow Material deviator stresses at a Sample No. M-18,564-C below 15% strain Boring No. TP- 491 & TP-493 Depth -11 Date 2.0-10.1 TRIAXIAL COMPRESSION TEST REPORT 777 ENG FORM

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a		Tes	t No.		<i>①</i>	②	3	
	2	_	Water content	AO.	6.9 \$	7.0%	7.0 %	
	à (+)	Initial	Void ratio	e _o	. 186	.481	.488	
	i - Po	ä	Saturation Dry density,	So 7d	39 %	39 %	38 \$	*
		<u>.</u>	lb/cu ft Water content	v _C	17.9%	18.1 \$	112.1	%
	(-)	Shear	Void ratio	°c	. 478	.483	.460	
-		1 .	Saturation	Sc	100%	100%	100%	\$
	0 5 - 10 15 20	Befor	Final back pres- sure, T/sq ft	uo	14.4	13.5	15.1	
	Axial Strain, \$	12	Water content	۸ţ	17.9%	18.1%	17.2 \$	%
المهجردة في ر	Shear Strength Parameters	Fin	Void ratio	ef	.478	. 483	. 460	
, -	• = 0	6tre	or principal ess, T/sq ft	σ3	1.55	3.00	6.05	
·	tan • =		deviator ess. T/sq ft (01-03)			6.91	10.35	
	c =T/sq ft	Rate	to failure, min of strain,	tr		254	2:08	<u> </u>
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	Back Pressure	Սիե	deviator (01-03))				
	Controlled stress	1		D ^O	1.4	1.4	14	
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9891-2 95 7 6 13 排排 1 1 1 H 16 3 + 5 SAWPLE WO 0) TIT **[-**] 80 E .1-1-1 PLANES -----0x 60 1::: ... ا دا ك 7.8£3.8£5 -----1-1-1-8 + 1 **1** E CT IV T . . . . . 1:1 i · · . Elek ₹: : ; ; ::: i... ..... LLI. 7 7 7111 . . . 9 1 --! ı i, İ :: 11. 1:1 · : 1 .! ; 1351° FFZELS WYZHS +1 : ; [ ] ; <u>;</u> i_ ; ፦ ŧ , . ١. 111 1 1 I.I.F. Ī: i l in . 60 W. 3

JVUGL Recent 9891-2 10 0 Normal Stress, o, T/sq ft (I)(2) Test No. 3 5 Water content 9.5 \$ 9.4% ď -9.6% ¥o Void ratio eo .480 476 482 (+) Saturation 53 \$ \$ So 53% 53 % Dry density,  $\gamma_{\mathbf{d}}$ 112.9 0 112.7 --//25 lb/cu ft Water content 1729 17.4% 17.28 ۶, ٧c رے) Void ratio ec .458 .465 459 \$ Saturation sc 100% 100% 100% 5 L 10 Final back presu_o 15.0 14.5 17.8 sure, T/sq ft Axial Strain, \$ Water content 17.2 \$ 17.4% 17.25 ¥ſ Shear Strength Parameters Void ratio er 459 .458 465 Minor principal 1.55 2.90 6.05 stress, T/sq ft Max deviator stress. T/sq ft (01-03) max 15.19 9.68 /6.80 Time to failure, min 395 325 201 _____T/sq ft Rate of strain, percent/min 0.04 0.05 005 Method of saturation Back Pressure Ult deviator stress, T/sq ft (01-03)ult Controlled stress Initial diameter, in. 1.4 1.4 1.4 Controlled strain Initial height, in. 30 3.0 3.0 Type of test R Type of specime Classification SAND, silty (SM) NP PL NP PI NP 2.67 Project DIERKS DAM Remarks Area Portrio Materia! Sample No.M-18,548 Boring No. TP-488 Depth Date JA'. 0.0-80 TRIAXIAL COMPRESSION TEST REPORT

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APPENDIX E

COMPACTED MATERIALS

SECTION 3. DIRECT SHEAR TESTS

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SWDGL Report 9467 *K£ ~{\^ Normal Stress, o , T/sq ft 3 (1) Test No. Water content 15.9\$ 15.9 \$ 15.95 Void ratio 519 s Saturation Dry density 110.6 lb/cu ft Void ratio after consolidation
Time for 50% t₅₀ consolidation, Water content 16.4 \$ 15.45 Void ratio Horiz Deformation, in. Saturation Actual time to 1600 1500 1000 failure, min. Shear Values . Normal stress 1.5 T/sq ft 3.0 6.0 0' = 34.6° Maximum shear tan 0' - .690 1.40 2.22 4.23 strength, T/sq ft c' = 0.2 iest Type (Check One) Controlled, stress Controlled, strain .0002 in min. Type of Specimen Remolifica in. Square in. Thickness CLAY (CL) Classification De 63 2.69 30 ΡI 14 Remarks Project Dierks Dam Area Borrow Material (958, W+3) Sample No. M - 12,500 Boring No. 8A.203 Depth 0.0 - 3.0 Date NOV DIRECT SHEAR TEST REPORT ENG FORM C 3437

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APPENDIX F .

MONTHLY SUMMARIES OF

COMPACTION AND WATER

CONTENT TESTS

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CONTRACTOR Amis

SUMMARY OF FIELD COMPACTION CONTROL OF

(1) MonTAly Saline Ark Dirk

RIVER STATE TOWN

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Dierks

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IMPERVIOUS OR SEMIPERVIOUS SOILS FOR CIVIL WORKS PROJECTS

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SUMMARY OF FILLD COMPACTION CONTROL OF IMPERVIOUS OR SEMIPERVIOUS SOILS FOR CIVIL WORKS PROJECTS

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**5. SELECTION ONC. 18" MC-021 (+1) (MAIN FORM) RCS, ENGCW-5-11(RI) COMMENTS ON TESTS PRESSURE IPSI) CONTACT CORPS OF GHGINEERS COM. 100 100 2 73.78 ----FIELD AND LABORATORY CORRELATION 15.0 12.4 22 1 00 00 11.7 12.4 -0.3 113.3 15.2 2.1 115.9 15.2 +1.1 U & ARMY ENGINEER NUMBER OF PASSES DAX WATER DAY CON. CON. CON. (1) (ECF) (3) COL 33 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 COL 34 C SUBMITTED BY: REPORT NO. PECM_ 5377 LABORATORY TEST DAT IT HOLD OF UTINO) IN LANGUAR WOLD, IN LANGUAR WOLD AN OUTLINED IN WARNAL HIDSINGS
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None of the last

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Results questionals HCS: ENGCH-E-II(RI) COMMENTS OH 18878 MET 2 05 2 CONTACT PRESSURE (PSI) CORPS OF ENGINEERS PESIDENT ENGINEER 110 707 8 102 707 28 23 10 99 104 35 10,0 -2.6 10.4 76.57 -0.Y -0.4 -0.9 , " 6 3 なる 10.0 -1.0 10,8 0 HUMBER OF 2.7 O S ARMY ENGINEEP ŕ 9.3 16.8 REPORT HO 13.1 11.3 120,9 12,87 123.3 11.6 10,0 7.7 LABORATORY TEST DATA
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THE WETHOL WITH MAS PARTICLE MET GRANGER THAN METHOD OF RETAINED IN WANDE, 1192-1164.

LAN CONTROLL WITH MAS AND WITE CO. BANKER CAN BE RECORDED IN COL. 12 AND 21, AND HE PROPERTION TO A DEVISION OF THE WAS DESIRABLE OF MODERN WAS DEATHER CANDED TO THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABLE OF THE WAS DESIRABL STANDAND METHOD 26-5-6 DATE OF REPORT DENSITY (FCF) 200 7687 20100 2 00 PACTION THE 13.3 87 6.3 17.3 8.5 15.3 17-40 61-41 61-47 64-16 11.4 13.5 9.0 8:5 1.6 COMPACTION EQUIPMENT (N) Amis Const. Co. 75.2 12211 121.3 134.6 124.6 6.77/ 135.2 OENCHY JPC+1 COL 10 124.4 11.2.6 121.4 1261 IN-PLACE DATA \$ 5 5 5 5 5 5 5 3 TOTAL SAMPLE CONTRACTOR DENSITY (PCP) ACONT TO LINES CLASSIFICATION £ 5 3 DACU-56-6-71-0151 1 8 \$100 GRADATION COC 13 - \$ 5 : \$ CONTRACT NO 77.70 COC 31 33 COL 30 # 100 100 100 100 Pancham Ext TA CLASS WORD OR LETTER STUBOL COL 10 Registr AREA C, FULLWAYGE TO TOP OF DEBUTY TEST
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APPENDIX H

PLATES

## DIERKS DAM AND RESERVOIR SALINE RIVER, ARKANSAS

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## EMBANKMENT CRITERIA AND PERFORMANCE REPORT

## APPENDIX H - PLATES

Plate	Drawing	<b></b>
No.	No.	<u>Title</u>
1	1960-C3-2/1.10	General Plan and Sections
2	1960-C3-98/2	Foundation Exploration: Top of Rock Contours - Flood Plain
3	1960-C3-12/1.5	Embankment Plan and Profile
4	1960-C3-13/1.2	Engineering Measurement Devices: Embankment, Plan & Sections
5	1960-C3-98/3	Overburden Investigation: Sections AA, BB & CC - Borings & Test Pits
6	1960-C3-98/4	Overburden Investigation: Sections DD, EE & FF - Borings & Test Pits
7	1960-C3-98/5	Overburden Investigation: Sections GG & HH - Borings & Test Pits
8	1960-C3-98/6	Overburden Investigation: Sections II & JJ - Borings & Test Pits
9	1960-C3-98/7	Overburden Investigation: Sections LL, MM & KK - Borings & Test Pits
10	1960-C3-98/8.1	Overburden Investigation: Borrow Areas D & E - Plan & Logs
11	1960-C3-98/9A.1	Overburden Investigation: Borrow Area C - Plan & Logs
12	1960-C3-98/10	Foundation Exploration: General Geology & Plan of Exploration
13	1960-C3-98/11	Foundation Exploration: Geologic Sections A-A thru C-C
14	1960-C3-98/12	Foundation Exploration: Geologic Sections D-D thru F-F
15	1960-C3-98/13	Foundation Exploration: Geologic Sections G-G thru M-M
16	1960-C3-98/14	Foundation Exploration: Geologic Sections N-N thru P-P
17	1960-C3-98/15	Foundation Exploration: Geologic Sections & Rock Classification
18		Impervious Density Test Flot Profile Along Dam Axis
19		Density Test Plot Typical Embankment Sections "A" and "B"
20		Density Test Plot Typical Embankment Sections "C" and "D"
21		Density Test Plot Typical Embankment Section "E" and Right Embankment Section
22	1960-C3-8/5.4	Diversion & Excavation: Spillway Excavation - Plan, Profile & Section

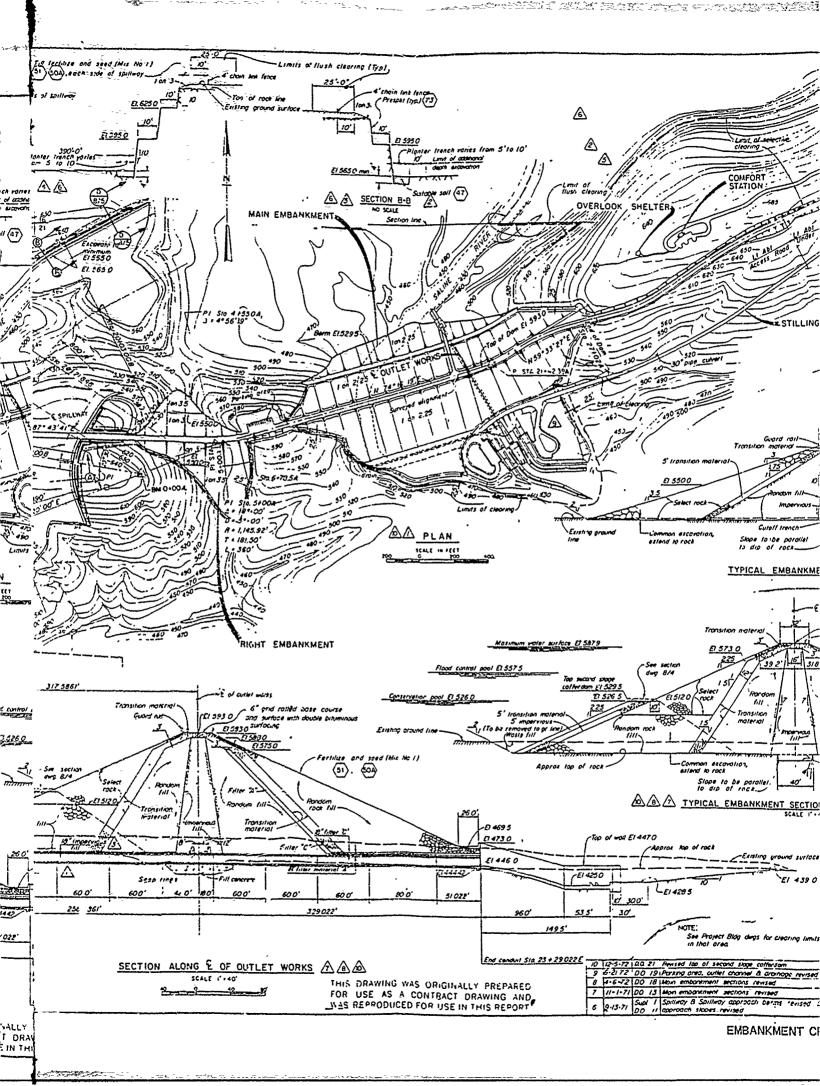
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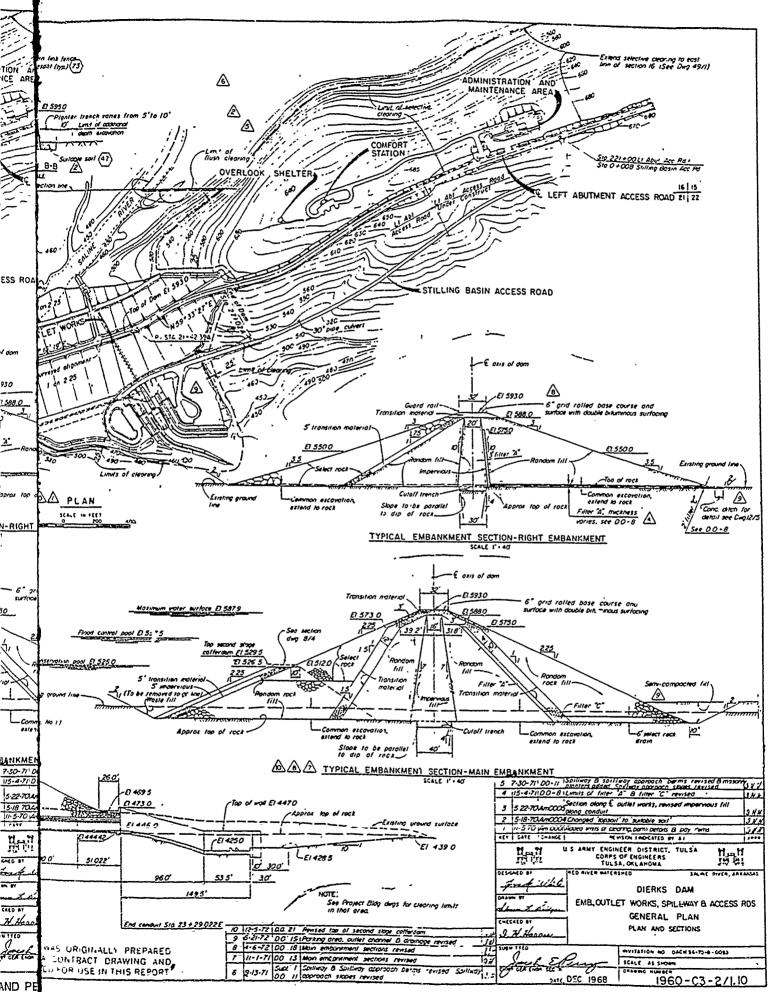
## APPENDIX H - PLATES (continued)

Plate No.	Drawing No.	<u>Title</u>
23	1960-C3-13/2.5	Engineering Measurement Devices: Embankment Sections and Details
24	1960-C3-13/3	Engineering Measuring Devices: Outlet Works and Service Bridge - Plan, Sections and Details
25	1960-C3-22/22.1	Outlet Works - Conduit: Concrete & Reinforcing - Conduit and Seep Rings - Sections & Details I
26	1960-C3-22/24.1	Outlet Works - Stilling Basin: General Plan, Excavation & Details
27	1960-C3-8/4.5	Diversion & Excavation: First Stage & Second Stage Diversion - Plans & Sections
28	1960-C3-12/2.10	Embankment: Typical Sections
29	1060-DM8-12/5	Main Embankment: Typical Sections
30	1960-DM8-97/10	Right Embankment: Upstream Sta. 2+50 - Stability Analysis - Sudden Drawdown Cond.
31	1960-DM8-97/11	Right Embankment: Downstream Sta. 5+20 - Stability Analysis ~ Steady Seepage Cond.
32	1960-DM8-97/12	Right Embankment: Upstream Sta. 2+50 - Stability Analysis - Partial Pool Condition
33	1960-DM8-97/13	Right Embankment: Downstream Sta. 5+20 - Stability Analysis - End of Construction Cond.
34	1960-DM8-97/12	Embankment and Spillway Stability Analysis - Partial Pool
35	1960-DM8-97/15	Embankment and Spillway Stability Analysis - Sudden Drawdown
36	1960-DM8-97/16	Embankment and Spillway Stability Analysis - Steady Seepage
37	1960-DM8-97/17	Embankment and Spillway Stability Analysis - End of Construction
38		Embankment: Typical Section
39		Part I Foundation Emb. Grouting - Grouting Profile
40		Sta. 0+00 to 7+00
		Part I Foundation Emb. Grouting - Grouting Profile Sta. 7+00 to 14+00
41		Part I Foundation Emb. Grouting - Grouting Profile Sta. 14+00 to 21+00
42		Part I Foundation Emb. Grouting - Grouting Profile Sta. 21+00 to 27+00
43	1960-C3-13/4	Engineering Measurement Devices - Conduit - Sections and Detail
4.	1960-C3-13/5	Engineering Measurement Devices - Embankment - Terminal Cabinet - Sections, Elevation and Details
45	1960-DM8-97/1	Materials Usage Chart - Contract
46	1960-DM8-97/18	Embankment and Spillway Materials Usage Chart - August 1972

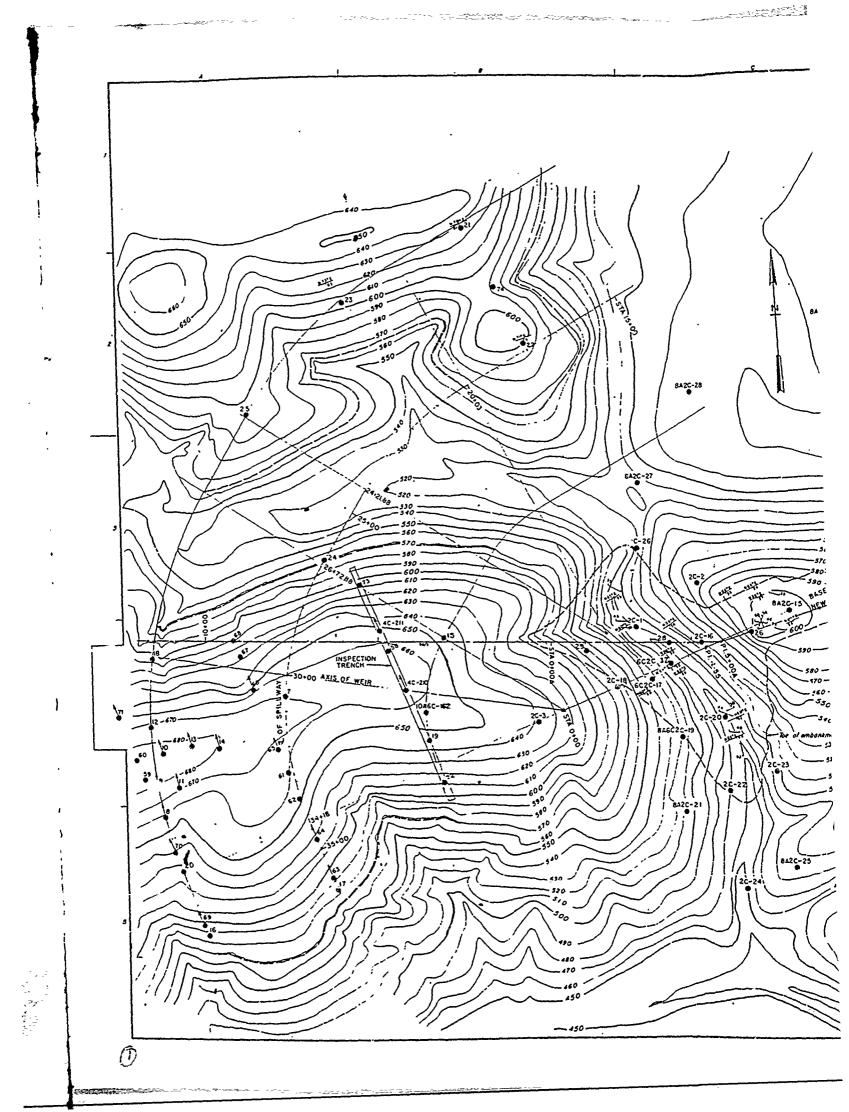
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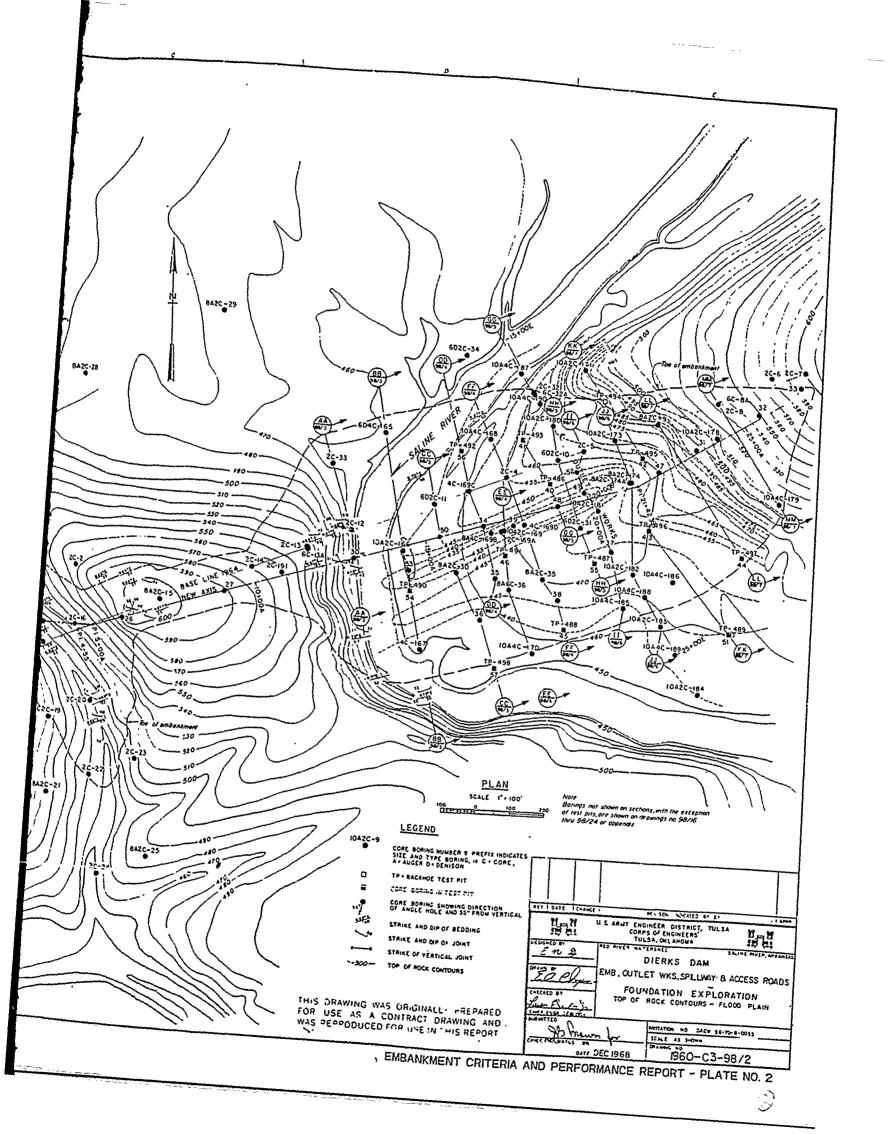
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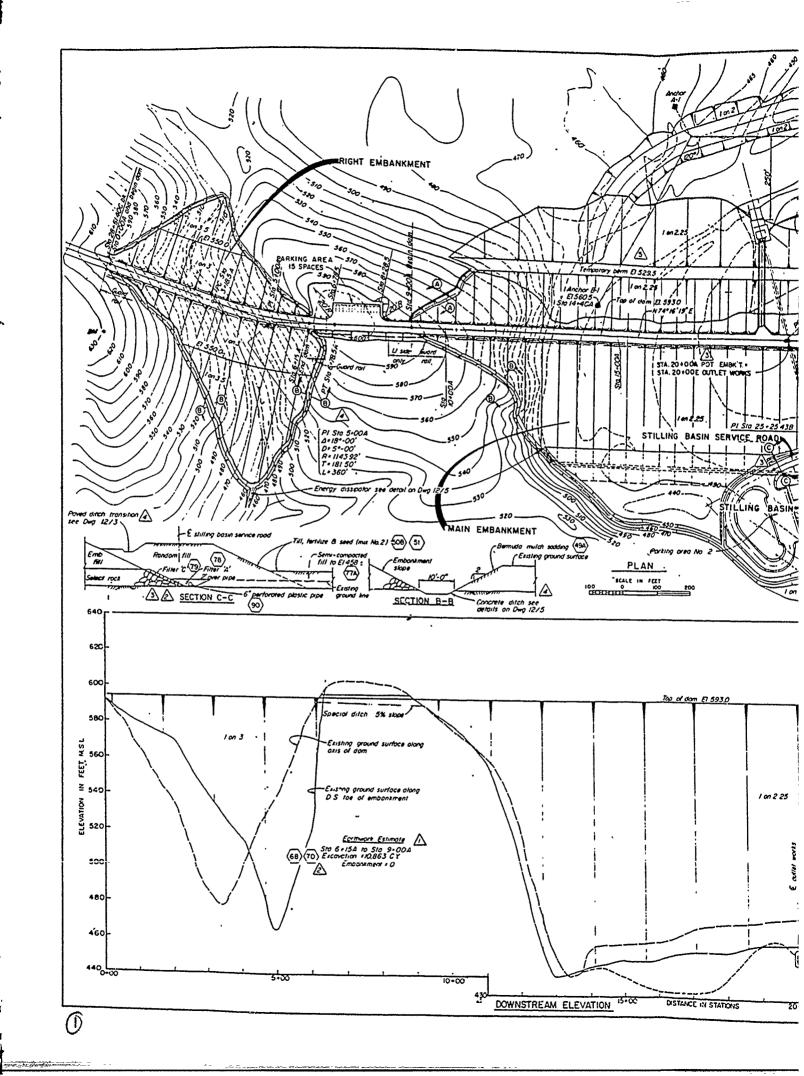


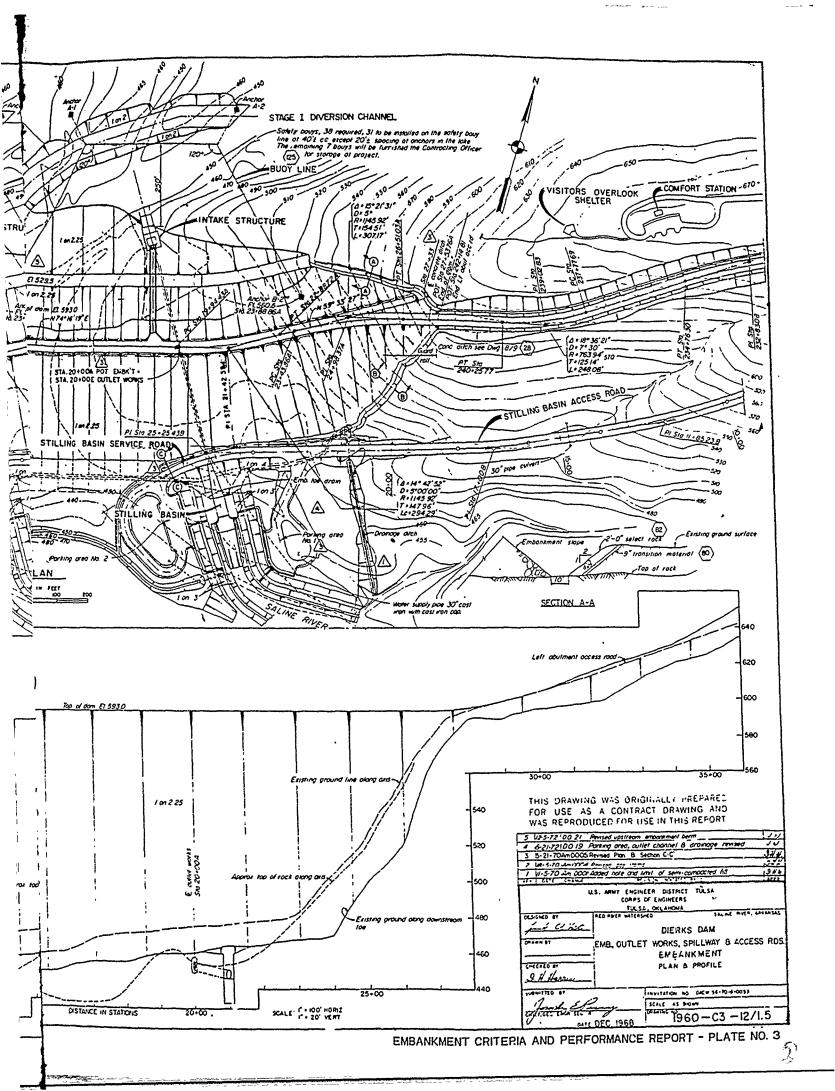


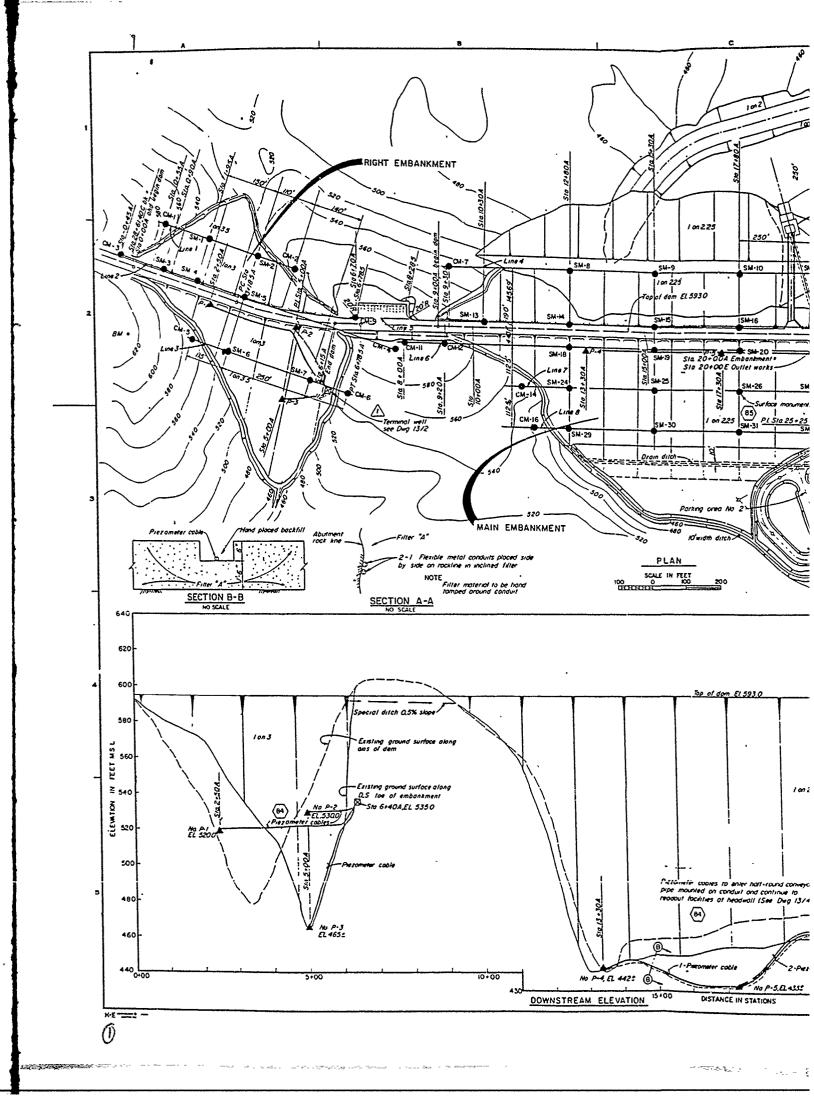
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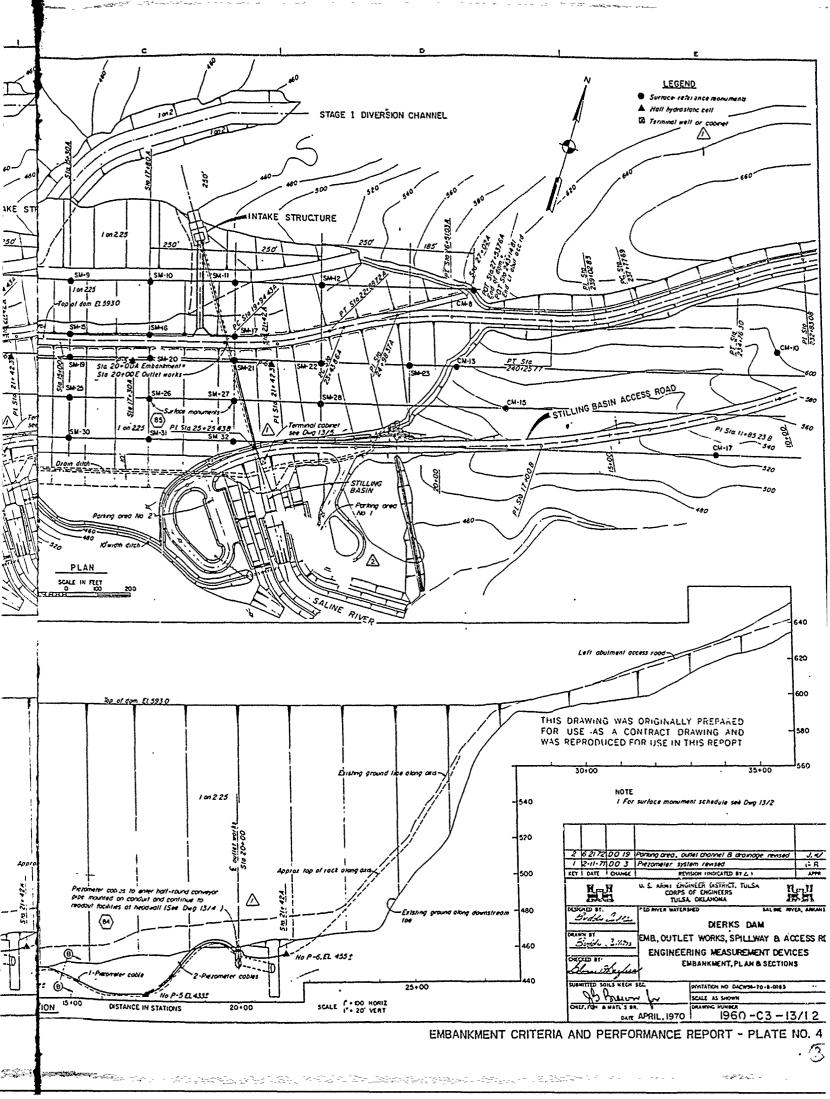


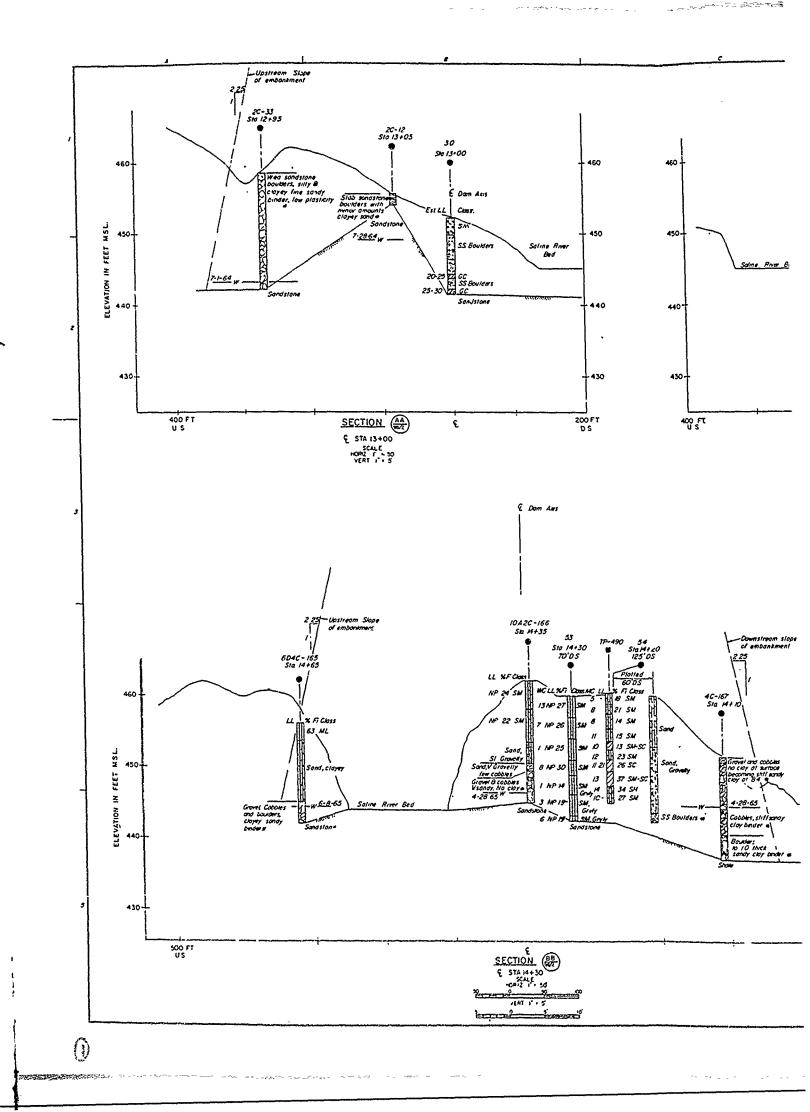


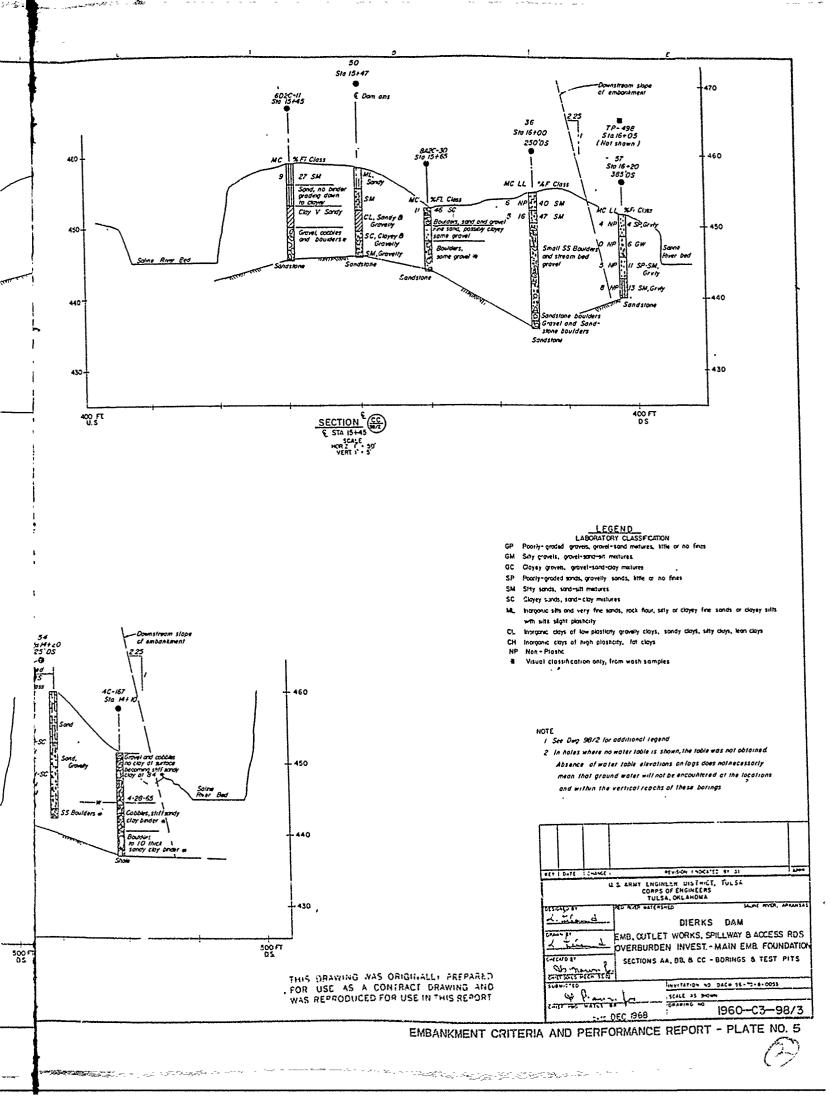


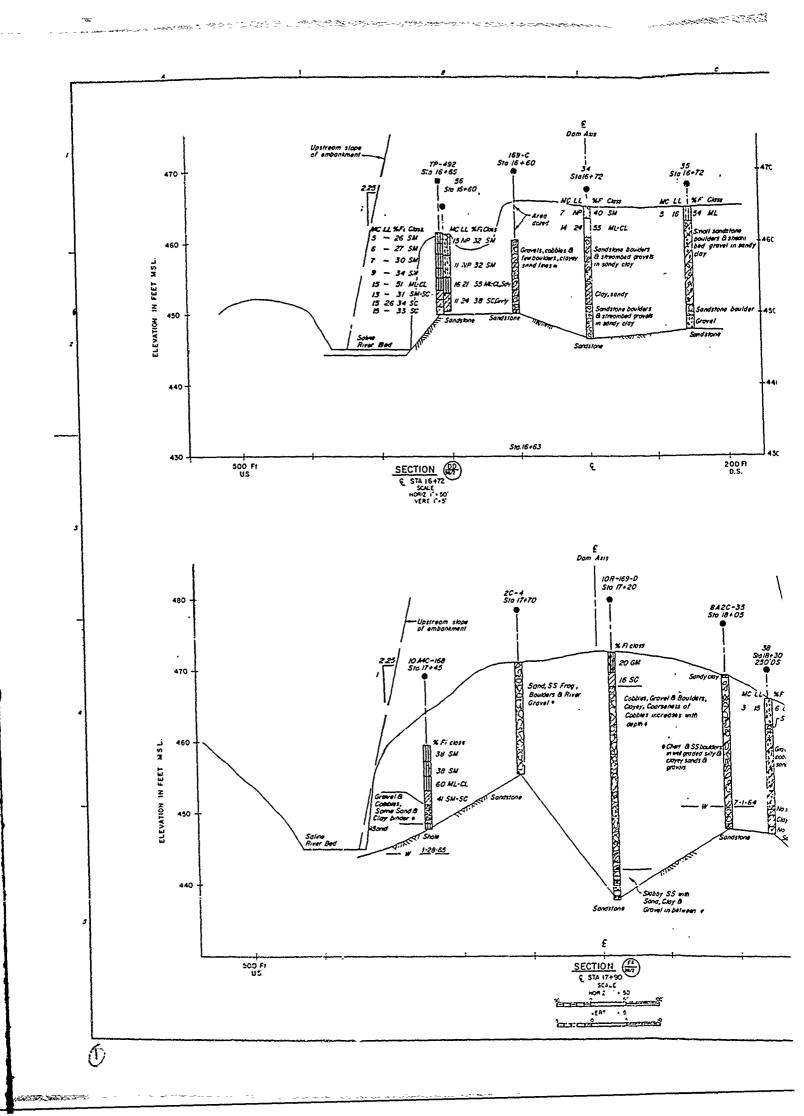


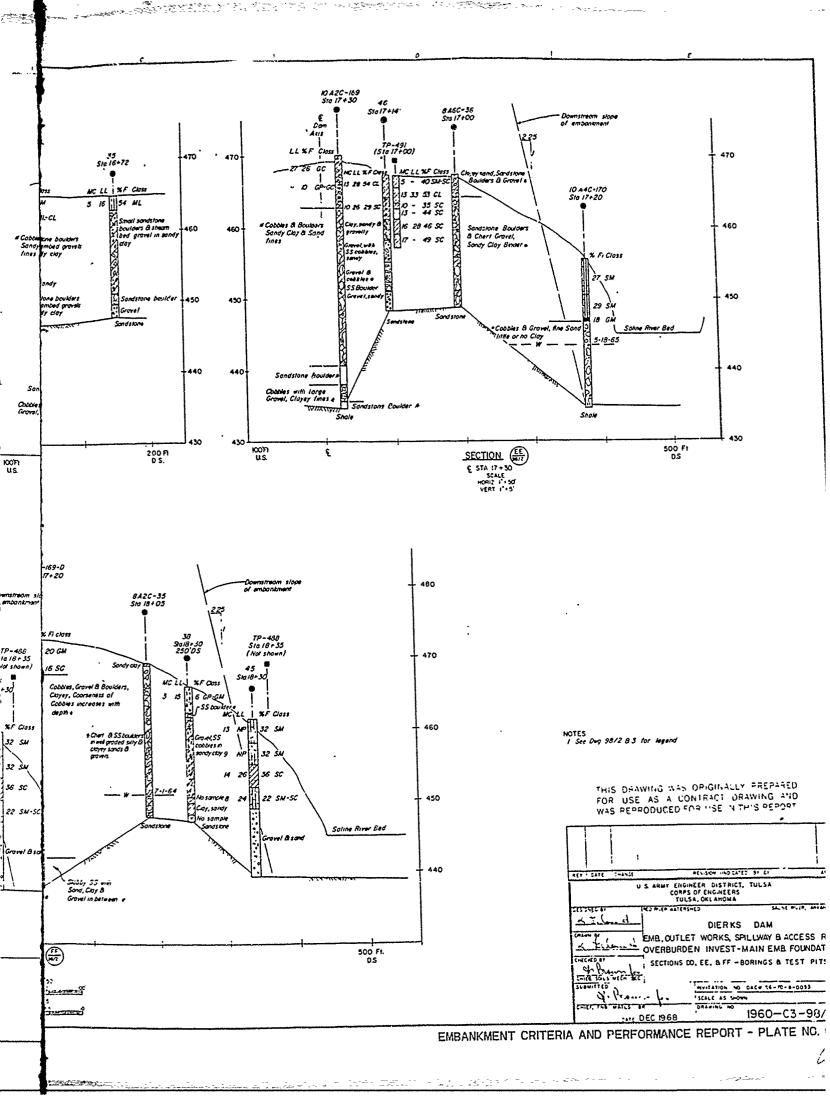


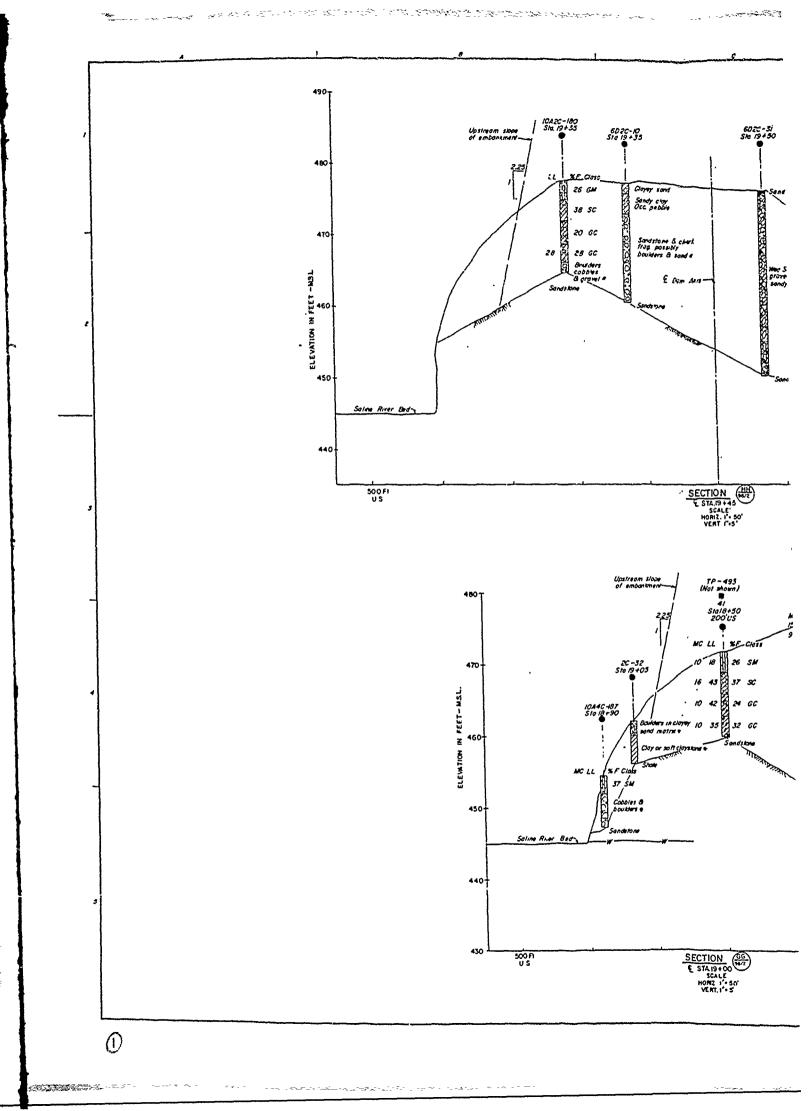




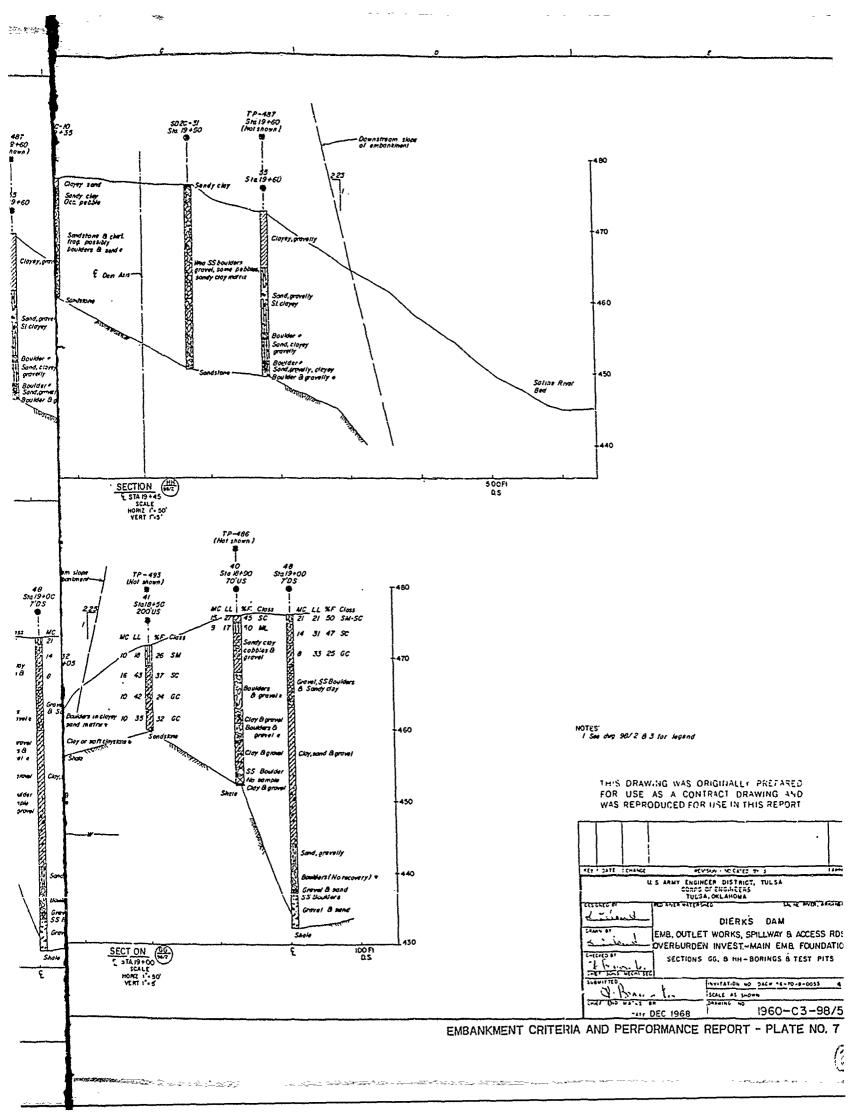


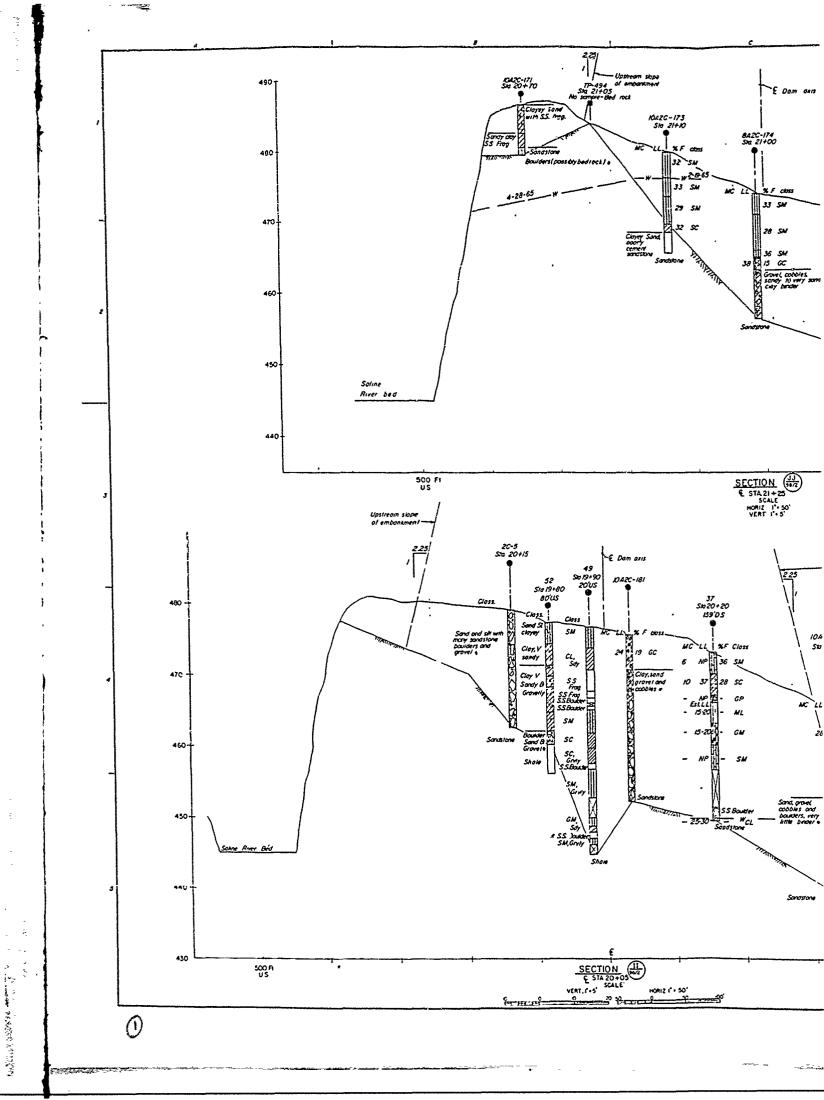


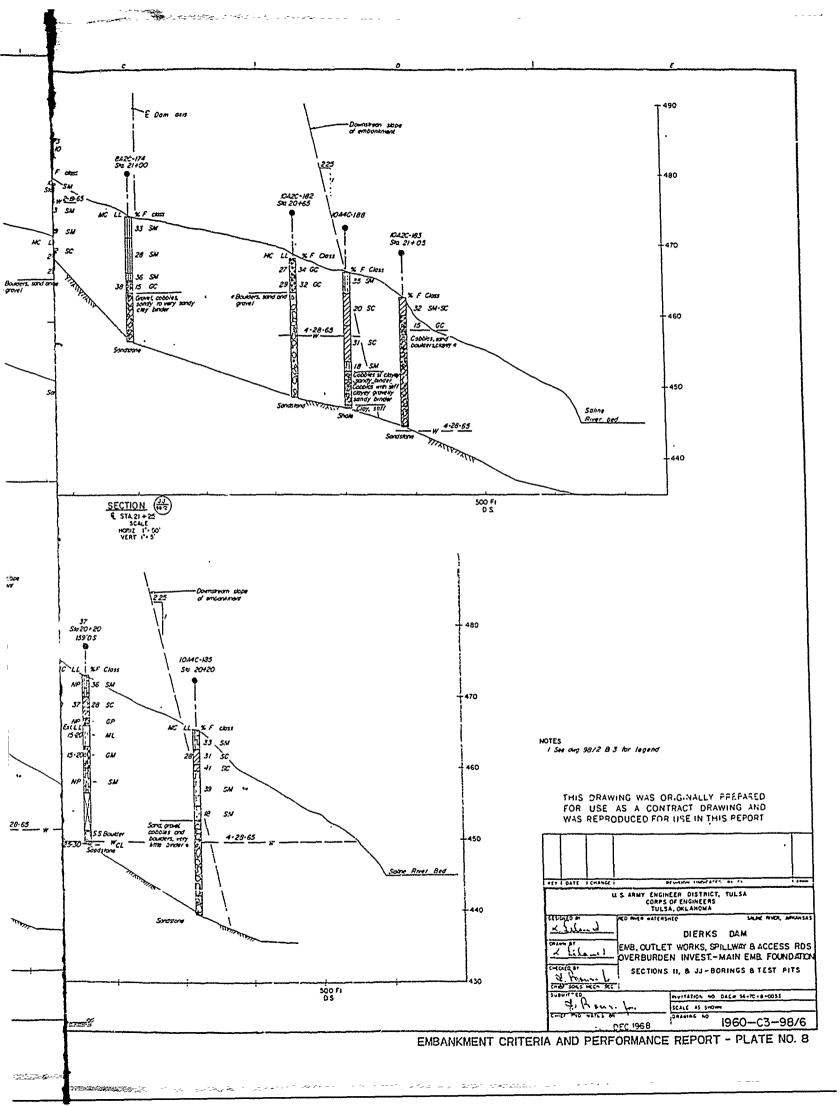


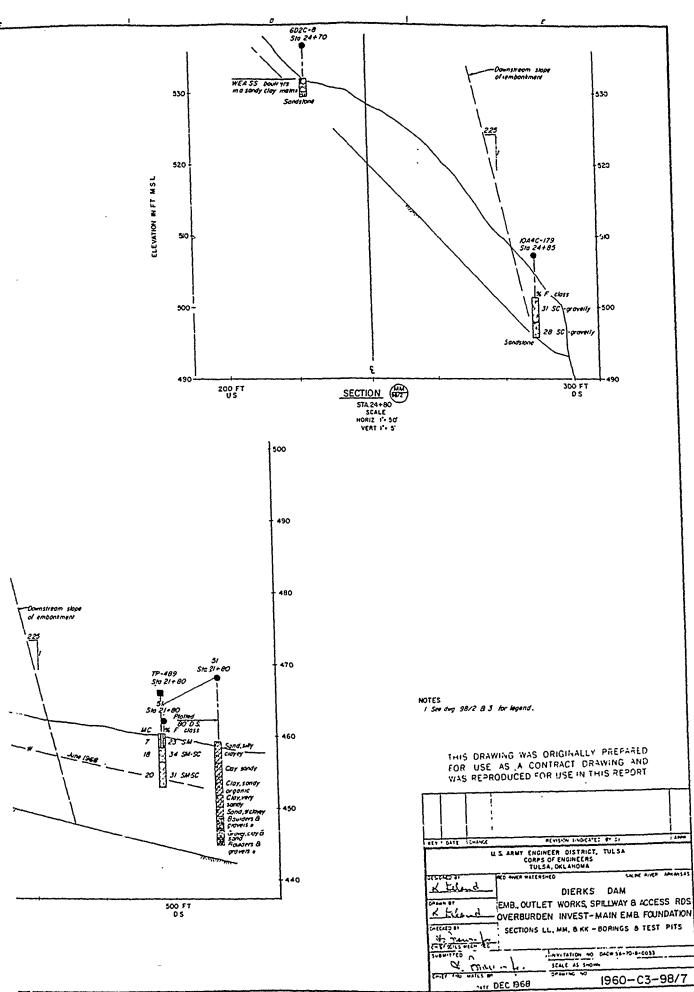


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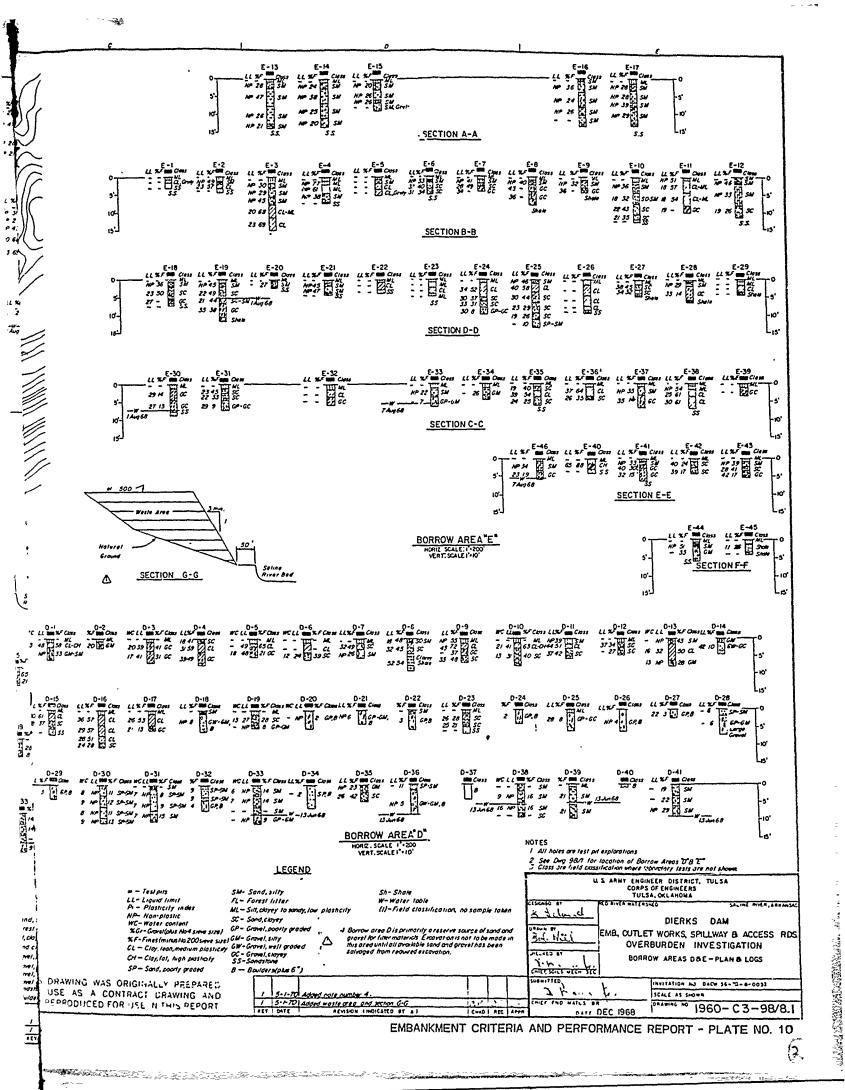


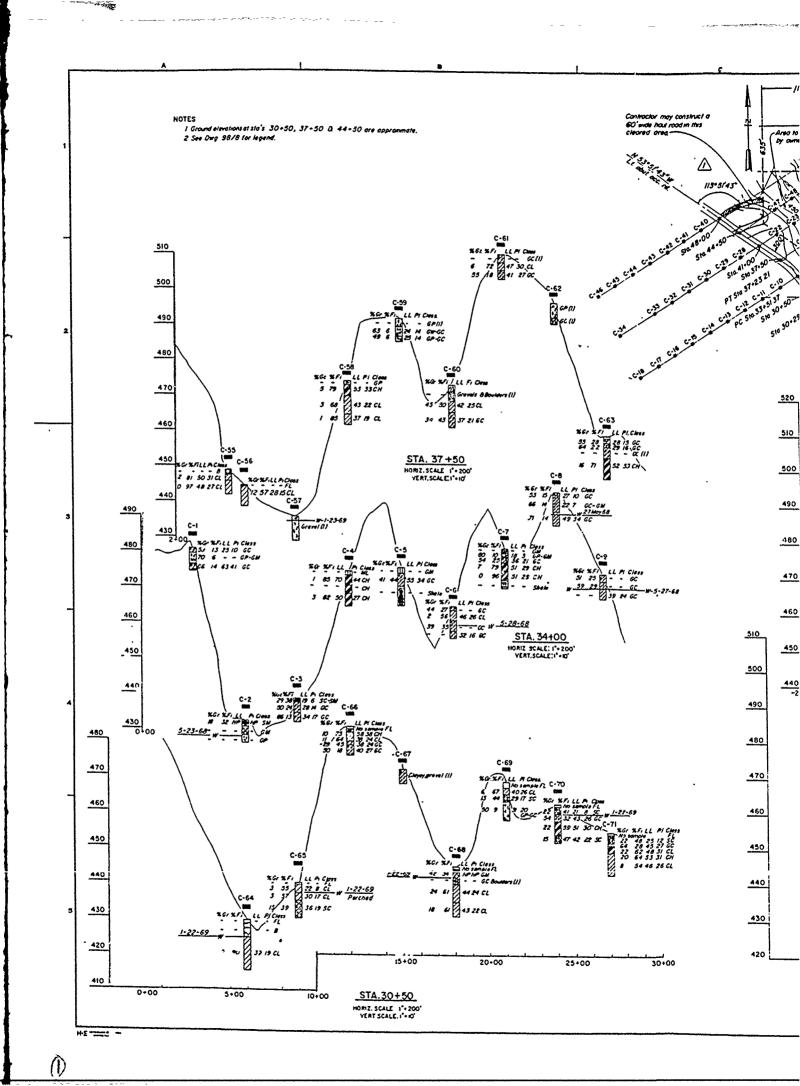


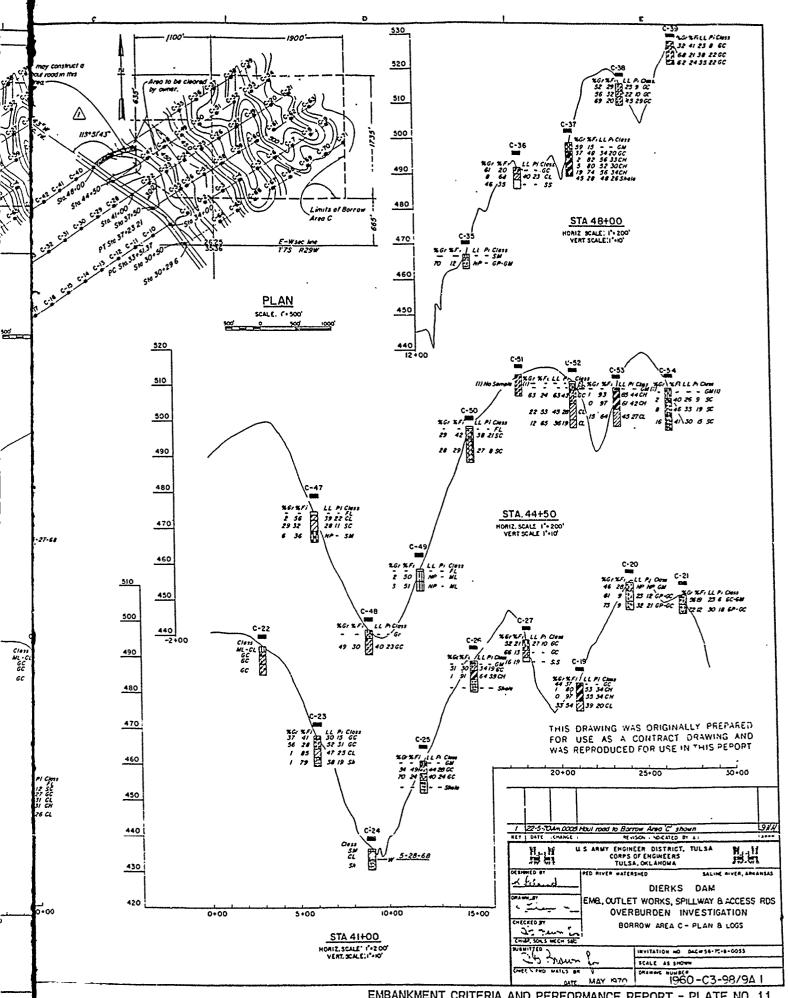
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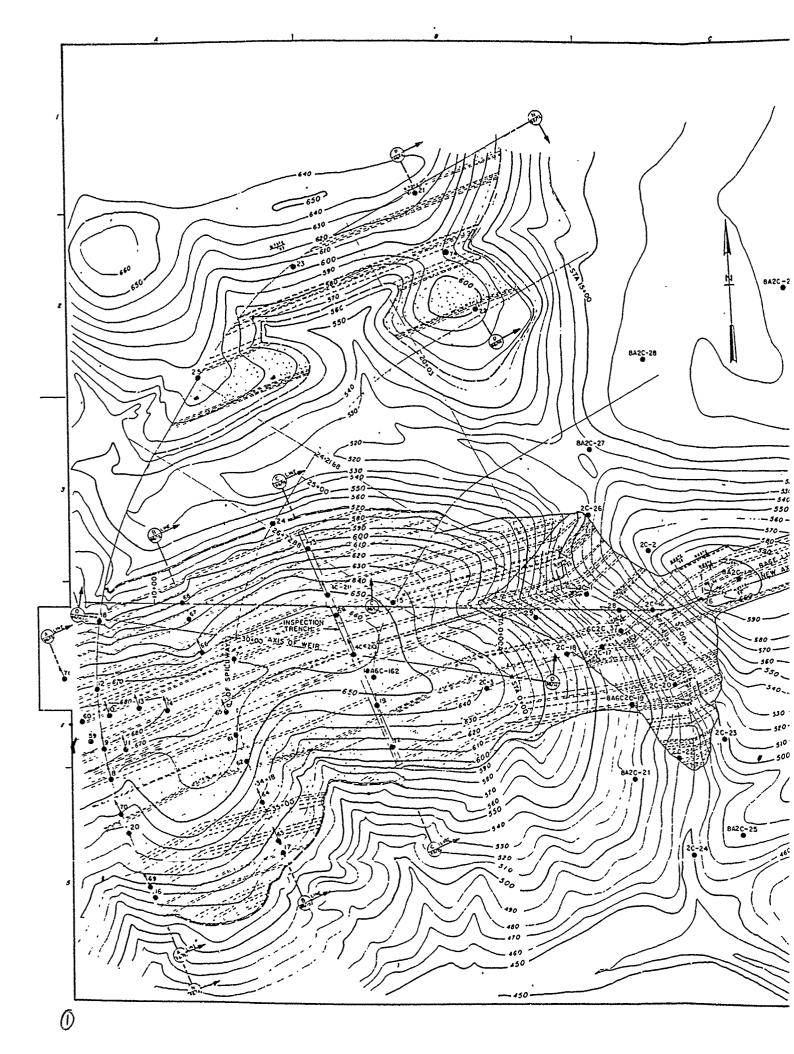
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Pr - Plasticity index

NP- Non-plastic
WC-Water content
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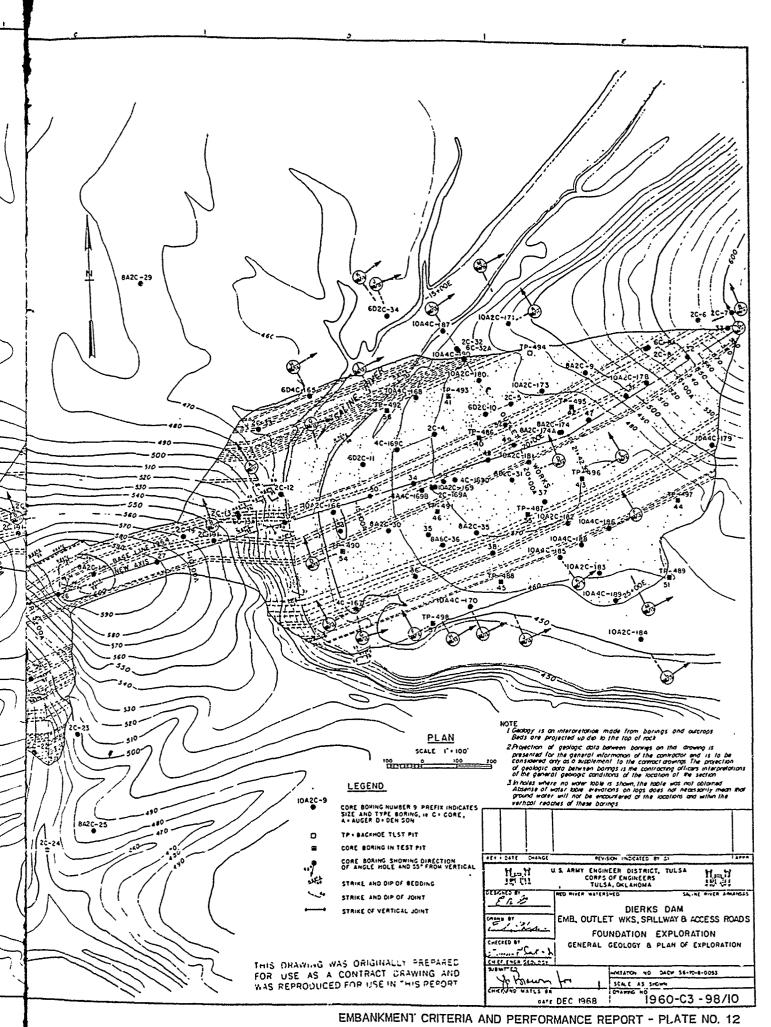


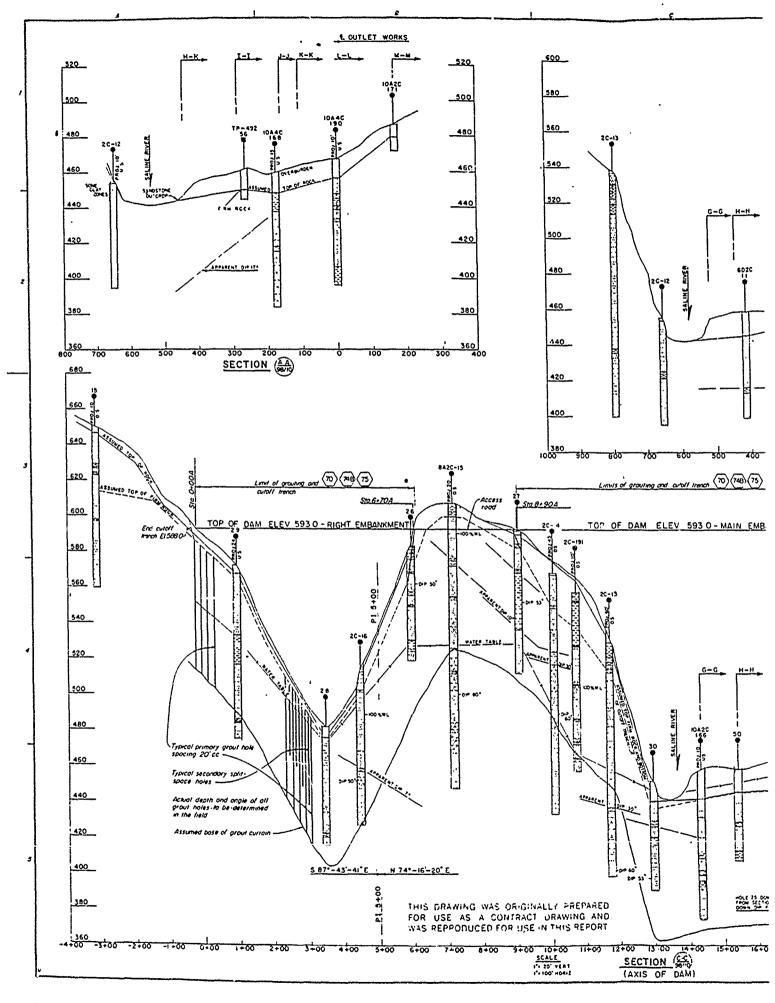




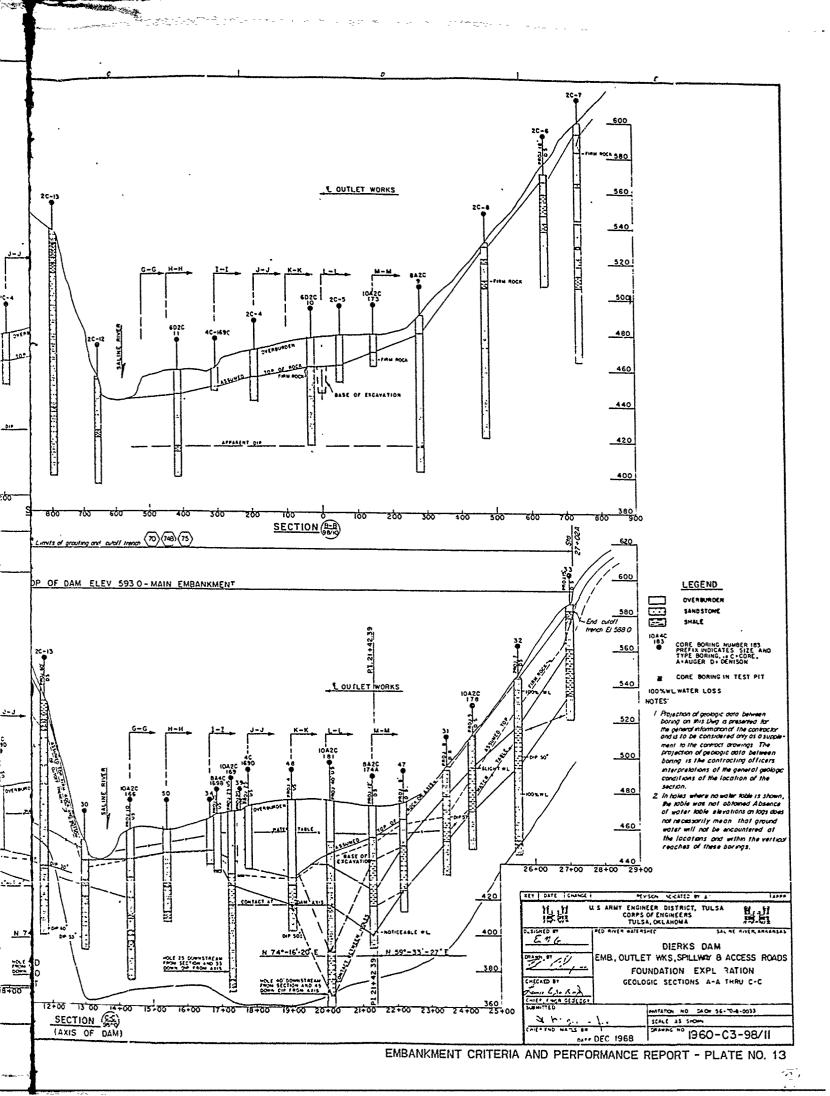


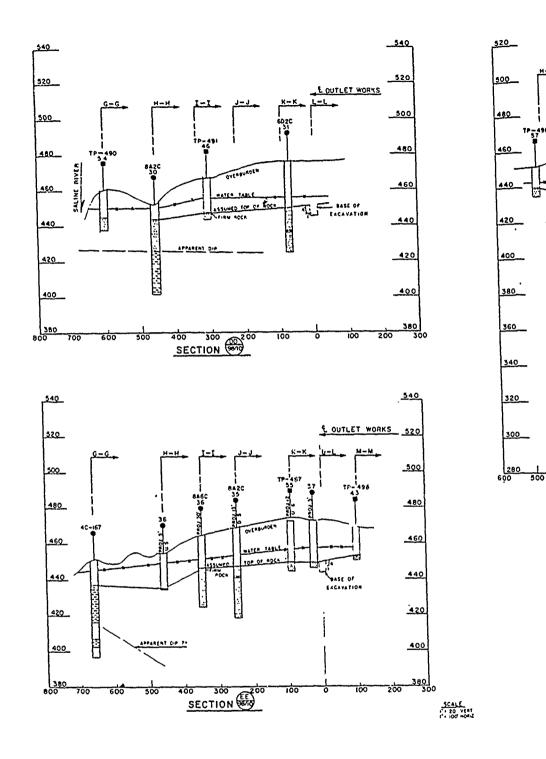
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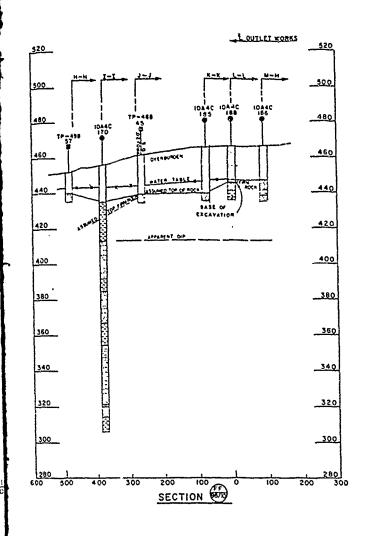


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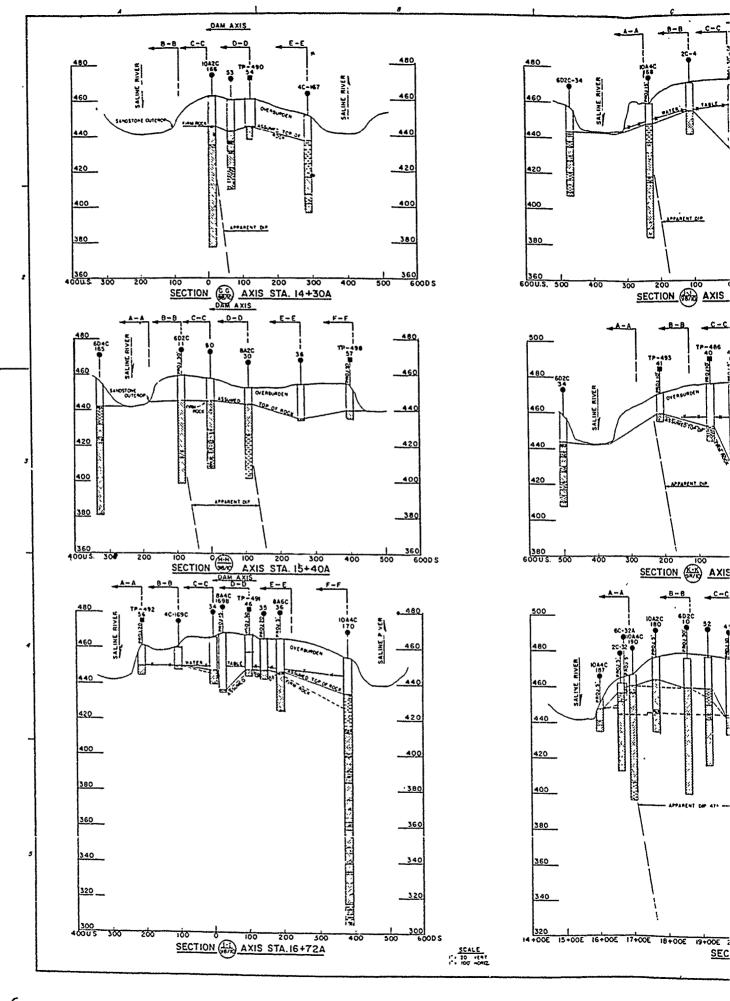
NOTES

Projection of geologic data between boring on this Dwg is presented for the general information of the controctor and is to he considered only as a supplement to the contract drawings. The projection of geologic data between boring is the contracting. officers interpretations of the general geologic conditions of the location of the section

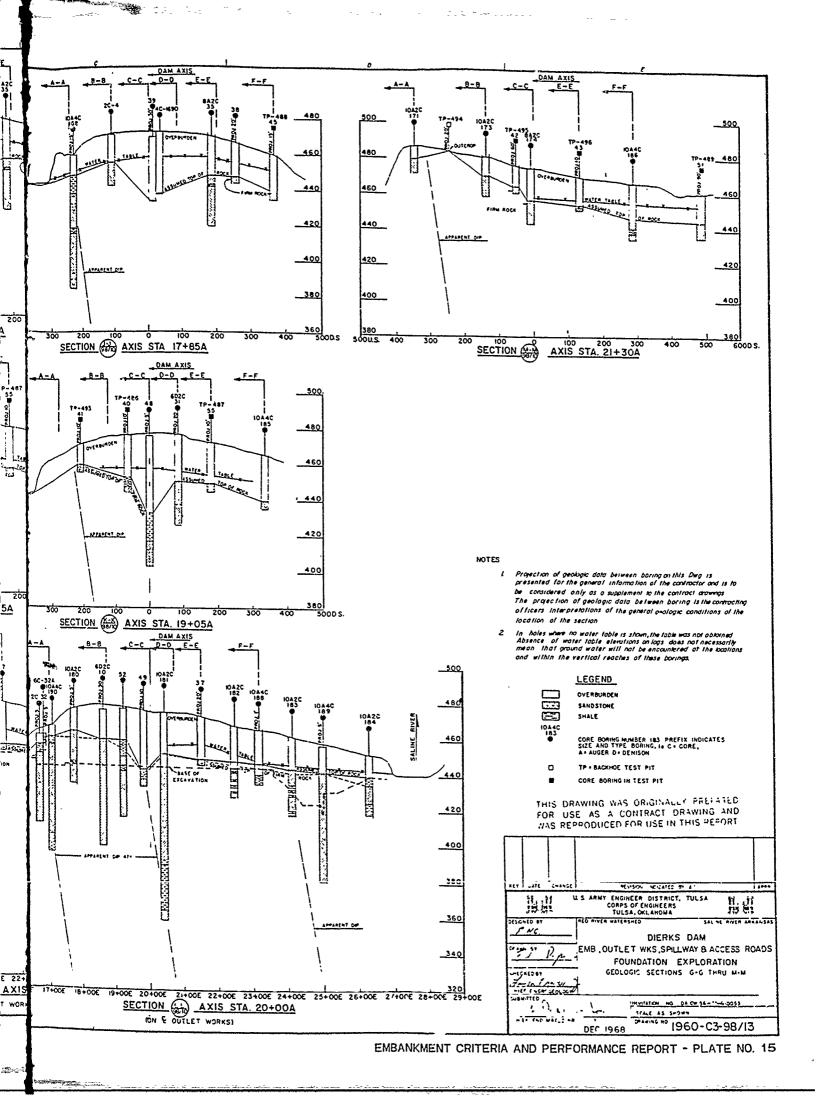
2 In holes where no water table is shown, the table was not obtained Absence of water table elevations on logs does not necessarily mean that ground water will not be encountered at the locations and within the vertical reaches of these bornings.

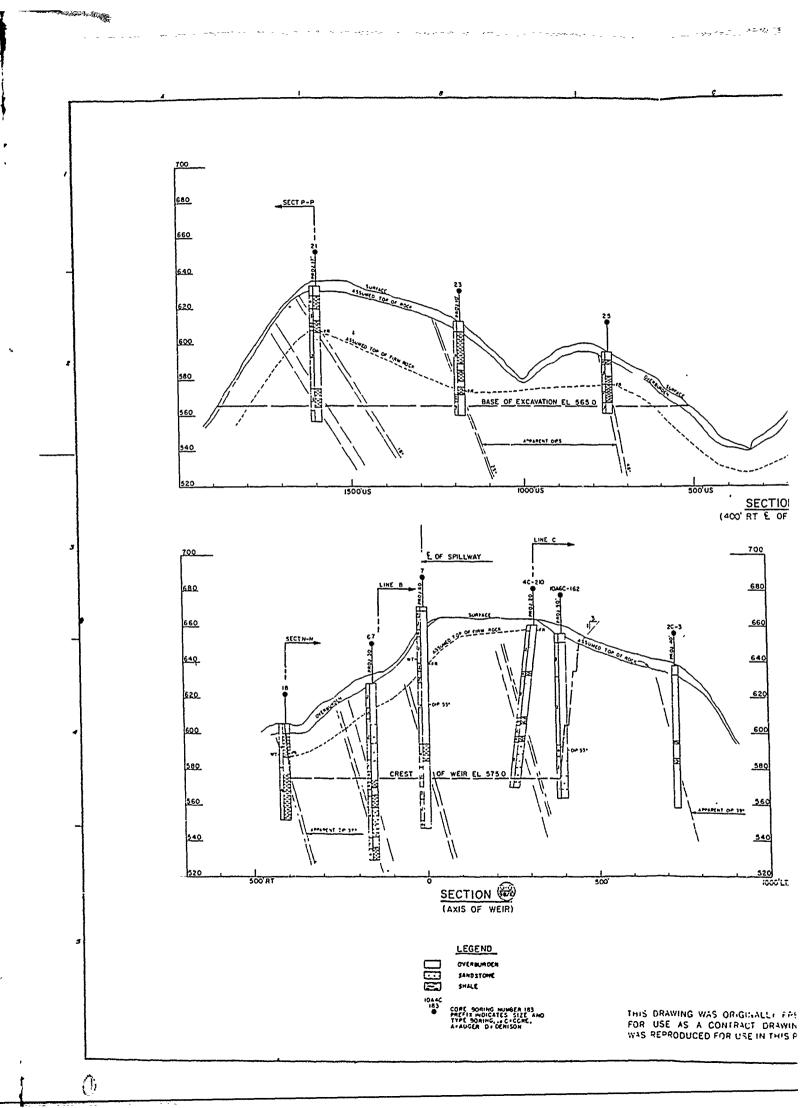
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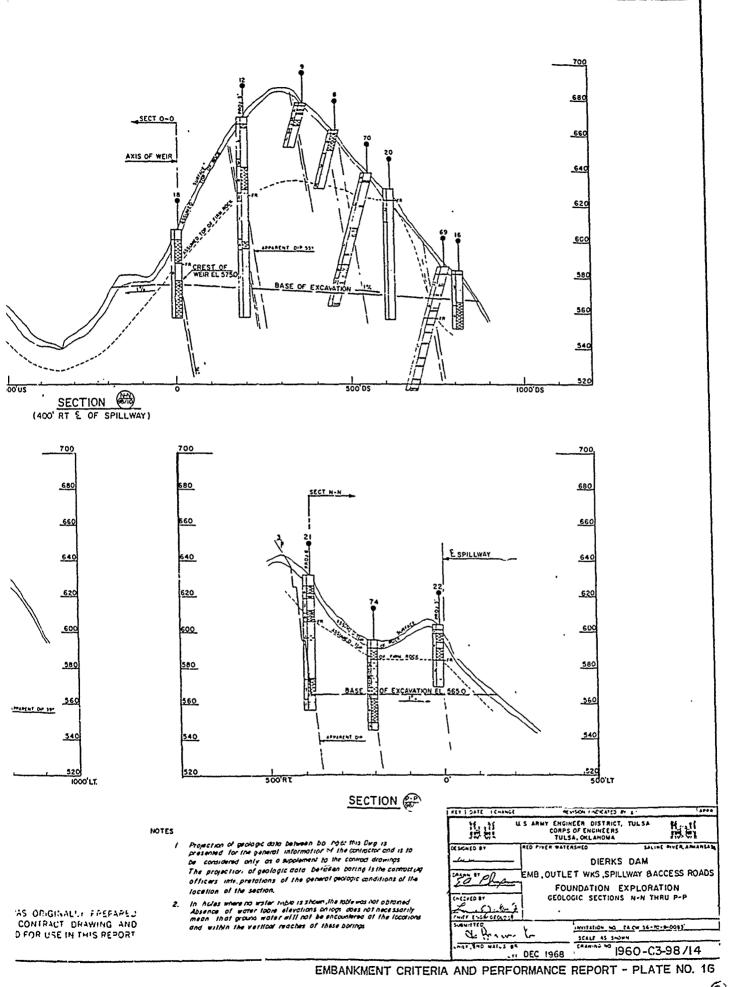
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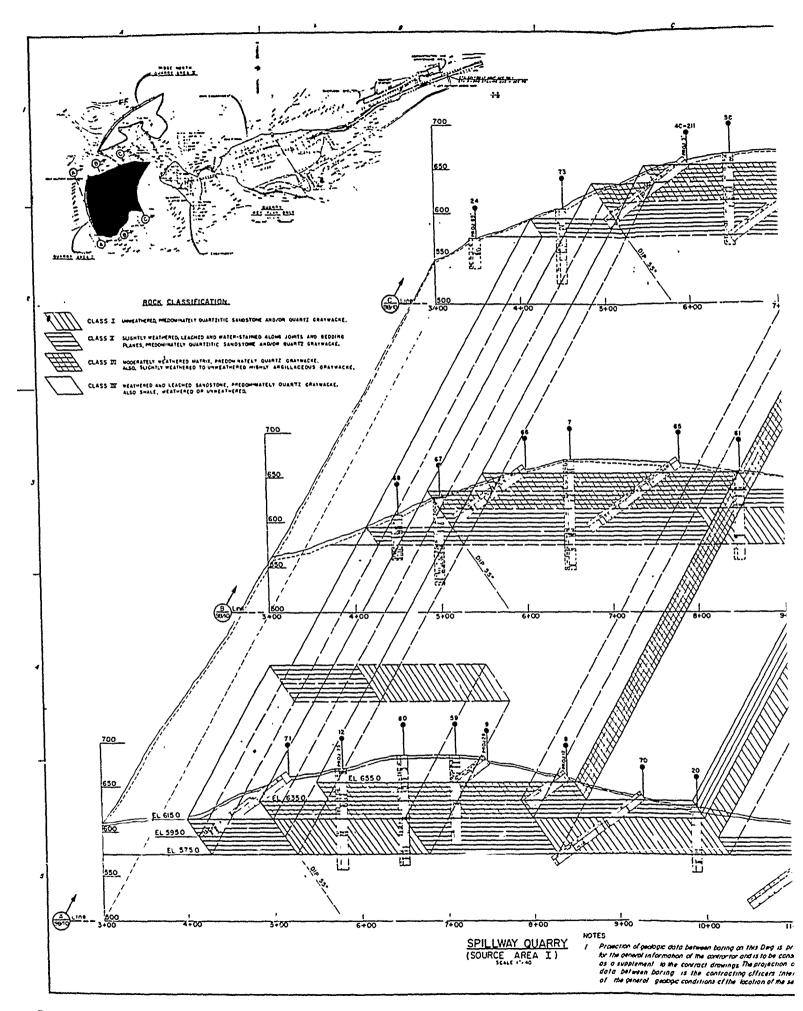
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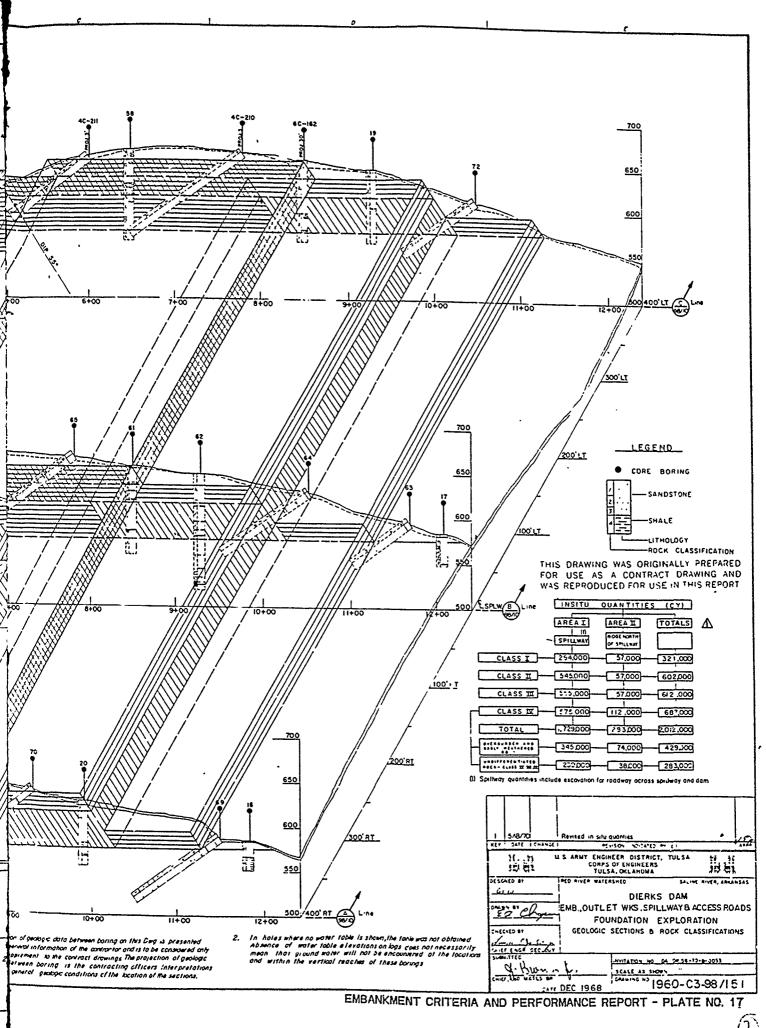


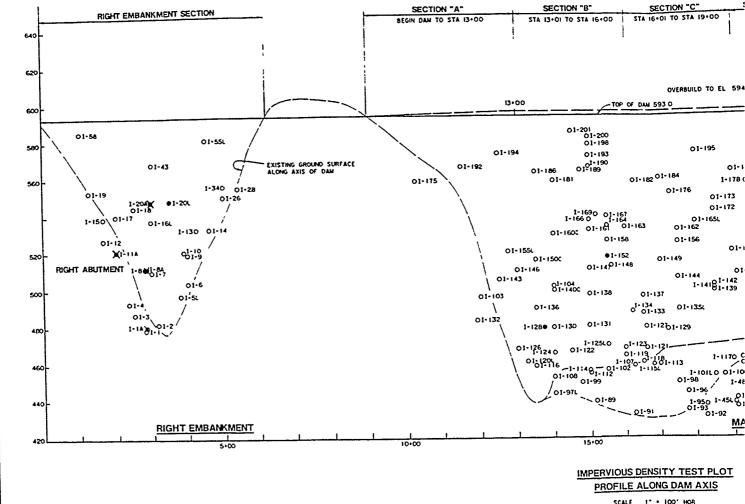


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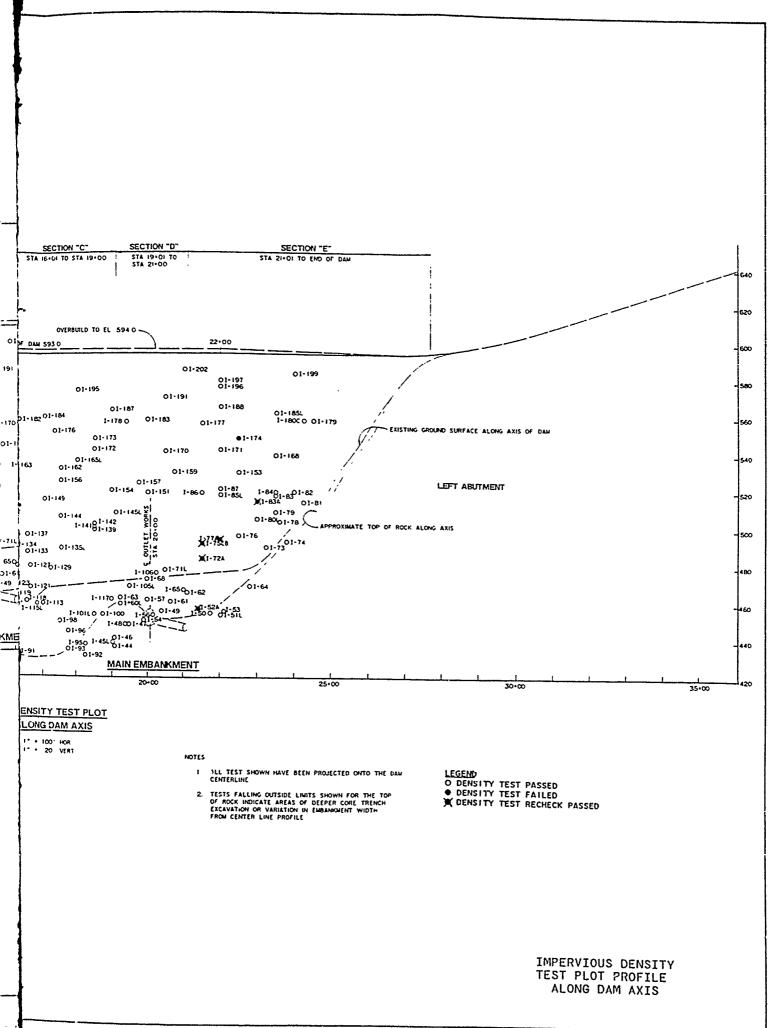


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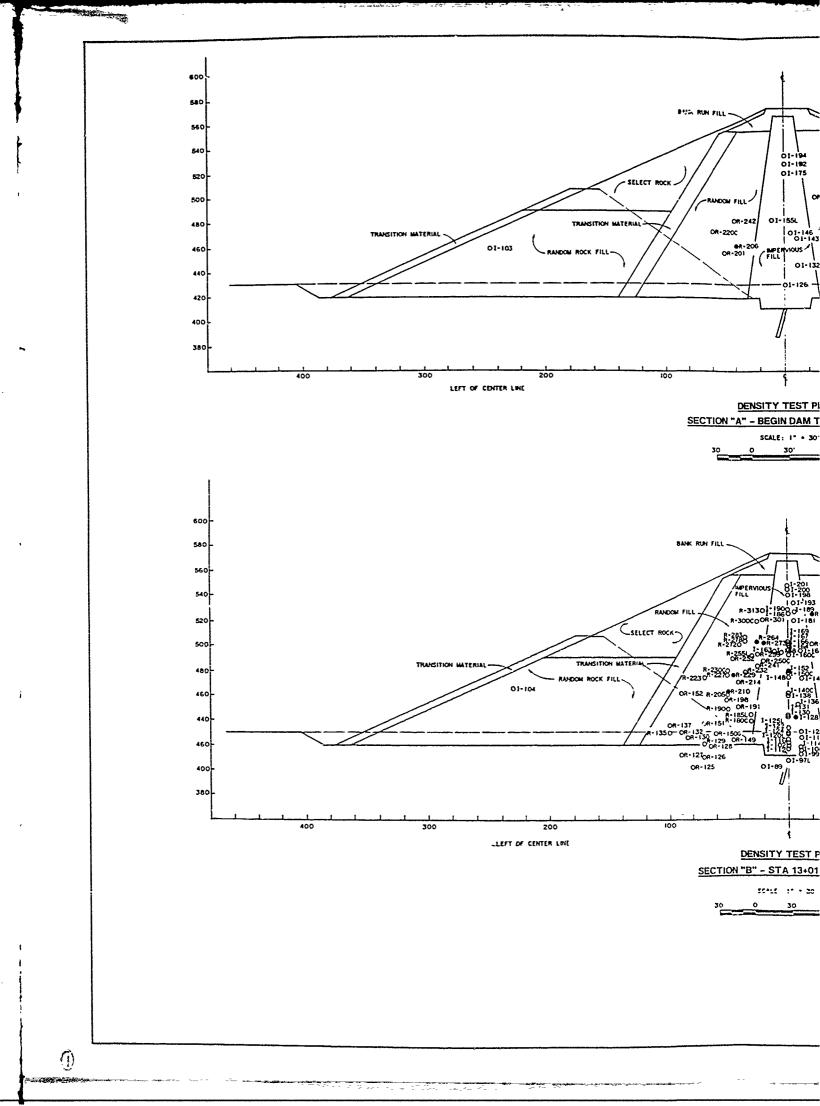


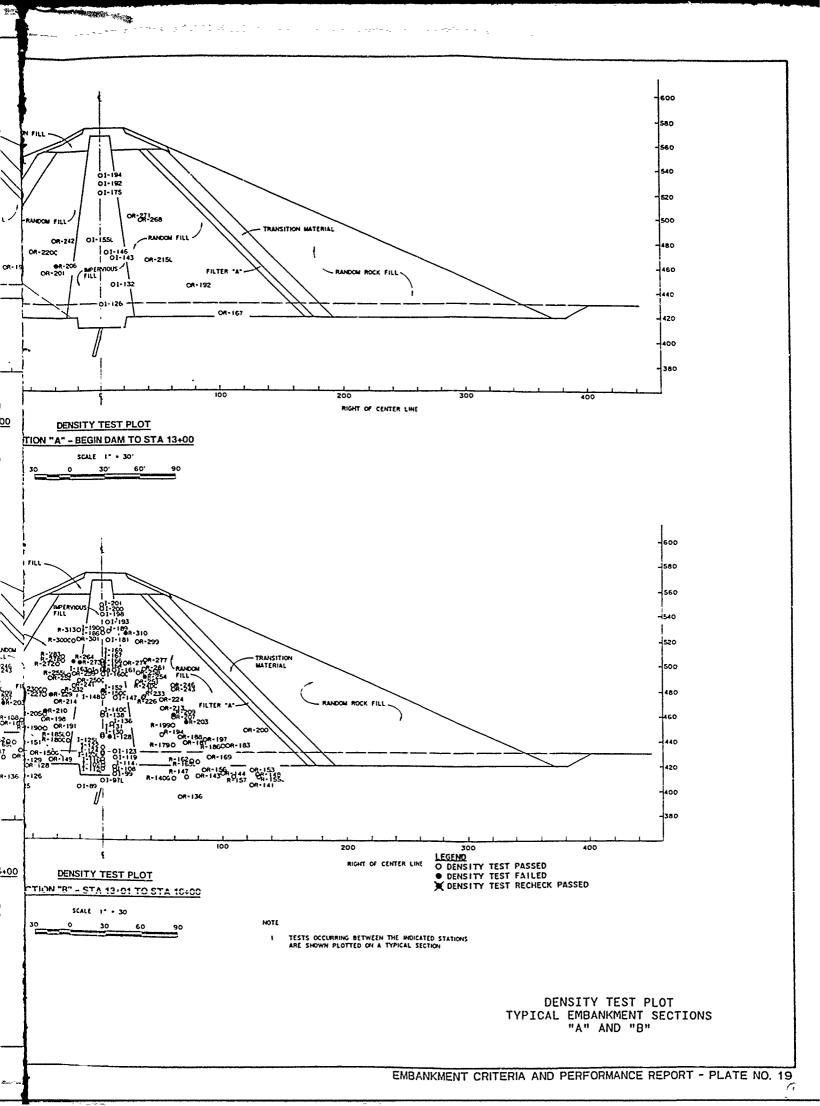


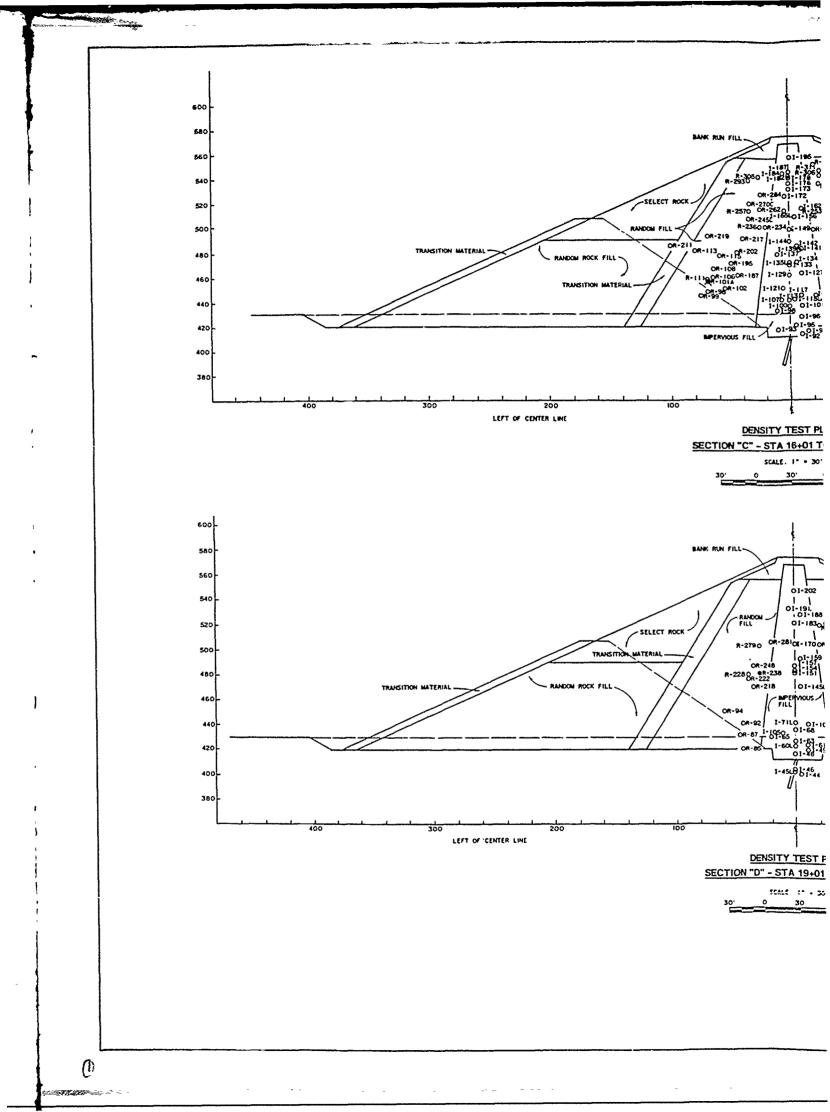
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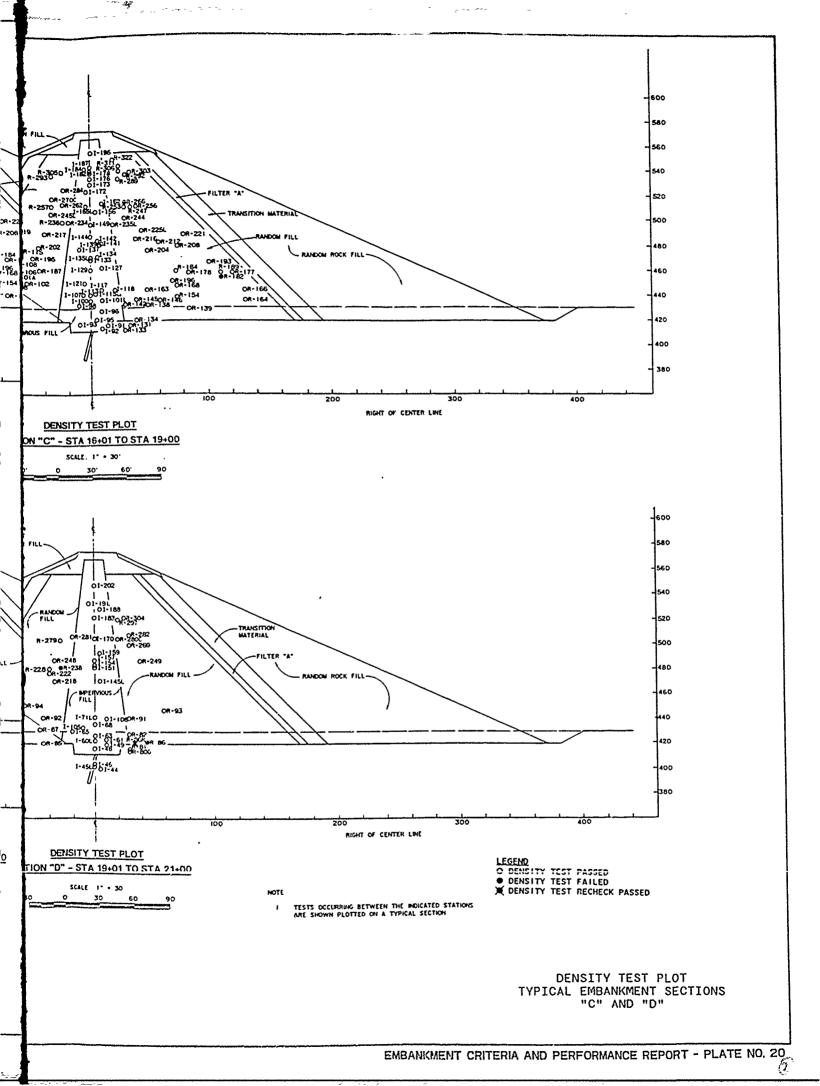


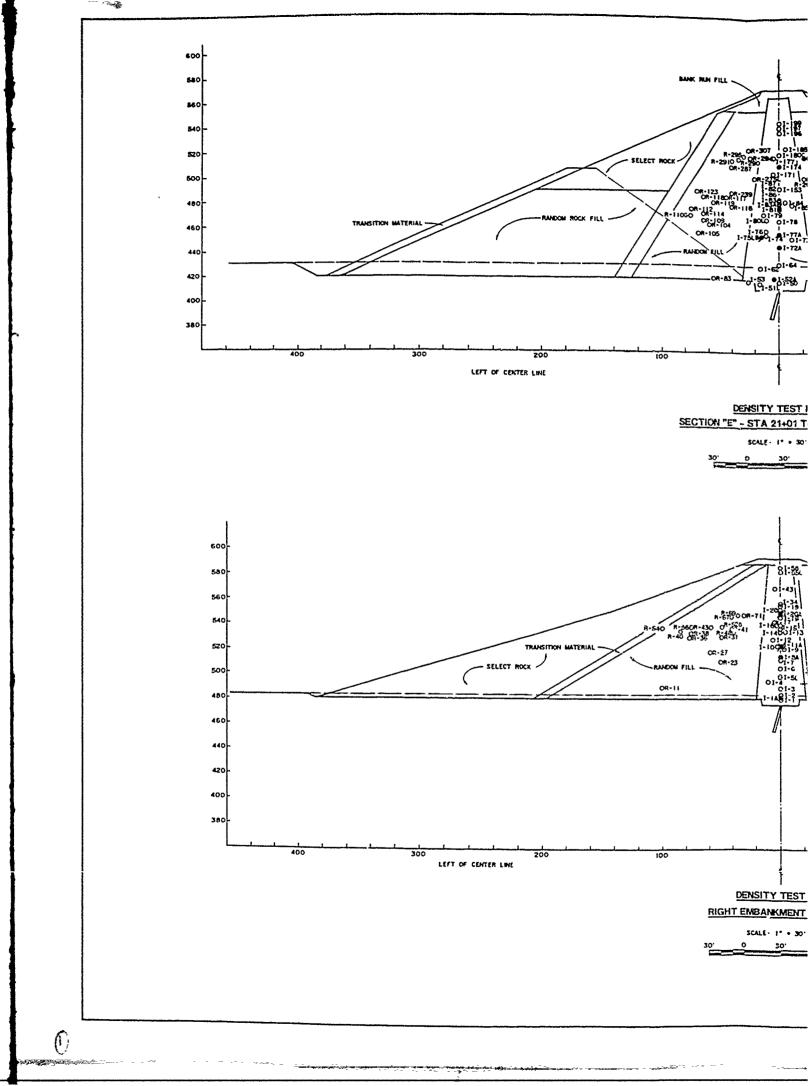
EMBANKMENT CRITERIA AND PERFORMANCE REPORT - PLATE NO. 18



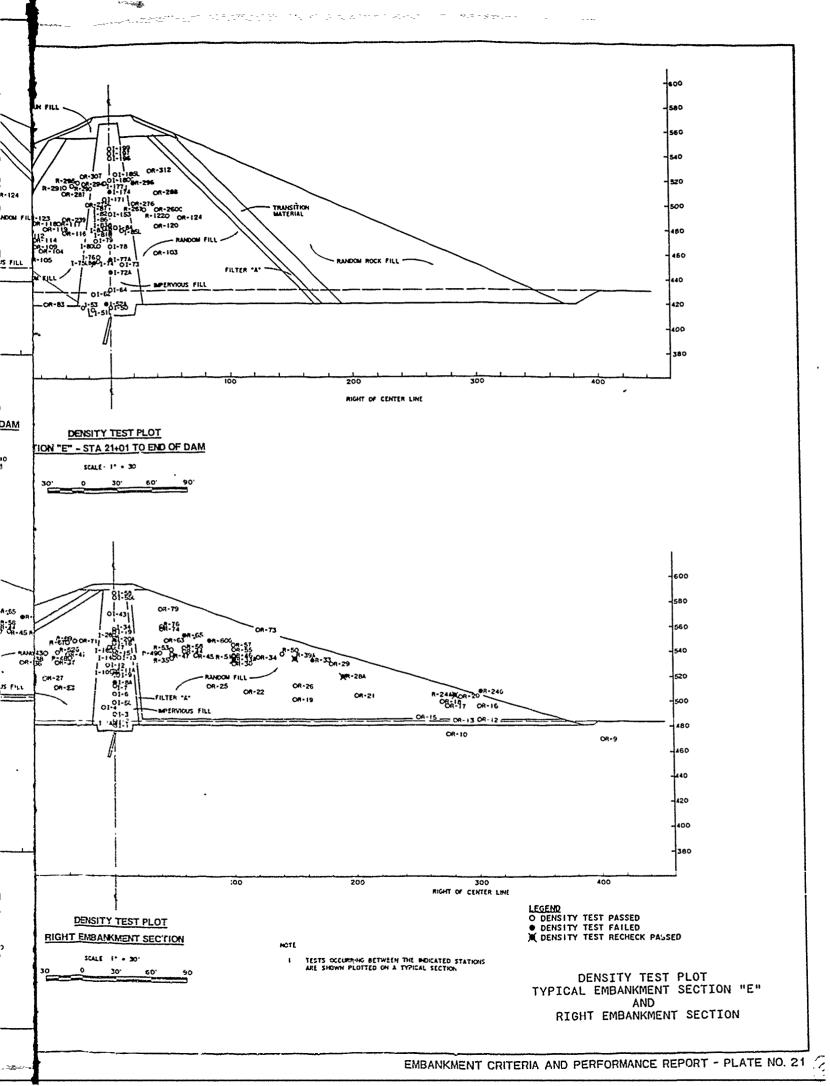




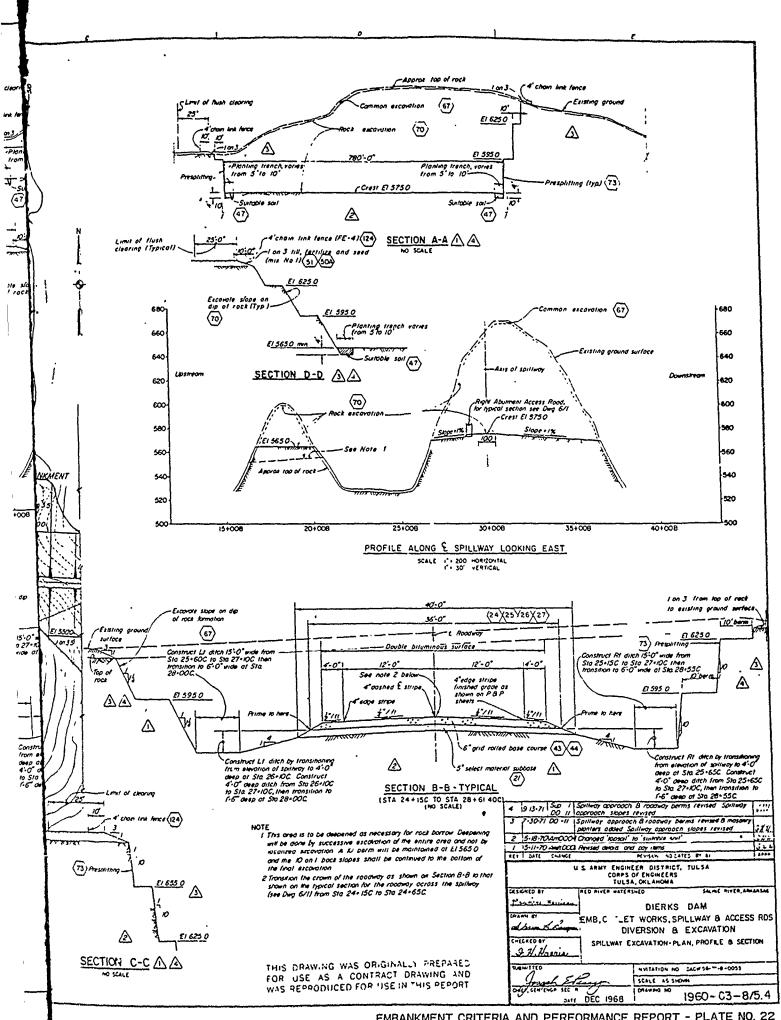


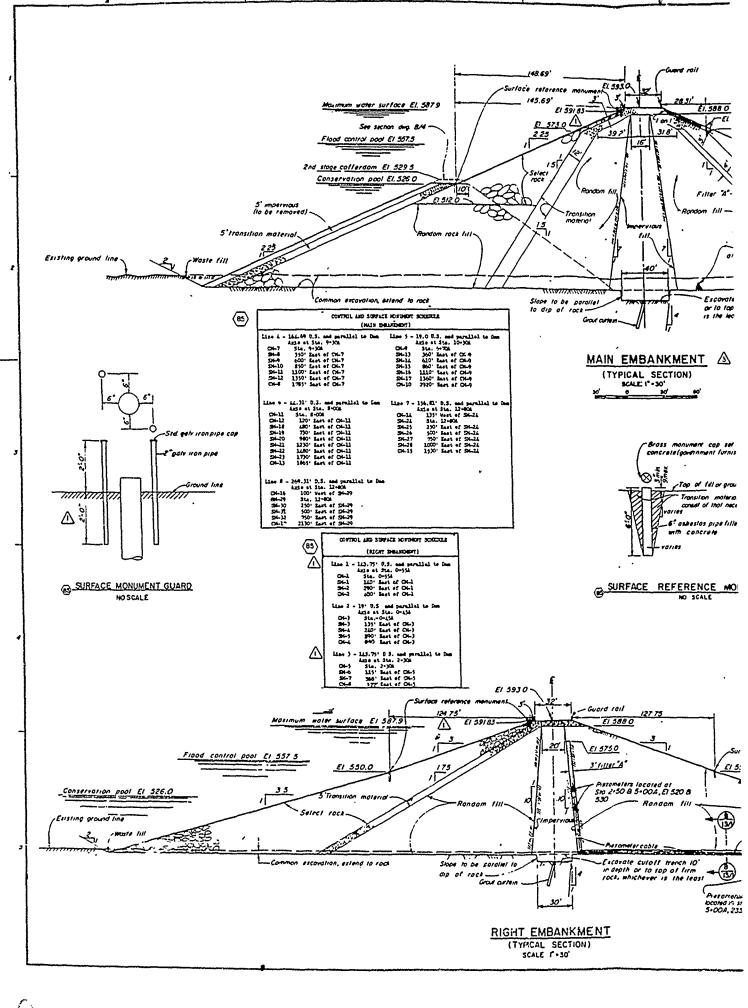


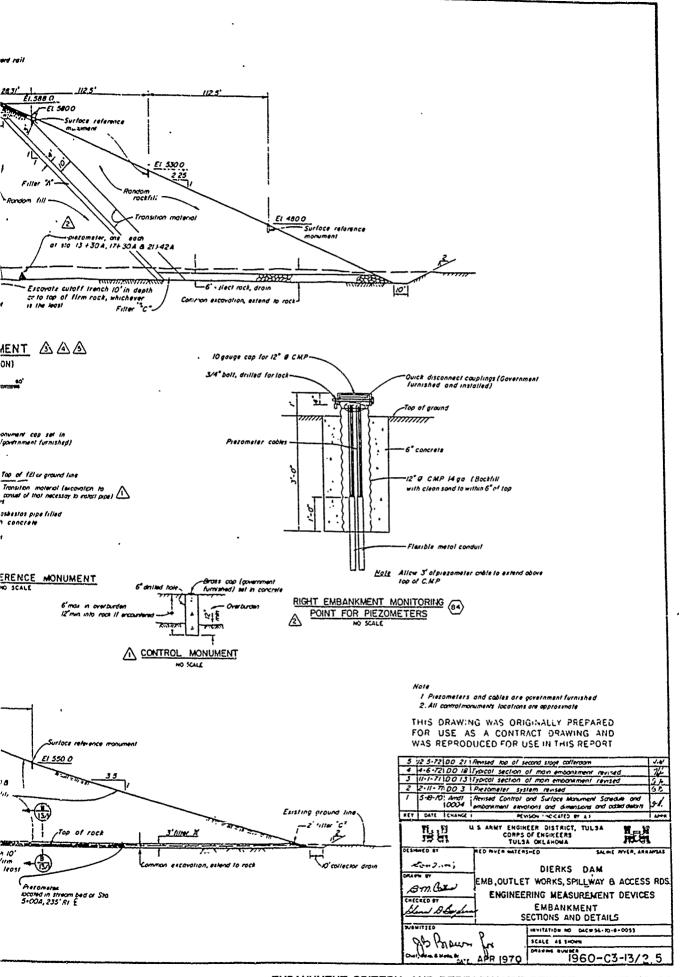
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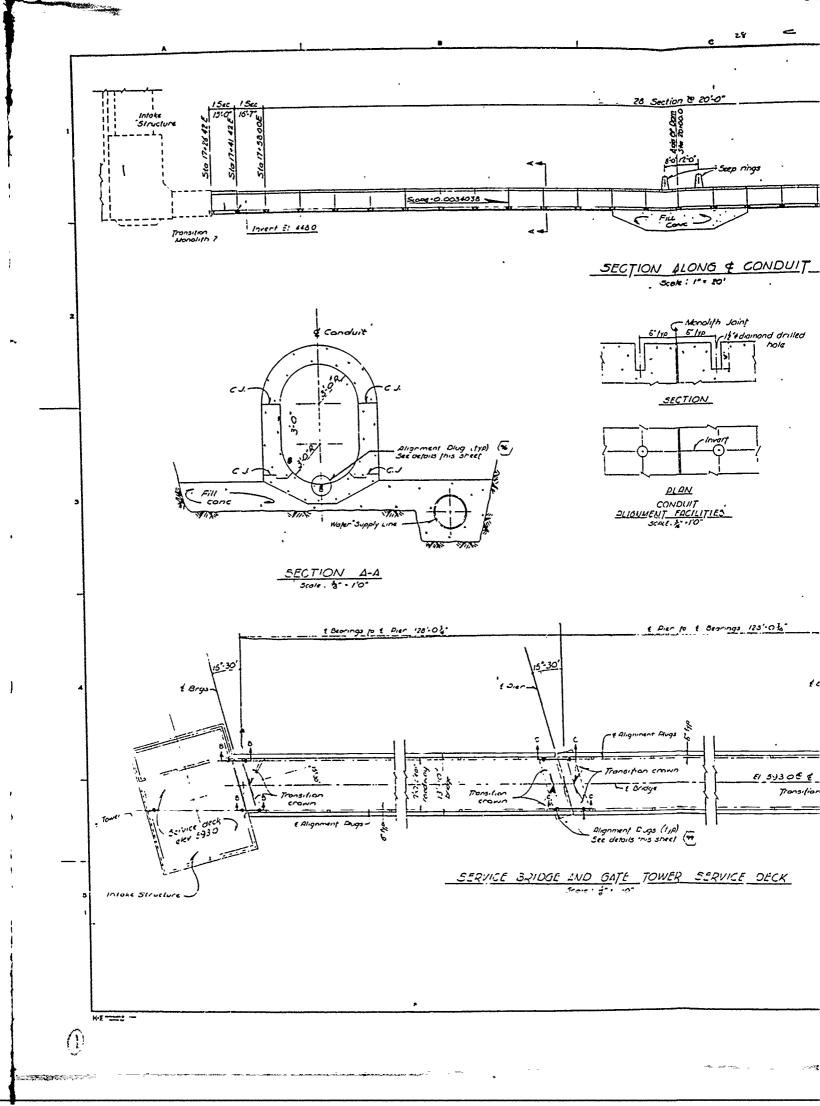


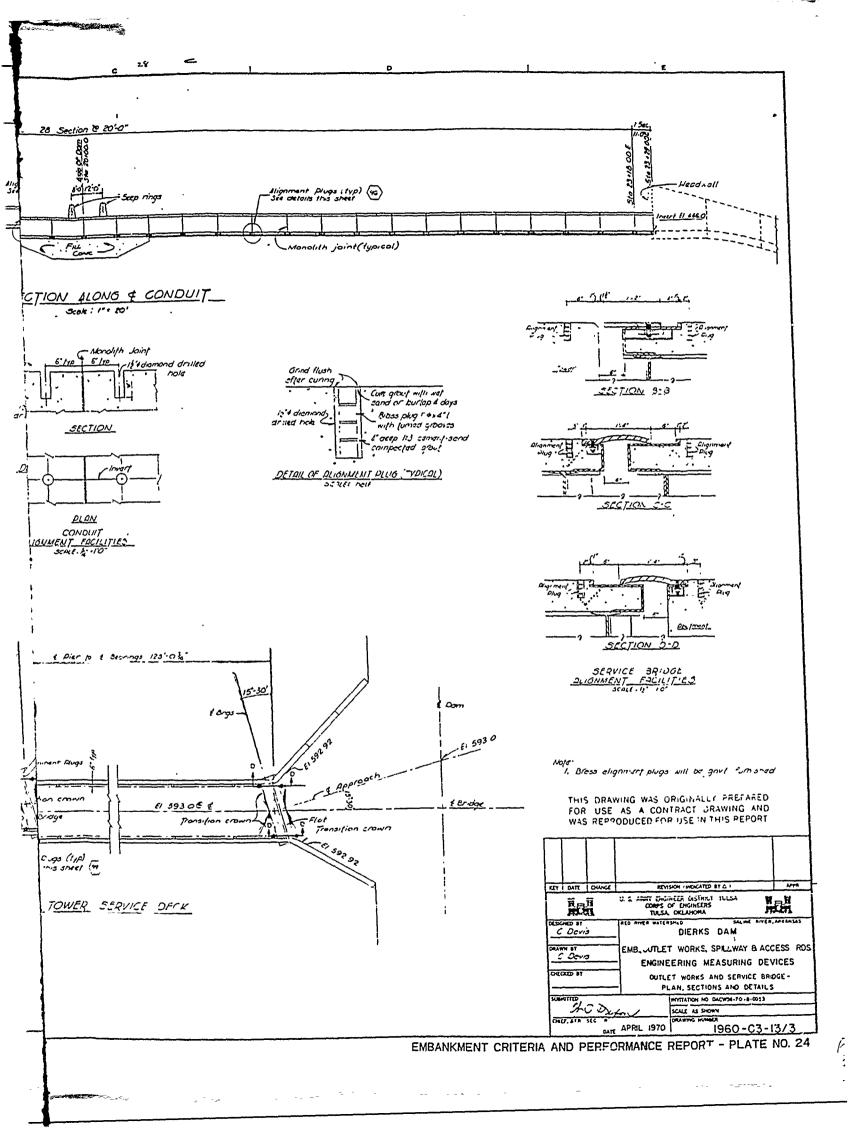
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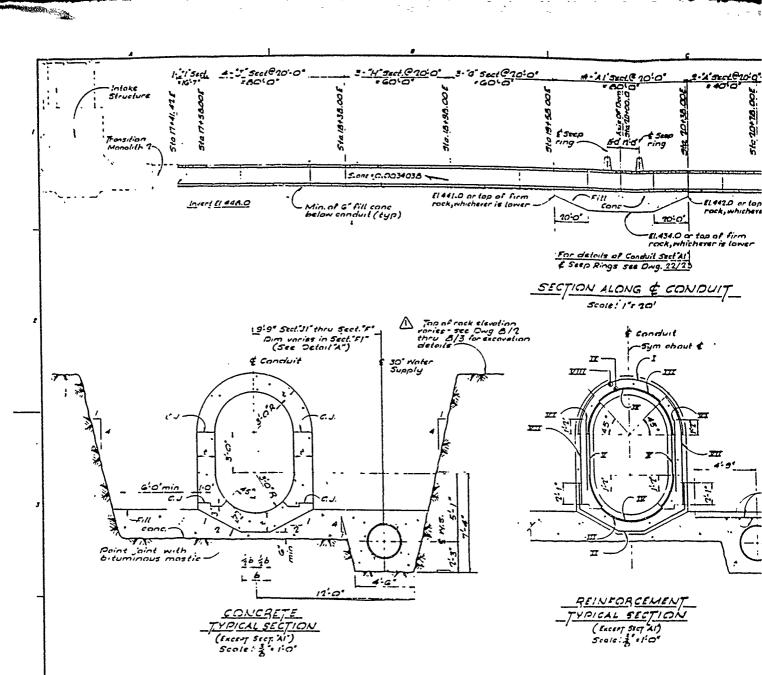












	NO.			DULE
SECTION	SECTIONS REQ'D	Ь	ŧ	-ENGTH
A	2.	2:2	1:7*	20:0
5	3	2:2	1:50	20:0"
с	3	2:2	1:5"	20:0
۵	4	20	1:2	10:0
F	2	2:0'	1:1	20:0
G	5	7:7'	1:7"	20:00
н	3	3,3,	1:5	20:0
7	. 4	7:1	1:5	20:0
FI	· /	2:0	1.2	11:0
J1	,	7.2	1:5	16-70
AI	: 4	520	: 2	19.22/2

			<u> </u>	REINFOR	CEMENT	5CHED	IULE	
COTION	NO. SECTIONS	r	п	: <i>1</i> 227	<i>1</i> 27	x	327	327
10,	REQ'D	SIZE & SPACING	SIZE & SPACING	SIZA & SPACING	5'ZE #SPACING	SITE & SPACING	SIZE & SPACING	15/11/
4	2	" 6C 12"	"AC 17"	#7 C 12"	#88 12°	#11@ 12°	40 C 12'	00
8	3	*B \$ 12"	€ 6 12°	7 € 12	#7 @ 12'	* 12°	#7 @ 12°	000
د	. 3	*se 12'	1 00 IT	60 12°	-GC 12	•6 e 17	#7 6 M'	#36
D	4	*BC 12'	*BC 17"	1 #G @ 12"	" se 12"	1 .9 e 12'	₹7 € 12'	100
1	2	ese m'	*ae 12°	#5@ 12°	#5 @ 12°	oce m'	€6 € 12'	000
G	· 3	1 "AC 12"	"BC 17"	#6C 12"	øre n'	#9 3 12°	#9 @ 12º	080
H	3	1 "BC 17	*60 IT	*se 12°	€7 € 12°	10€ 120	#:58 12°	150
5	٠ 4	l'ac n'	1 0 C 12	#5C 12"	*6 e 12°	199 12"	*9 @ 12°	1 03
FI	. ,	1 º 60 17	. BC 17	#50 12°	*5 C 12"	*60 12°	#5 9 12°	18
J1	' /	"6€ 12° -	1 00 12°	# 58 120	P & B 12"	■96 M'.	, #9 8 12"	*B
AI	4	See Dwg.	22/23		<del></del>		· <del></del>	

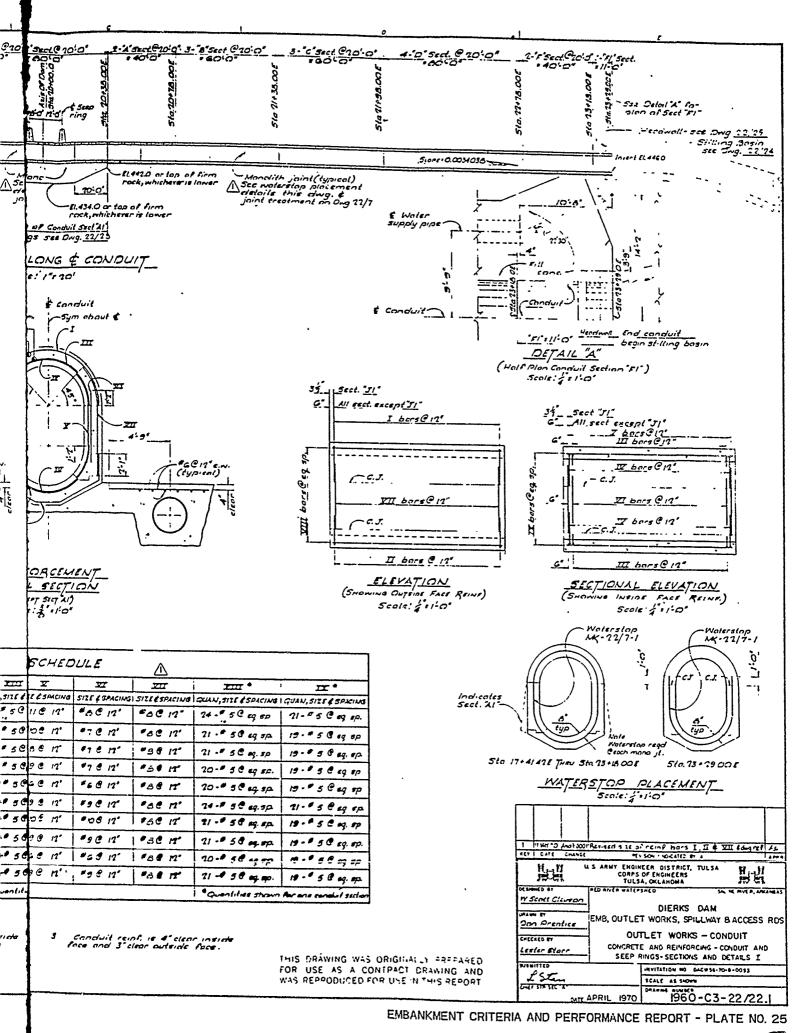
# Norse:

- For Several Vores and Severa. Re wroscenent Notes see Dag 11/1 Coveres a Concrete for conduct sections will be poid for under Bid Item 3G b. Concrete for seep rings under Bid Item -101

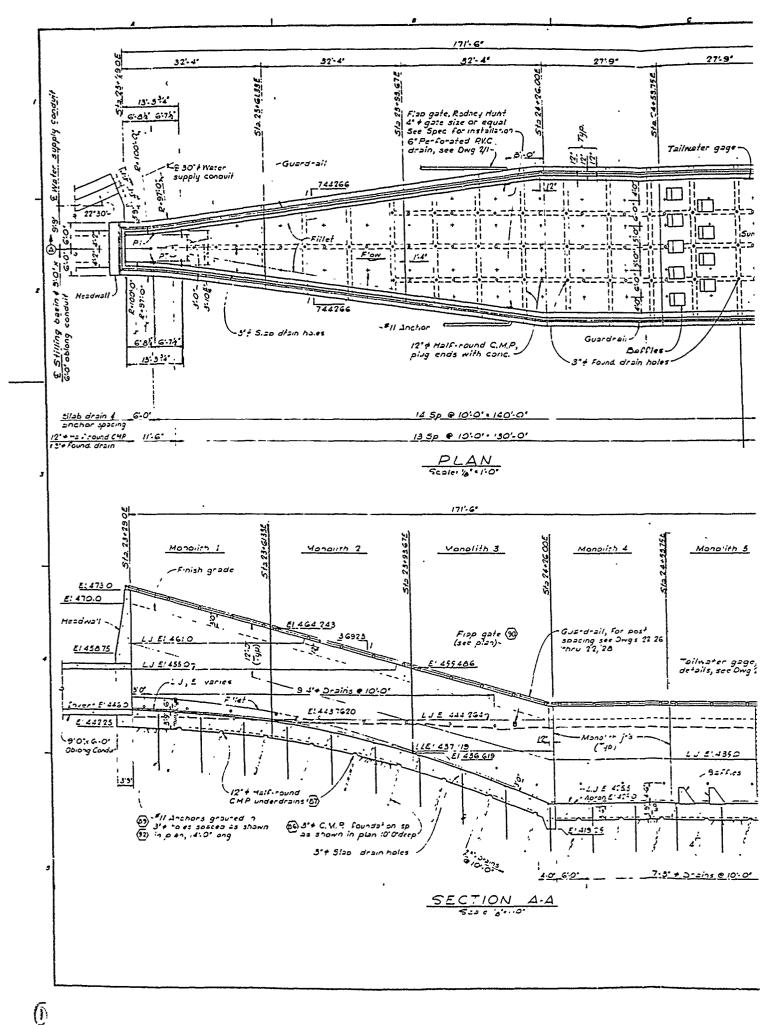
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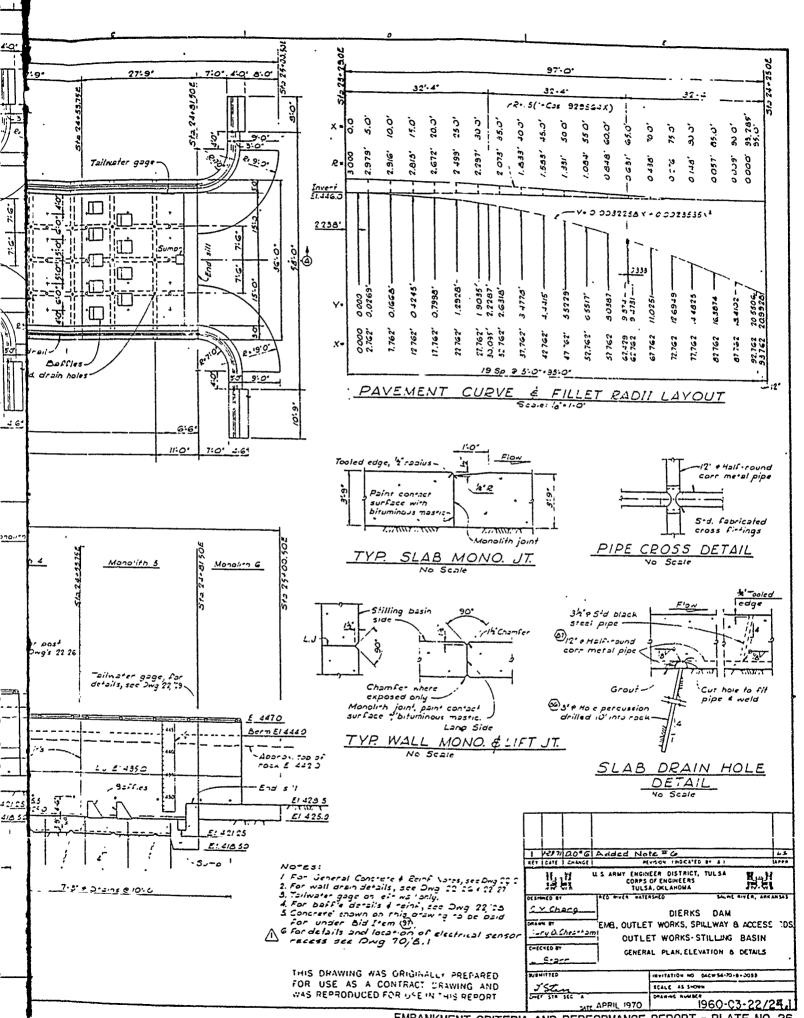
- c Fill concrete when used as conduit hase and water supply pipe encasement under Sid Item ()—101 d fill concrete when used as uscavation fill under Sid Item ()—101
- Conduct reinf, is 4 face and 3" clear out

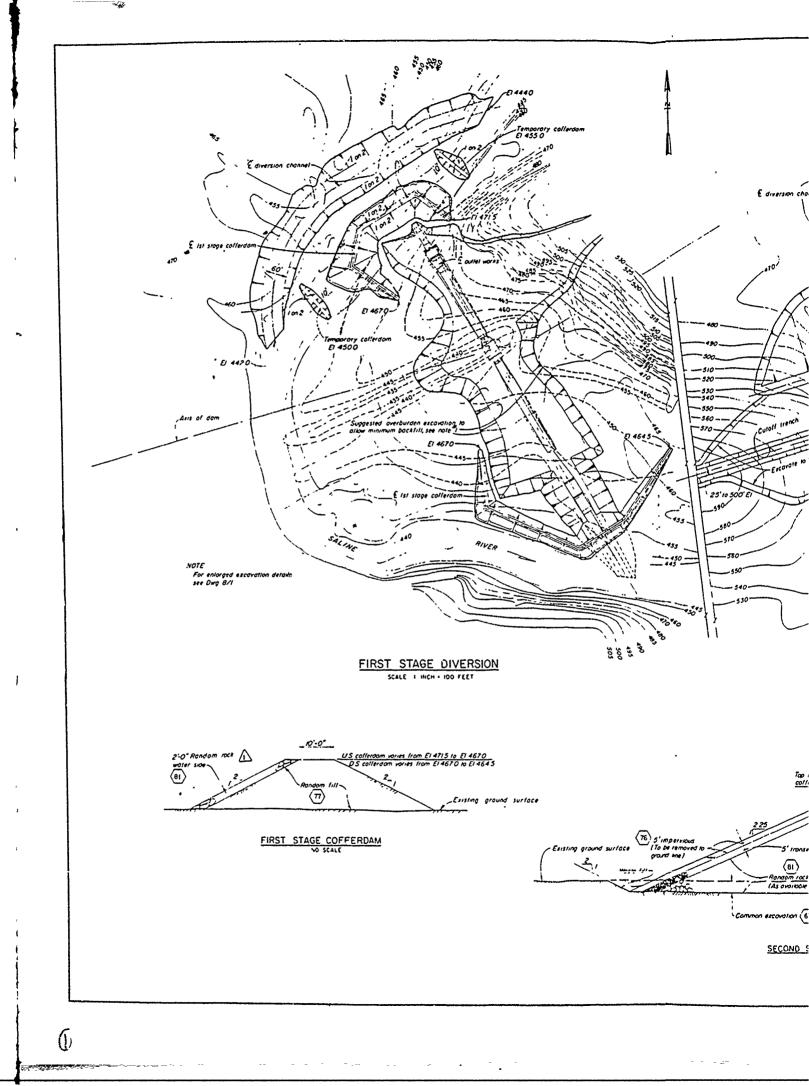


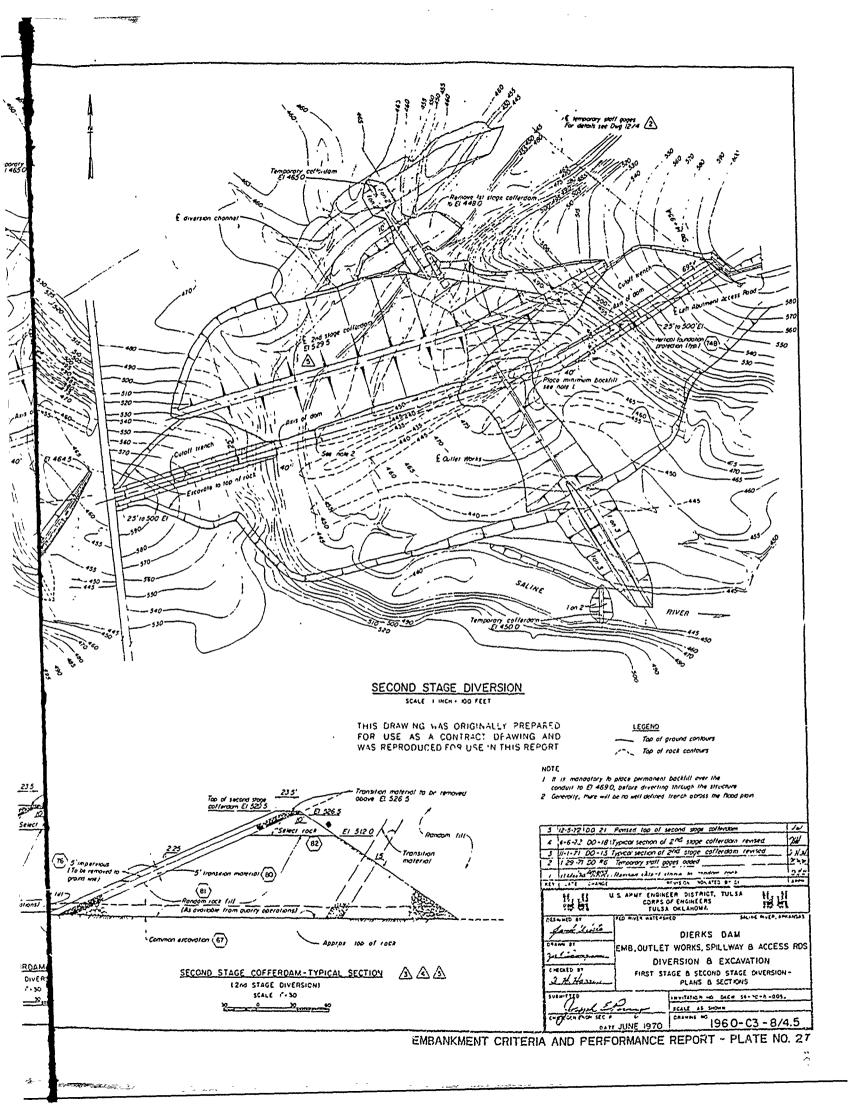


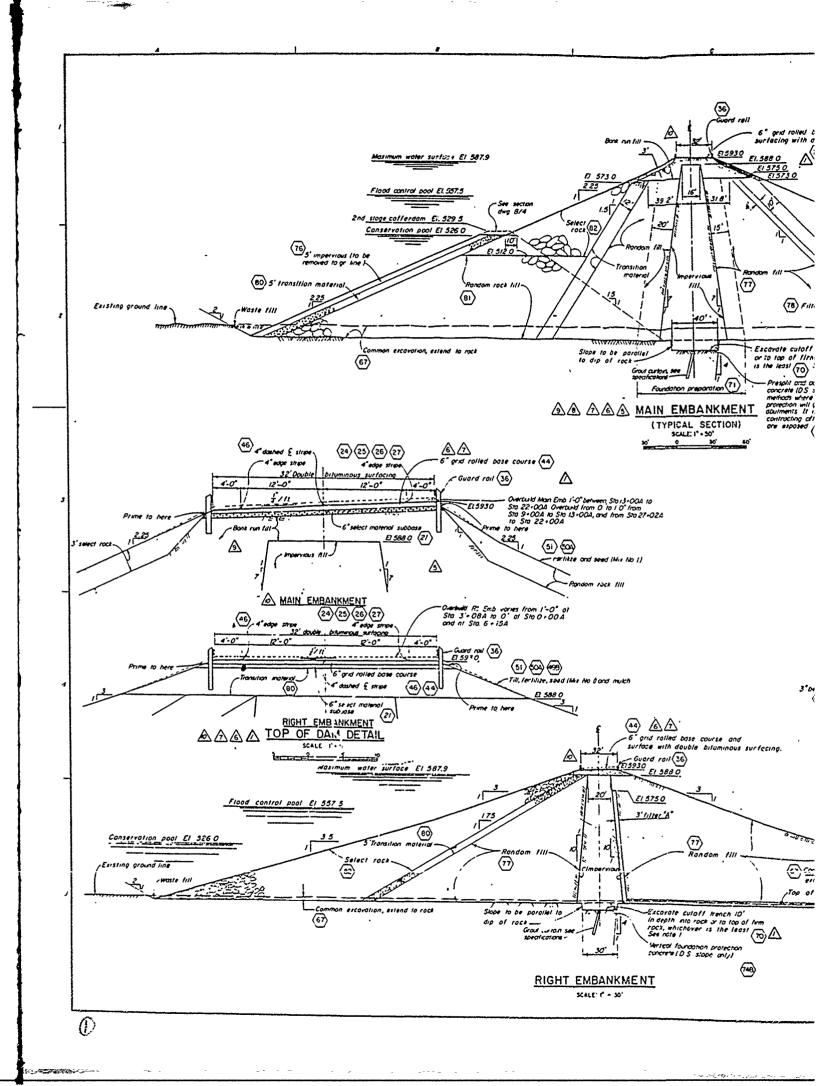
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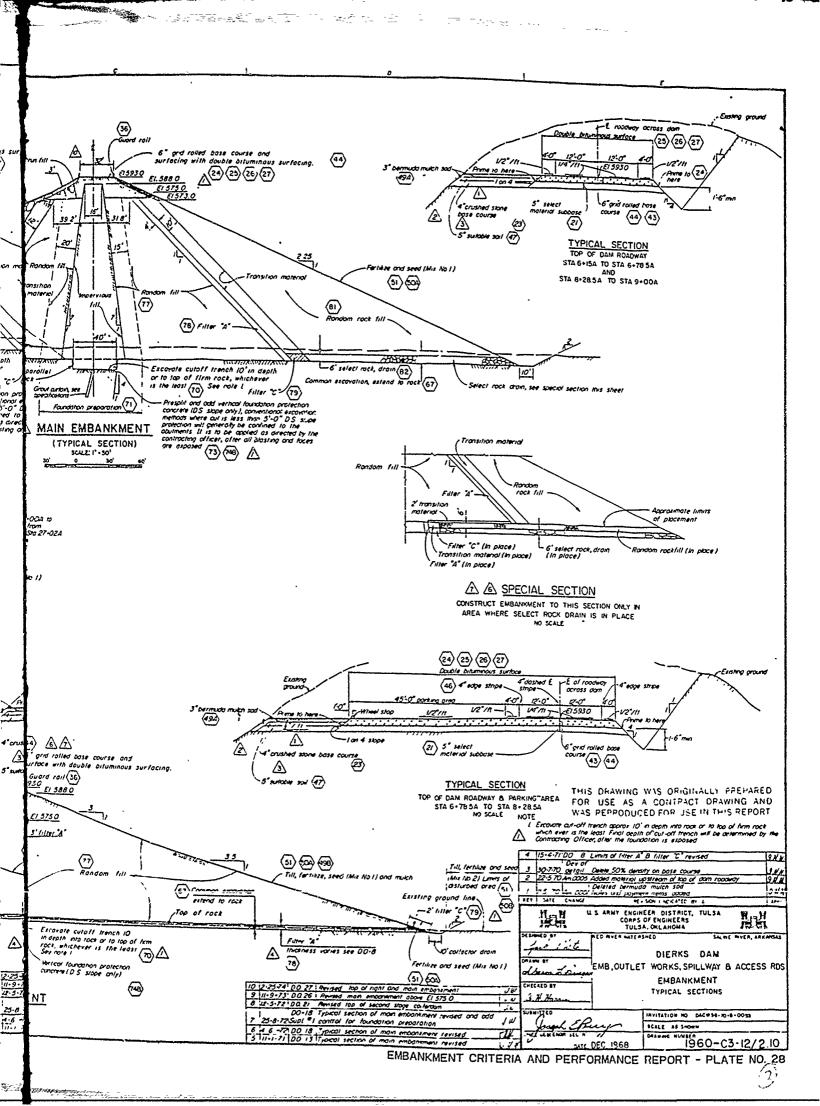


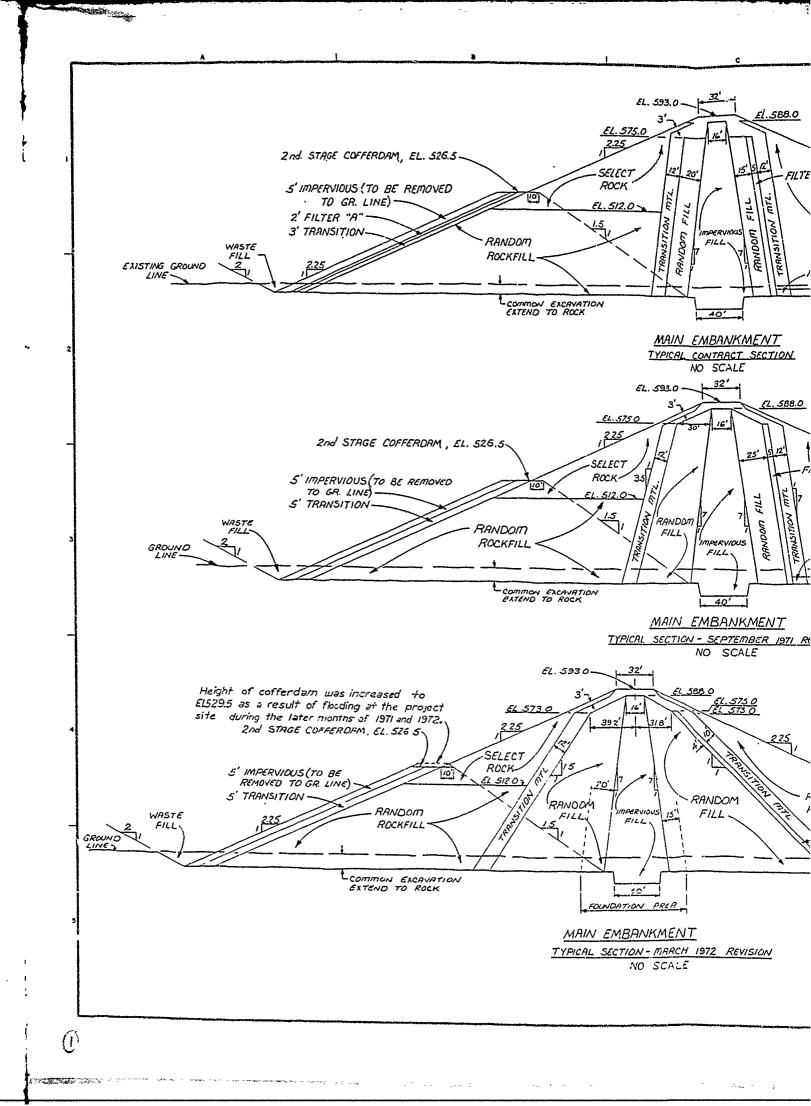


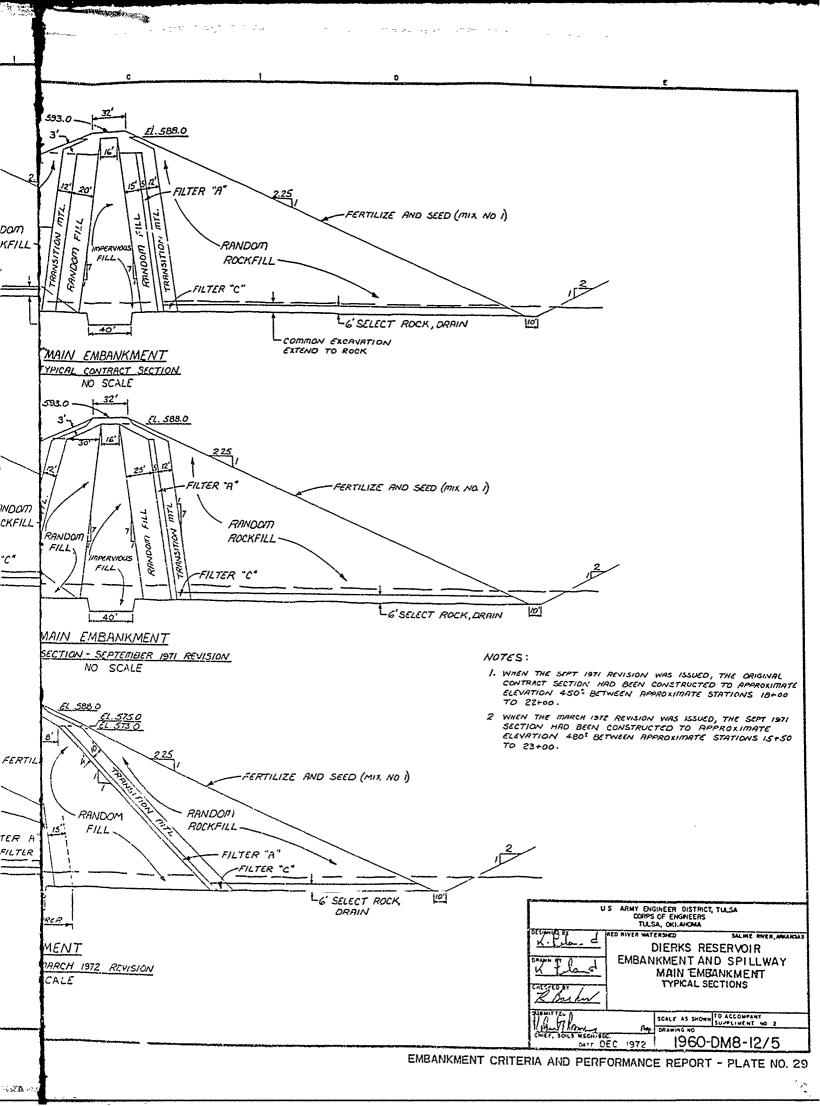


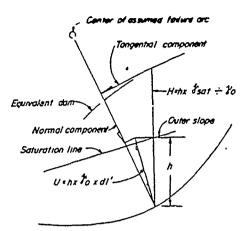












TYPICAL FORCE DIAGRAM

AREA SCALE Norm Area-B =08 sq.in SUMMATION OF NORMAL TANGENTIAL FORCES Area-G = 0.5 sg.m. Center of failure arc Max pool El 5879 E1.5500 -Select Cors. pool EL 526.0 TIXITYIIXII

SAFETY FACTOR SUMMARY				
Rodius	Coords ( center	Safety factor		
,(leel)	X(ft) Y(ft)		100101	
6/	-50	9	187	
68	-59	5 ~	1.74	
94 97	-85 -107	20 6	150 155	
104	-85	54	177	
108	-112	42	176	
140	-/38	54	149	
222	-162	137	141	
301	-168	229	173	
3/0	-232	219	181	

## NOTES:

The above table includes results of all arcs analyzed for the sudden drawdown condition Coordinates for arc senters are referenced from the E and crest of the dom

STABILITY ANALYSIS OF UPSTREAM SLOPE

CONDITION OF SUDDEN DRAWDOV

-- Random rock

SCALE' 1" . 30"

/ sq in 28.125

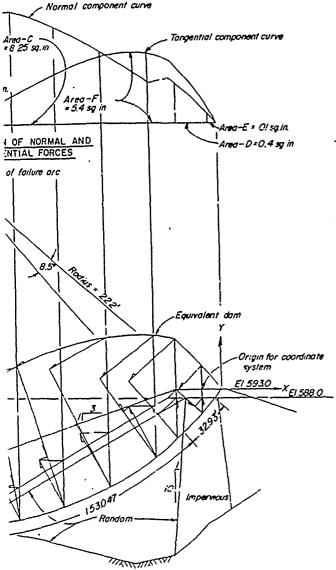
Tons

Struck Control

4. In .125 nns

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SCALE



A	OPTED I	DESIGN D	ATA		
Materials	Unitweight	Sheor strengths #			
•	Saturated	Submerged	a	R	S
Select rock	135	72.6		#=35° C=0	C=0
Random rock	135	726	#=33°	ø=33°	
Transition matt	135	726		0=33°	0-33° C=0
Impervious	125	62.5	0=10°	0=140	C=0
Random	125	62.5	1010	0=R	1.29

C = Cohesion (Tons /ft2)

#### FORCES ACTING ON FAILURE ARC

Positive tangential force Area – F = 151 875 Tons Negative tangential force Area – G = 14 063 Tons Total = 137 812

#### NORMAL AND RESISTING FORCES

## SAFETY FACTOR COMPUTATIONS

Formula SF. = \(\frac{\xiN \times \times \LC}{\xi\T}\)

For consolidated undrained (R) condition

S.F. 75 938xTan 3942278/x Tan 334 232 03/x Tan 12°+11 250x Tan 14°+64 509

SF * 140

THIS DRAWING WAS ORIGINALLY PRÉPARED FOR USE IN A DESIGN MEMORANDUM AND WAS REPRODUCED FOR USE IN THIS REPORT

US ARMY ENGINEER DISTRICT, TULSA

CORPS OF ENGNEERS

TULSA, CKLAPOMA

TULSA, CKLAPOMA

DIERKS DAM AND RESERVOIR

RIGHT EMBANKMENT

UPSTREAM STA. 2+50

LIGHT STABILITY ANALYSIS-SUDGEN DRAWDOWN COND.

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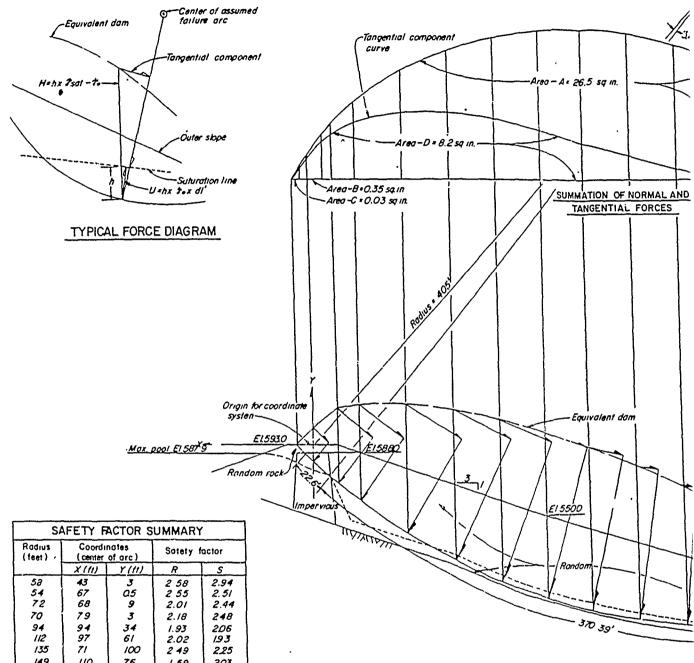
LIGHT STABILITY ANALYSI

1960-DM8-97/10

REAM SLOPES UNDER

1 DRAWDOWN

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198 179 109 1.53 2.23 1.92 1.41 1.54 

NOTES

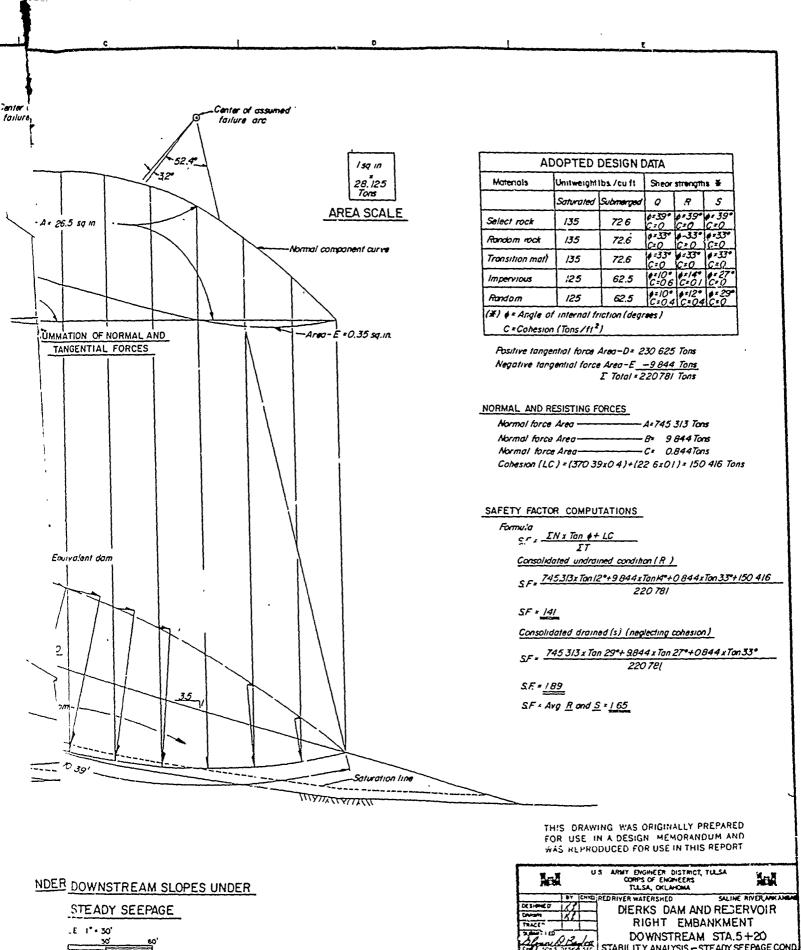
The above table , 'udes results for all arcs analyzed for the steady seepage-condition. The coordinates represent arc centers and are referenced from the  $\hat{\epsilon}$  and crest of dam

STABILITY ANALYSIS OF DOWNSTREAM S

CONDITION OF STEADY SEEPAG

SCALE ' I* • 30'

G



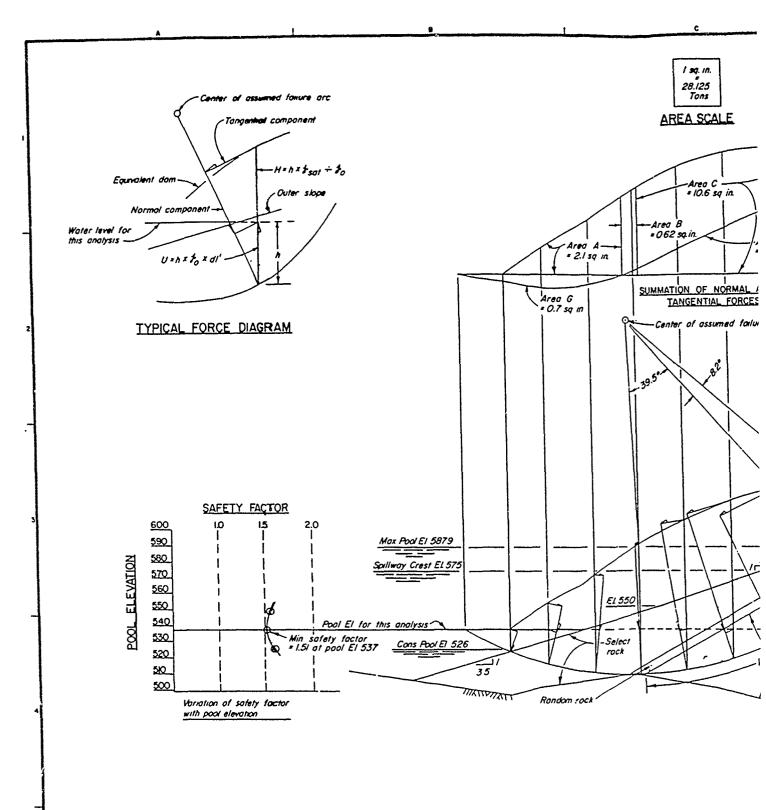
DOWNSTREAM STA.5+20

STABILITY ANALYSIS, STEADY SEPAGE COND.

APPROVED THE FORM OF THE PROPERTY NOV 1967

1960 - DM8 - 97/11

EMBANKMEN'T CRITERIA AND PERFORMANCE REPORT - PLATE NO. 31



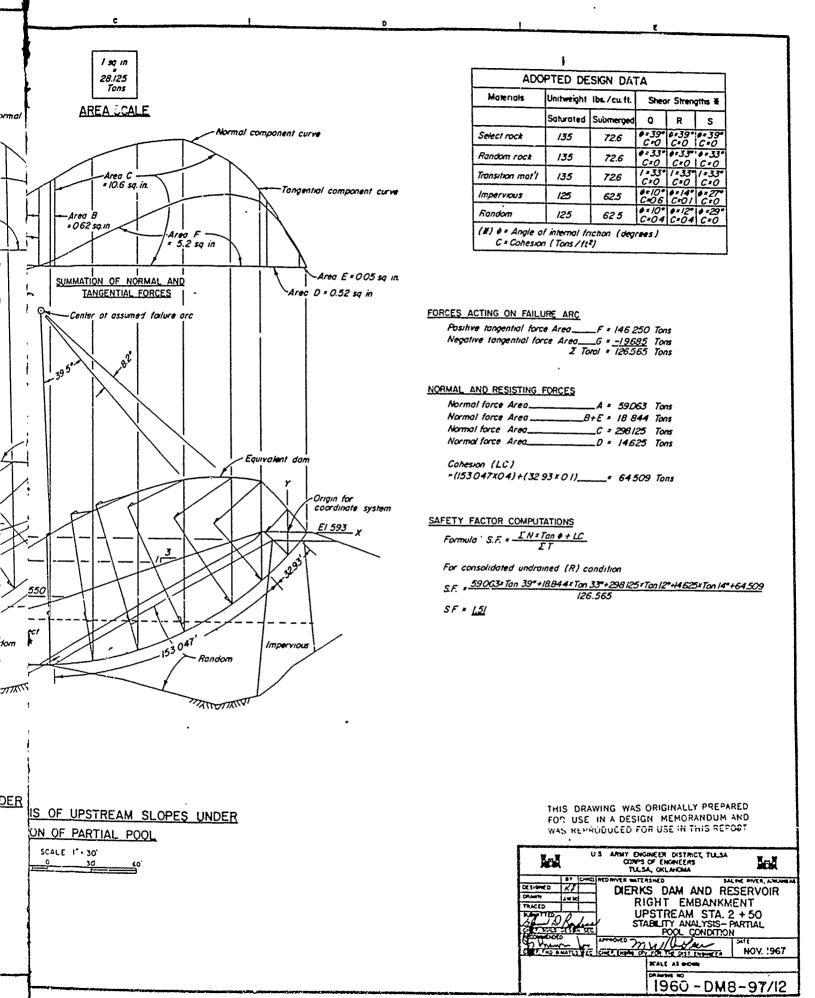
# STABILITY ANALYSIS OF UPSTREAM SLC CONDITION OF PARTIAL POOL

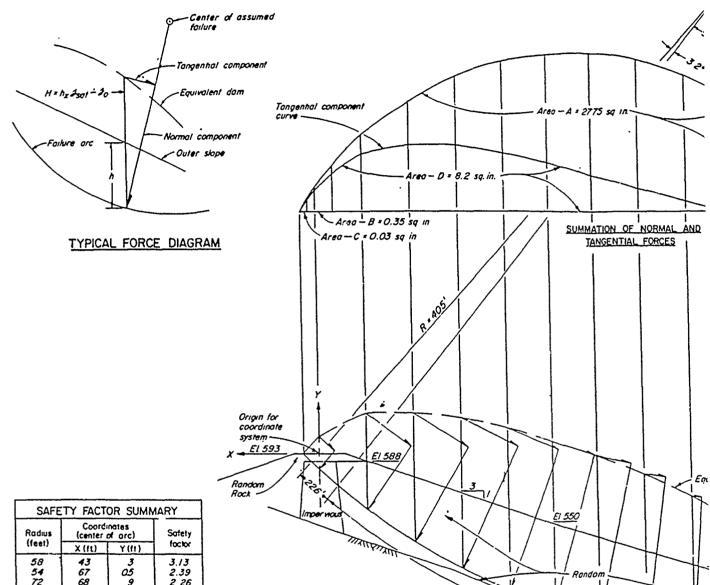
SCALE:1" + 30'

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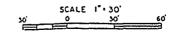
flotes

The chove toble includes results of all arcs analyzed for end of construction condition. Coordinates for arc centers are referenced from the £ and crest of the dam.

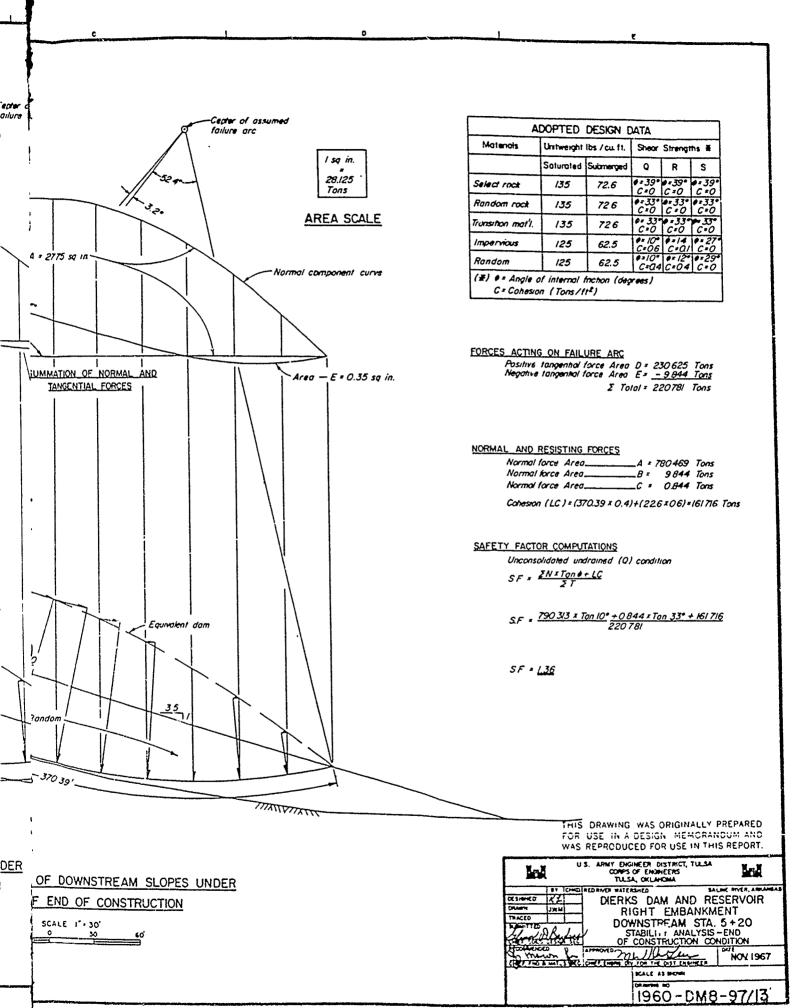
STABILITY ANALYSIS OF DOWNSTREAM SLO

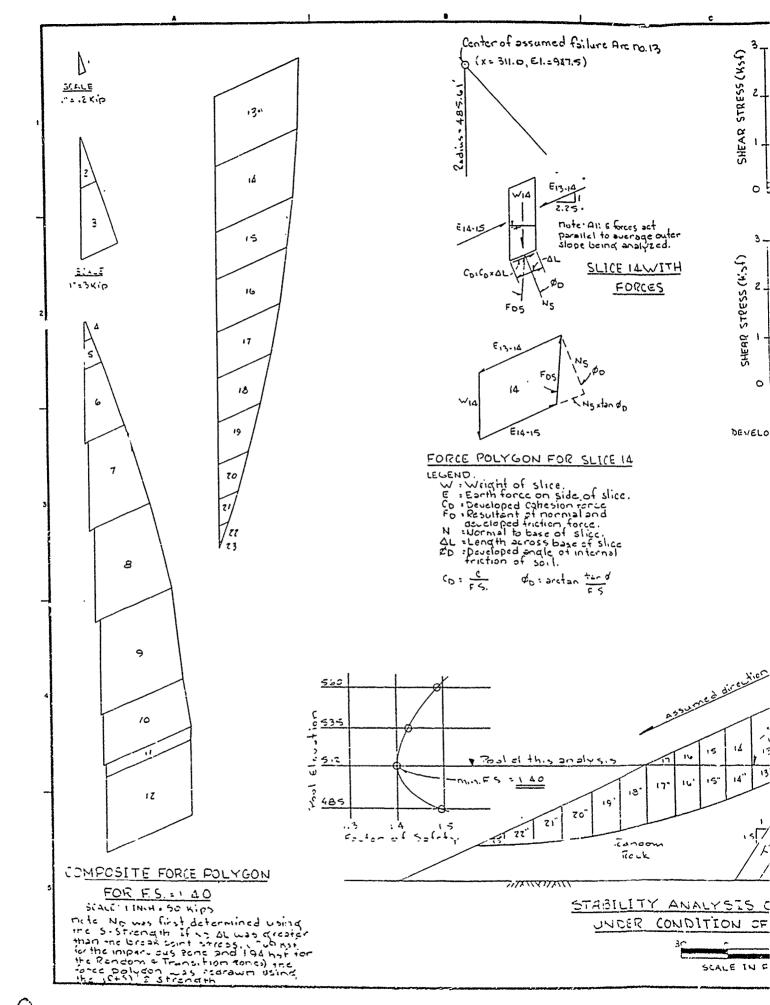
CONDITION OF END OF CONSTRUCT

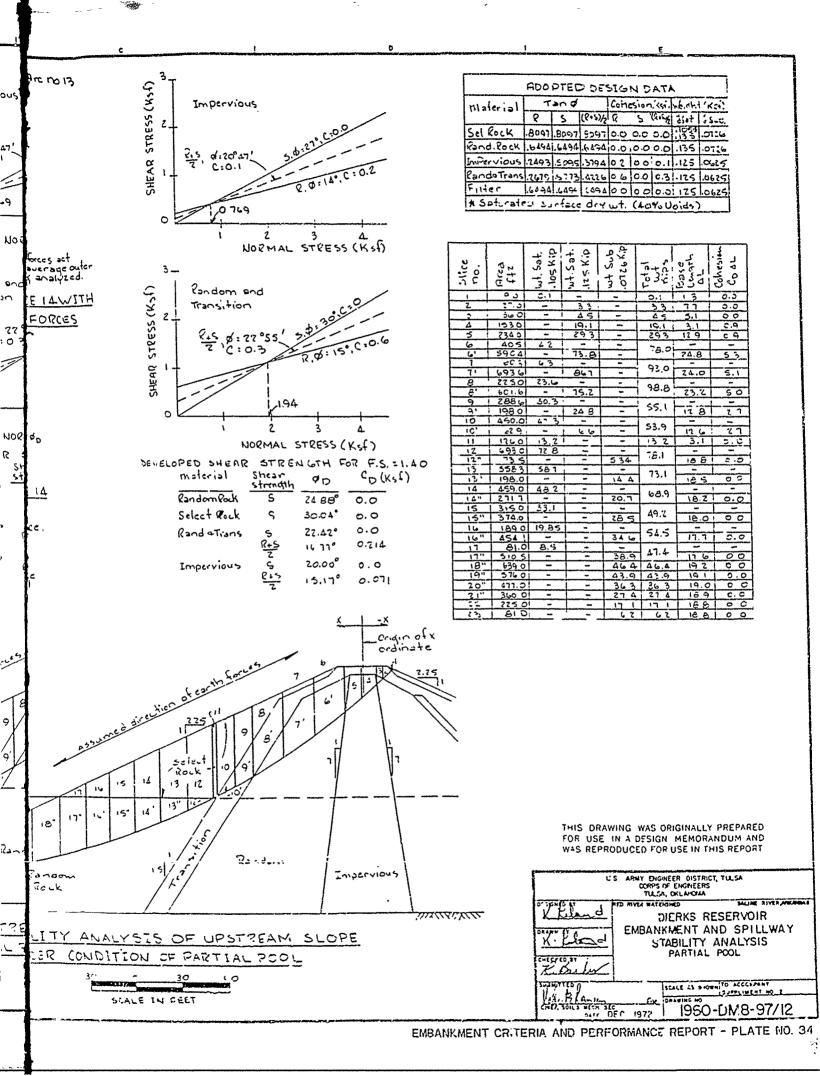
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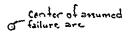


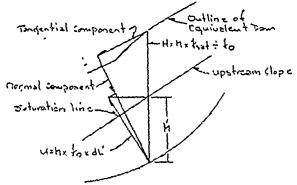
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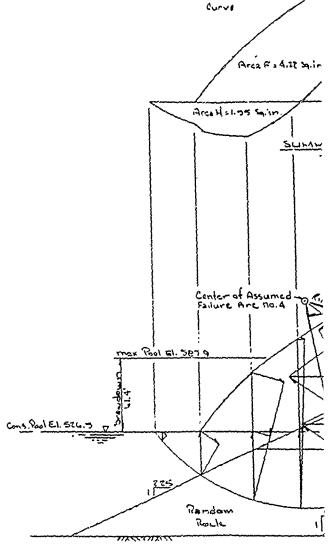






TYPICAL FORCE DIAGRAM

SAFETY FACTOR SUMMARY					
ARL	RADIUS	COORDIN RETHES)		Safety	
NO	(FEET)	X (F17)	Y (FT)	FACTOR	
1	₹35.8	-742.0	184.0	1.12	
Ζ.	509.2	-348.5	358.0	1.22	
3	217.5	-184.3	65.5	1.06	
4	171.5	-154.0	47.0	1.03	
5	485.6	-311.0	354.5	1.16	
0	276.2	-252.0	128.0	1.26	
٦	356.1	-236.5	238.0	1.08	
8	216.0	-181.0	111.5	1.10	



normal component.

STABILITY ANALYSIS OF UPSTREA

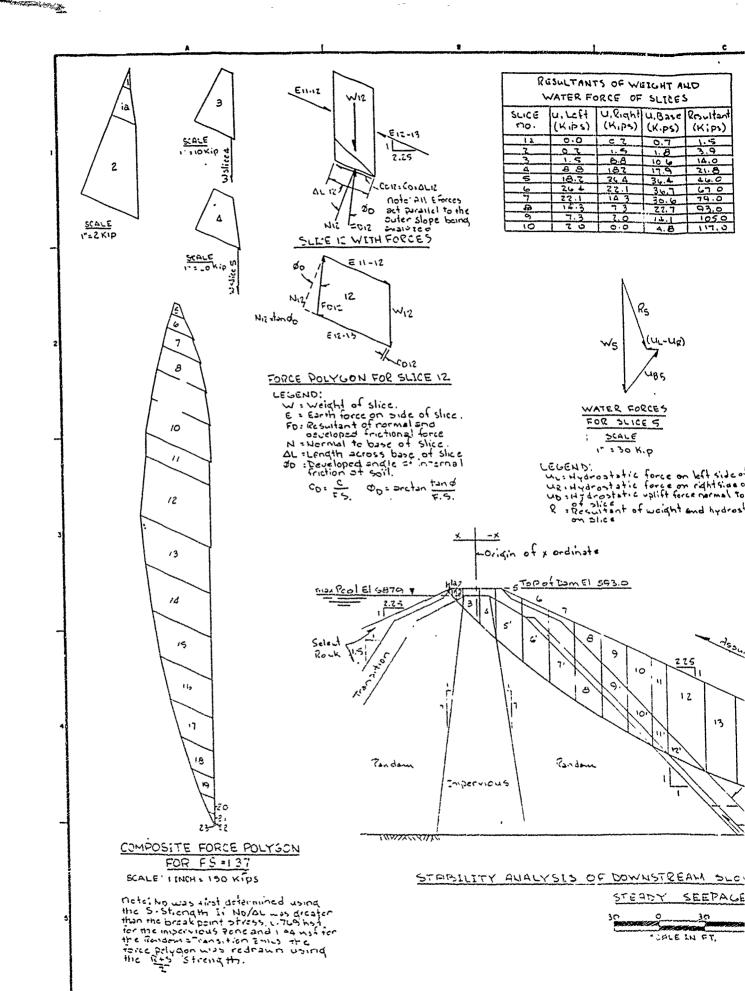
SCALE

(1)

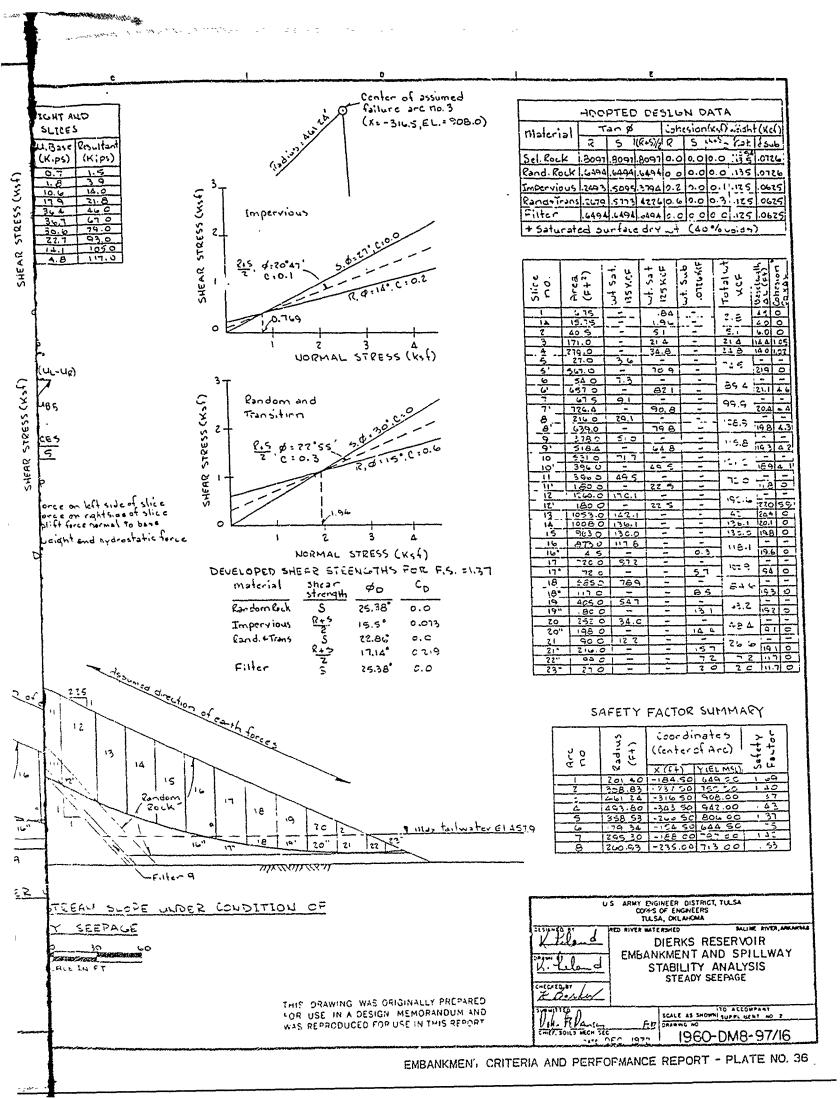
CARRENT LAND

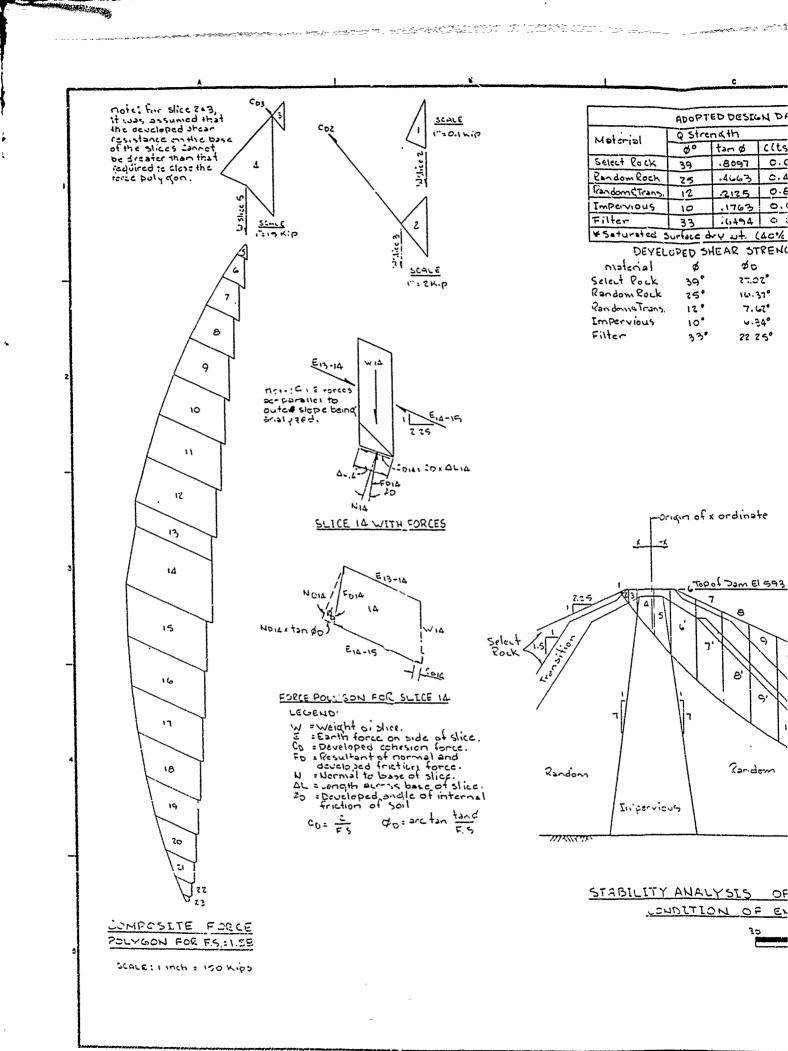
ADOPTED DESIGN DATA isd in Soil whs. Lbs/ft? Steerding \$0.0 moterials Saturated Submerged * 2 105*4 Ø 39 £ Select Rock 72.6 AREA SCALE 330 Random Rock ø 135 72.6 Ø . 5 Randons 125 62.5 14. Impervious 125 62 5 15 C.3 Transition 62.5 175 normal component * Ø : Angle of internal friction (degrees)
C: Cohesion (Tons/ft2) Curve Arca G ++ saturated surface dry wt (as/6 vaios) = 6.3 Sq.im. Ingential Component Curve FORCES ACTING ON FAILURE ARC Arca F = 4.72 34.in Positive tangential force Arca G Nogative tangential force Arca H = 315.00 Ton =-77.50 Ton Area b :305 2 Total = 237 50 TON E. -Area A + 1005 Sq.in. Arca E . 1.05 Sq in. Arcallelins Sqiin NORMAL AND RESISTENG FORCES Mormal force Area A normal force Area B normal force Area C normal force Area D -Area C : 45 59. m. 2 5,75 Tom 2 0.40 Tom 2 70 Tom 1 152,50 Tom SUMMATION OF YORMAL AND TANGENTIAL FORCES normal force Area & normal force Area F * \$1 50 Ton Cohesier LC 42.45 Ton . 141.5'x 0.3 5.39 Ton SAFETY FACTOR COMPUTATIONS Formula: S.F. = ZN xtmd+CC equivalent Dan For consolidated undersined (R) Center of Assumed Failure Are no 4 S.C. = (211.00 x tan 33)+(105 c5 xtan 15")+(125 x tan 14")+47.84 origin for coordinate system 237.5 TOP of Dem El. 593 y Select Rock 20mopuse Impervious Random Random Roule NOTES: 77755777555 The sudden drawdown condition was analyzed using the methods outlined in EM 1110-2-1902 Dec 1960. The computer program utilizing EM 1110-2-1902 April 1970 does not provide for the multiple Select Rock weights. wrens, tion sis of upstream slope under coudition of SUBDER BRAWDOWN U.S. ARMY ENGINEER DISTRICT, TULSA COPPS OF ENCINEERS TULSA, CKLAHOMA &o' WATERMED SCALE OF FEET DIERKS RESERVOIR EMBANKMENT AND SPILLWAY STABILITY ANALYSIS SUDDEN DRAWBOWN Dayles THIS DRAWING WAS ORIGINALLY PREPARED FOR USE IN A DESIGN MEMORANDUM AND SCALE AS BHOSH TO ACCOMPANY WAS REPRODUCED FOR USE IN THIS REPORT. 1960 - DM8-97/15 21: DEC 1972

Sen Al Al



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W

ADOPTED DESIGN DATA unit Wt (PCF) Q Stren4th ø° ((tsf) <u>\$ub.</u> tan Ø .8097 0.0 77.6 39 .4663 0.4 72.6 135 0.8 125 62.5 12 2125 0.6 125 .1763 62.5 10 33 :6494 0.0 125 62.5 Surface dry but: (40% Yold =)

ELOPED SHEAR STRENGTH S FOR F.S. = 1.58 ø  $\phi_C$ C CO

-igin of x ordinate

39° 27.02 0 25° 16.37 c.ansf .504 hsf 12 . 7.62* 1.655 1.008Ksf 1.2 Ksf 10. 6.34° -75 + Ksf 33° 22,250 0 0

Sender of insumed failure and the (x=-766.5, EL-8060)

elice no.	10000000000000000000000000000000000000	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Luleight (Kips) 125 16, 52t.	Tetal whigh!	325c length © DL (ft.)	(%, y & l. (Kips)
	a;	9.1	_	5.1 ;	1.8	
_2_	19.C		23	£3	3.5	2.6
3	72.5	-	23	5.0	93	3.3
Δ	257 Q	-	315	31.5	7- 2-7	30
1 2 3 4 5 6 6 7 7 7 7 9 8 9 10 10 11 11 12 12 12 12 12 12 12 12 12 12 12	3960	-	23 : 5 46 5 :		1.8 3.5 0.5 7.5 164	
9	34.0 	4.9	_:	93 i	27 3 27 3 21 3 20.5 	
(e^	747.0	-	⇒3.4 ¹	2:	:3.0	:3.6
	545	- 'y	-	۵,۵		
٦٠.	9730		109.1	۵.۵	273	22.3
<u> </u>	990	131	-	1253	_	-
<u>e.</u>	2::2		115.0	-75	21 7	2.3
9_	<u> </u>	34.5	-			_
9	<u>, 688.0</u>	í	103.5		20.5	20.5
16	· 435.0	58.5	6.53			20.5
٥.	711 0	-	65.9		10.0	10 8
11	<b>₹56.0</b>	1753	i -	5:.2		
117	612.0	-	-, -5	3	123	19.3
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12'	1410	1 -	35.:	43.5	18.2	E.A
13	442.3	40.E		<del></del>		<del></del>
13.	1350	1 -	16.8	1232	0 -5	19.3
14	1287.0	1173.7	1 -	T .	-	! -
14'	·35 0	T	1:5 B	; 55 5	27.C	27.0
15	117.0	157 9	.⊍ B	570		
16	1098	146.2	-	1.45.	12.3	3 5
17	1508	1361	1-	301	10 7	IGA
ie	900.5	1121.5	T-	1121.5	16.00	1 = 4
19	7741	1361	1-	1 164.5	F = 5	133
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21	477.0	4 4د د	1 =	24 6	1 18 5	1 5 3
72	20-	43.1	=	40.1	1 8 5	1 2 3
23	391	43.1	<del> </del>	113.4	187	2 5   G &   E &   E &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &   G &
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### SAFETY FACTOR SUMMARY

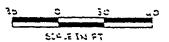
٠,	isadius (Ff.)	coordinates	과 b
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4	ر تر،	X (FT) IY (EL MIL)	\$ C
		WNSTREAM	
1	201.40	-184.50.449.00	1.90
2	306 93	-737.501755 50	1.60
3	461 24	-316.50 1908.00	اص. ا
4	493.80	1-343.501942.00	172
5	359 53	1-266.501806 00	1.58
و	179.34	1-154 50 644 50	2.01
	1395.30	1-188.001797 00	102
Ð	7:00.93	1-235.00.713 00	1.72
		UPSTREAM	
9	1335.82	767,00,177.00	1,71
10	509 17	348.50:951.00	1.76
11	217.48	186.50 658 30	202
12	171.52	154.00 : 635.00	2.21
13	1485.61	311 00 1947.50	1.83
IΔ	276 24	257.00 1721.00	1.60
15	356 09	238 50 : 831.00	1.23
16	216.04	18100 1704 50	7.63

Top of Dam El. 5930 Forces 12 14 15 16 Candom 17 ۱a Pock / 19 20 21 22 777*7*77777777 -=ilter A

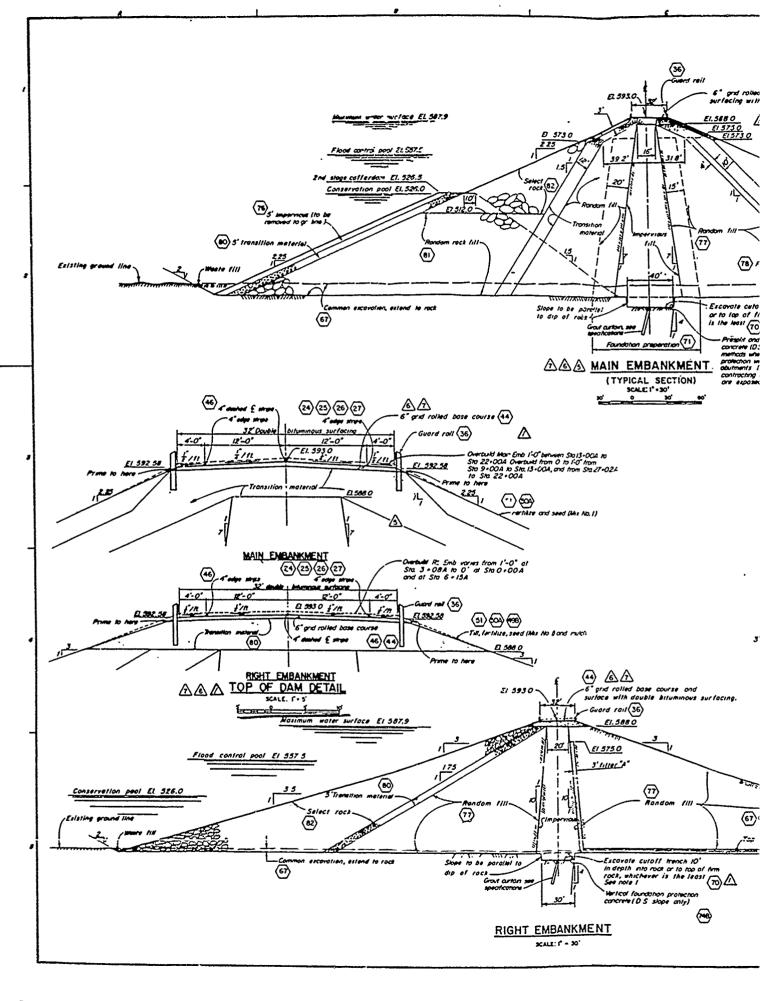
THIS DRAWING WAS ORIGINALLY PREPARED FOR USE IN A DESIGN MEMORANDUM AND WAS REPRODUCED FOR USE IN THIS REPORT

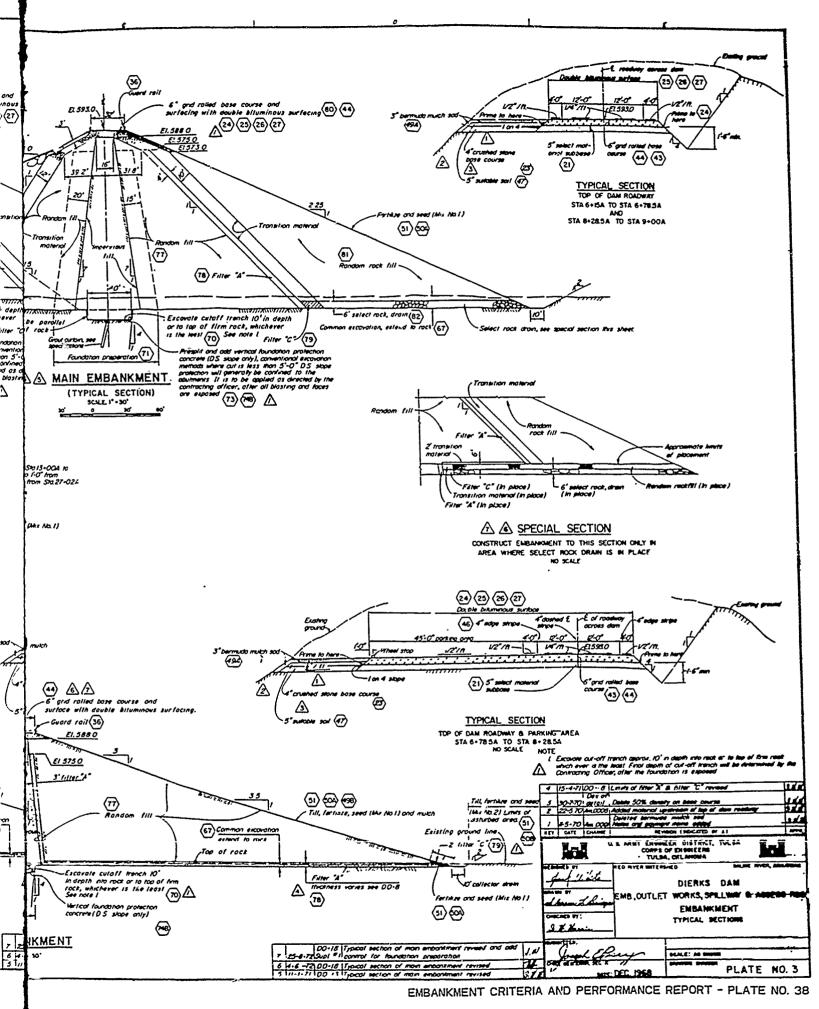
OF DOWNSTREAM SLOPE ULBER

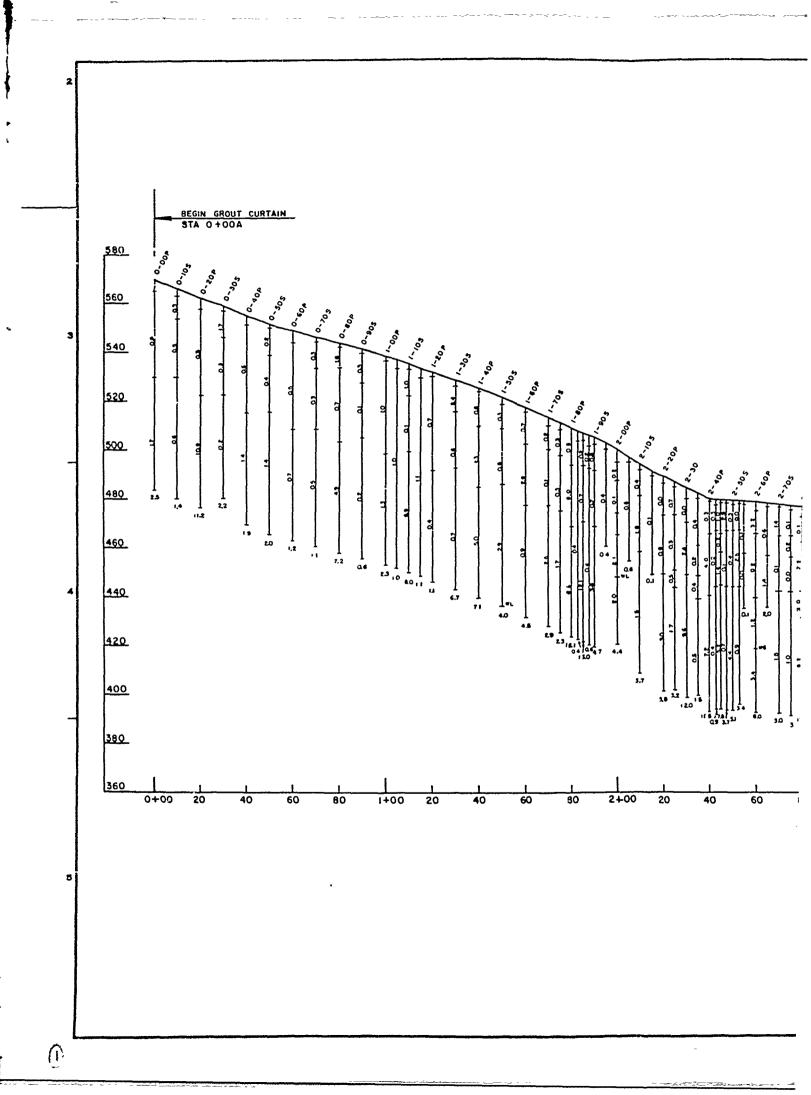
END OF CONSTRUCTION

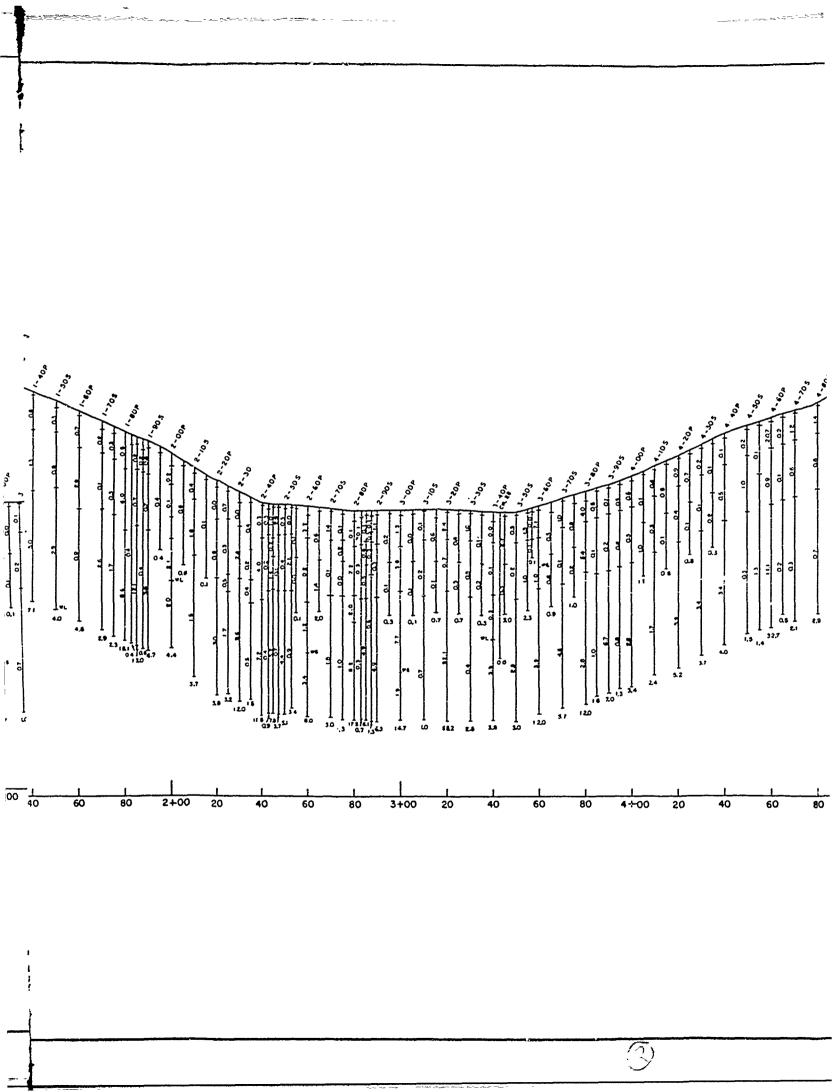


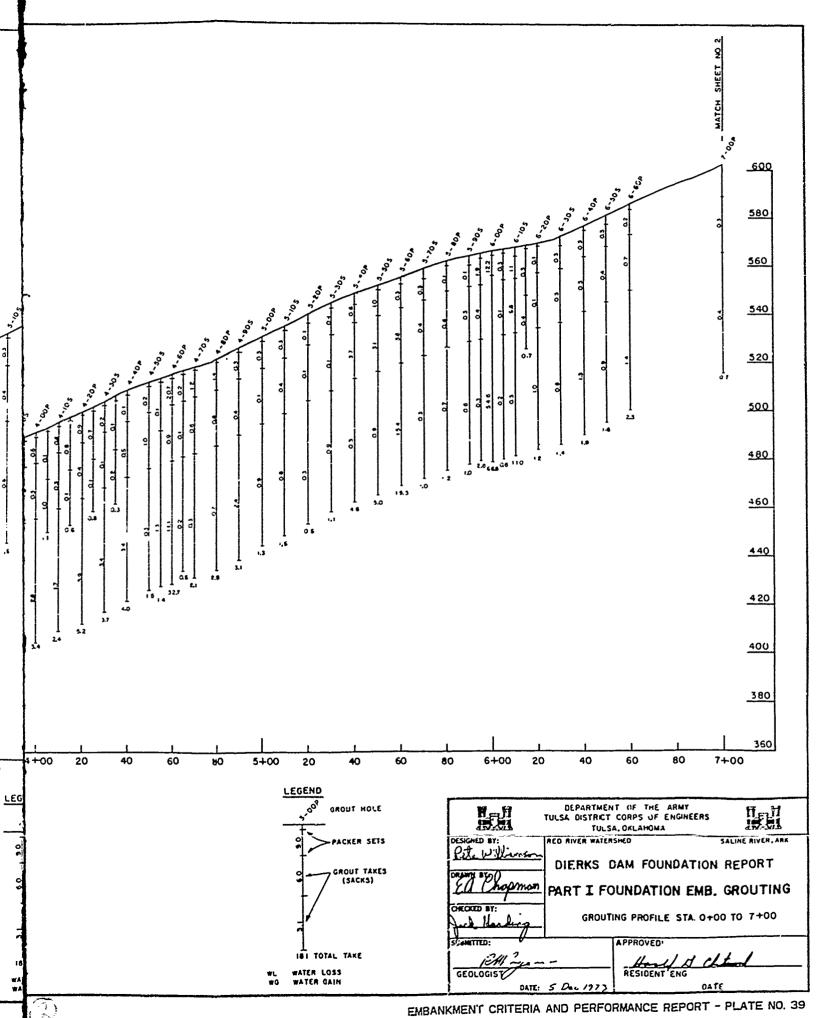
U,	CORPS TULS	ineer district, tulsa of engneers a, oklahoma
EMBAN		DIERKS RESERVOIR KMENT AND SPILLWAY TABILITY ANALYSIS END OF CONSTRUCTION
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CALL SOICE MICH SEC. 1972		1960-DM8-97/17

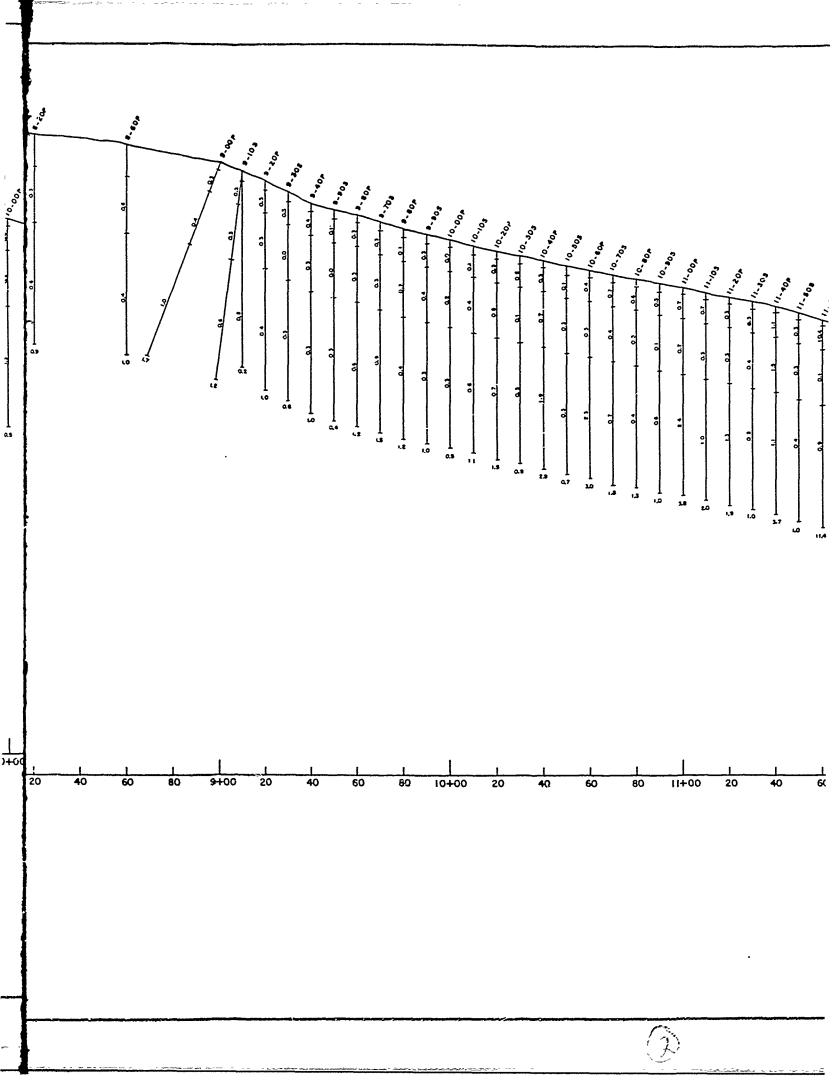


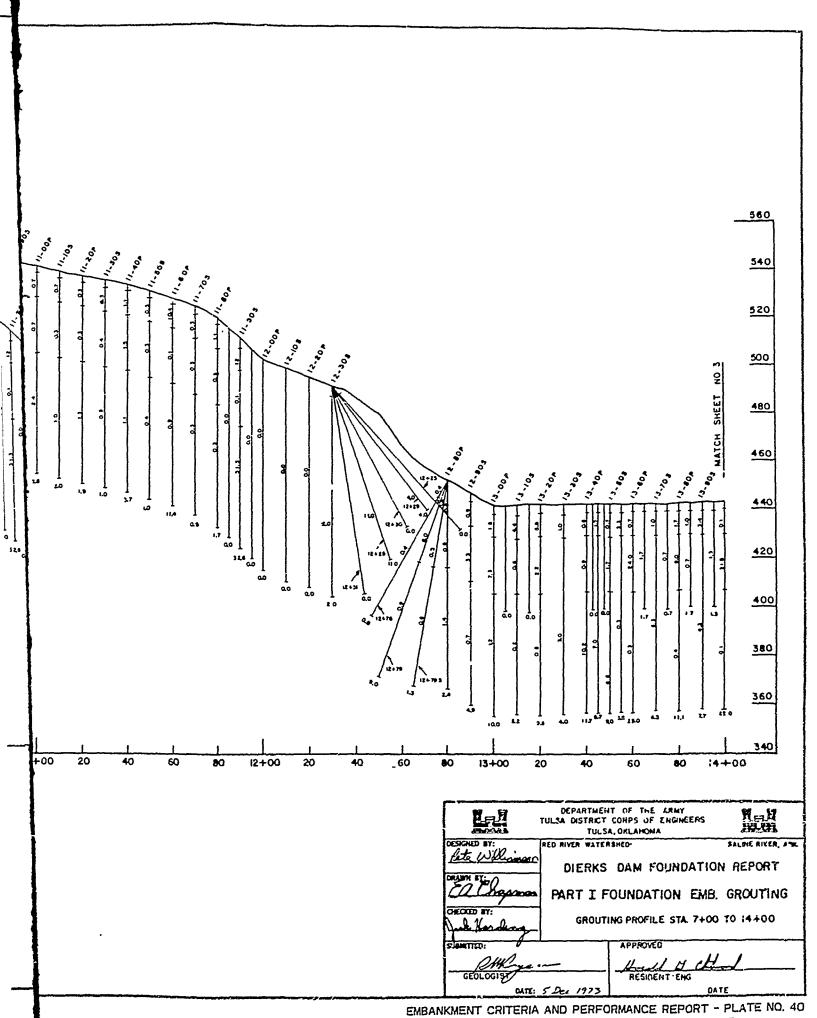


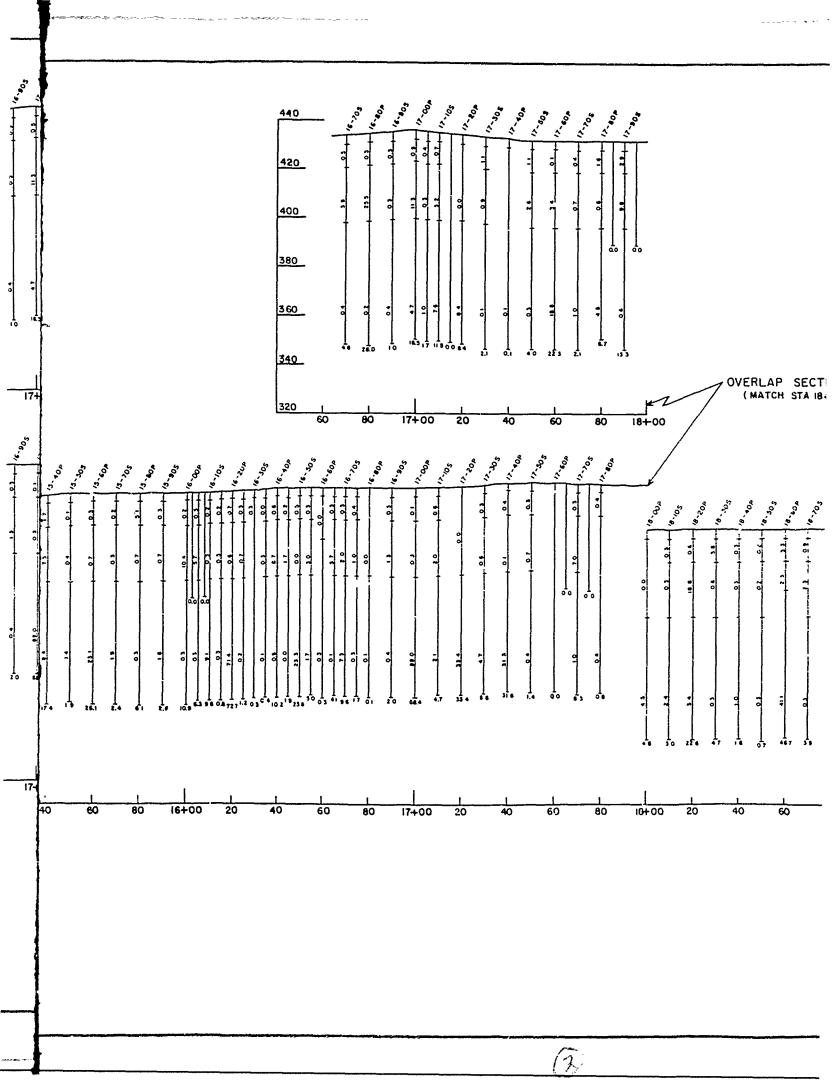


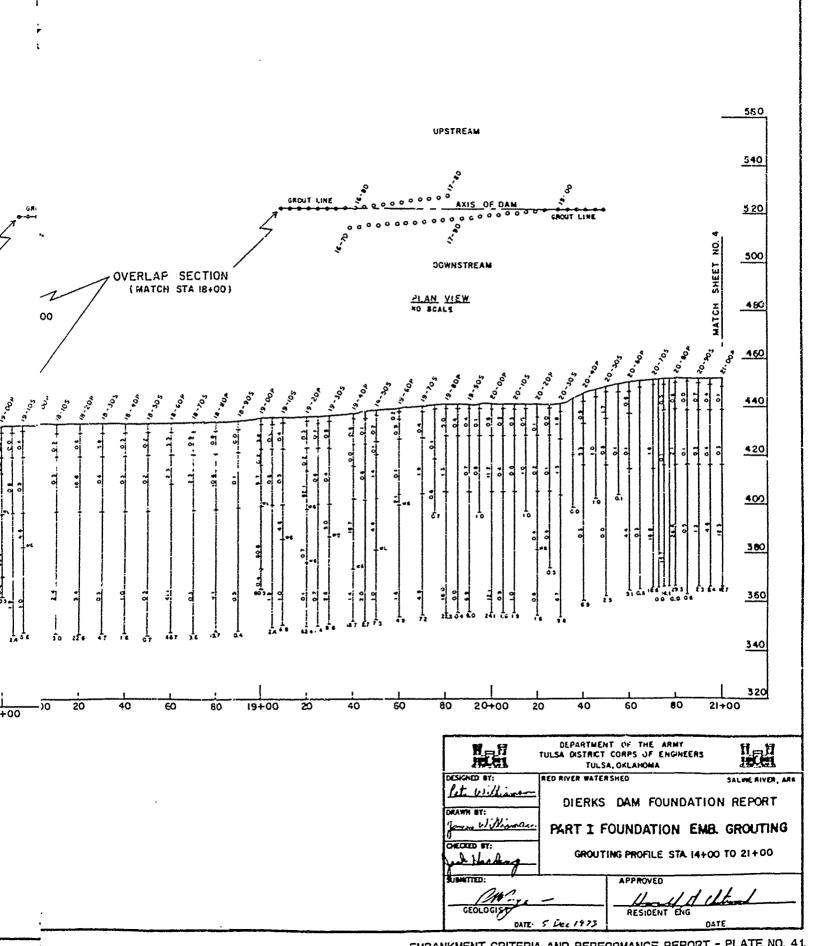


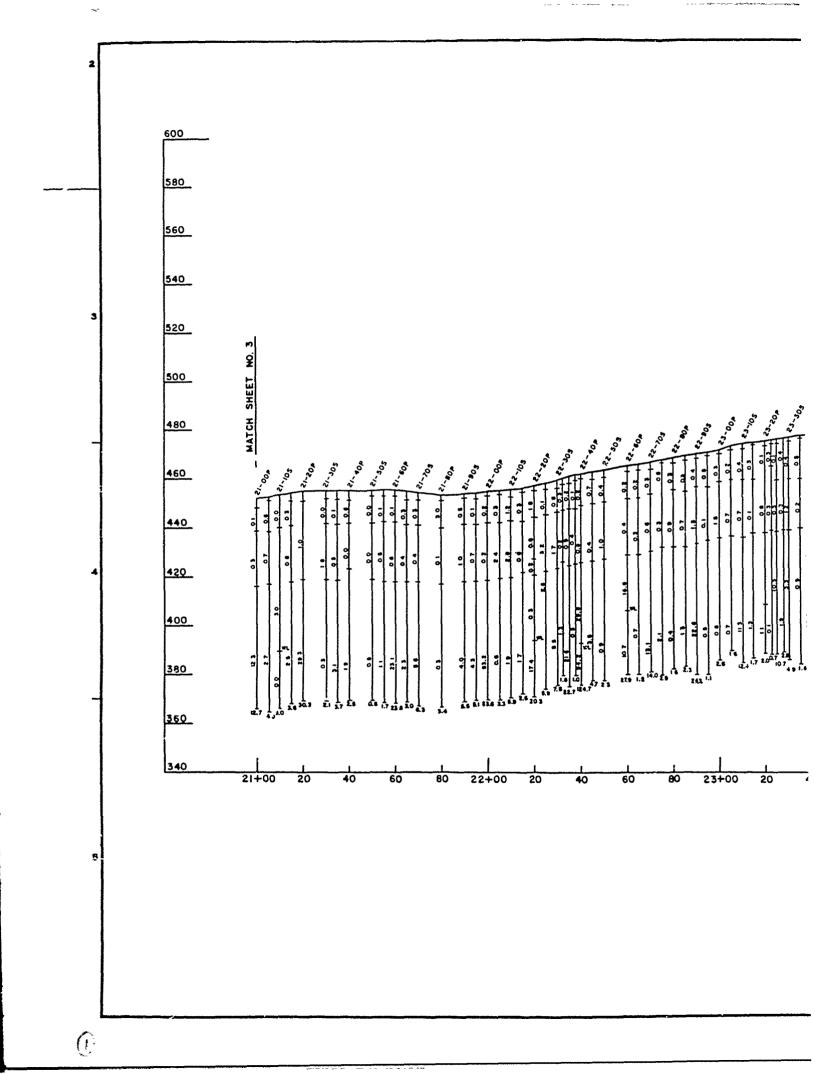


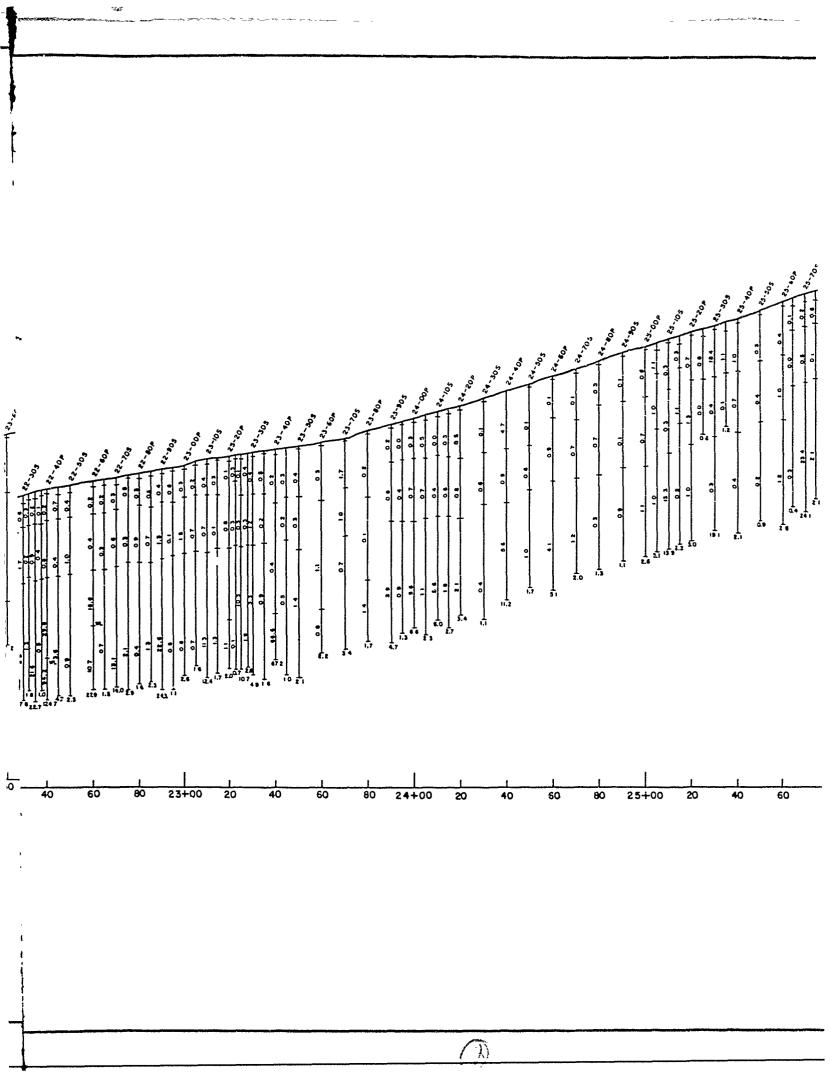


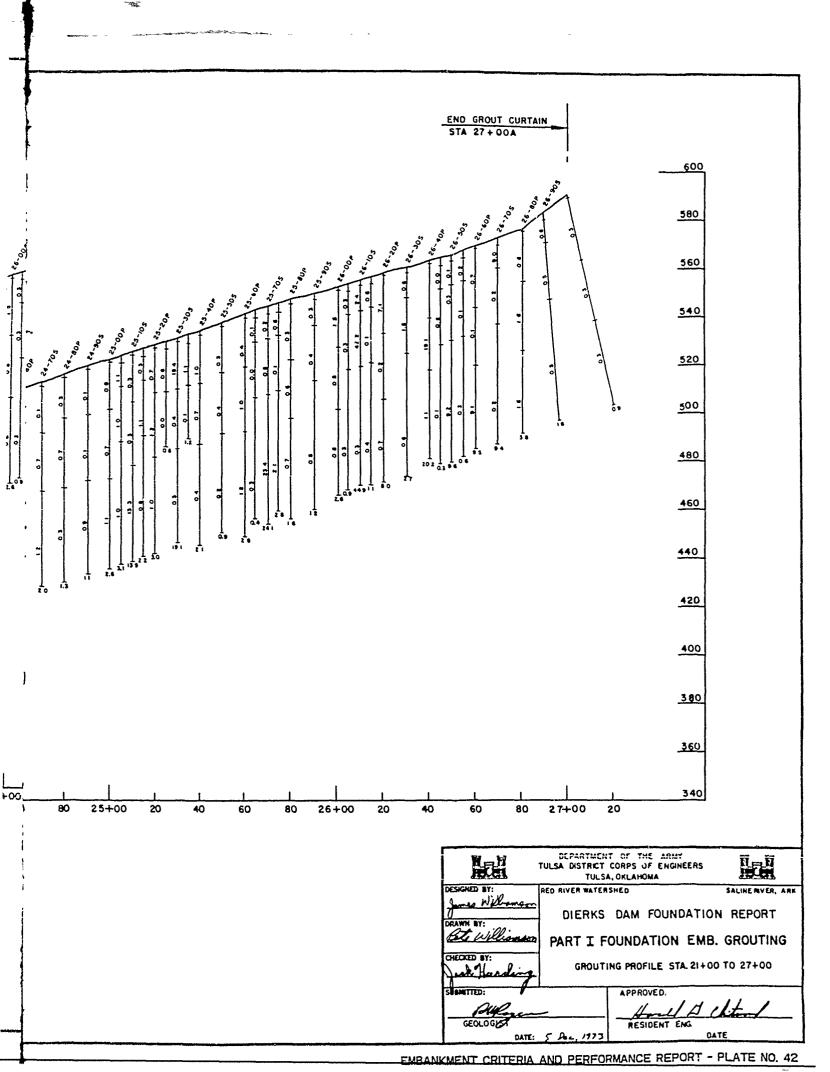


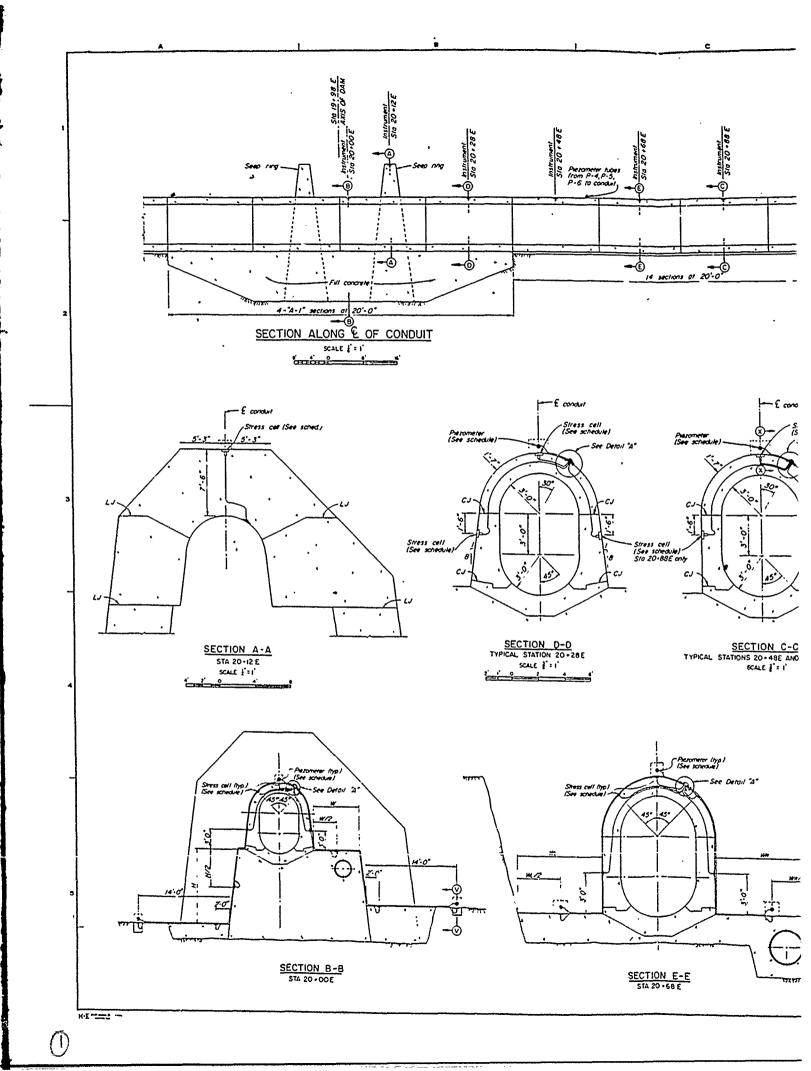


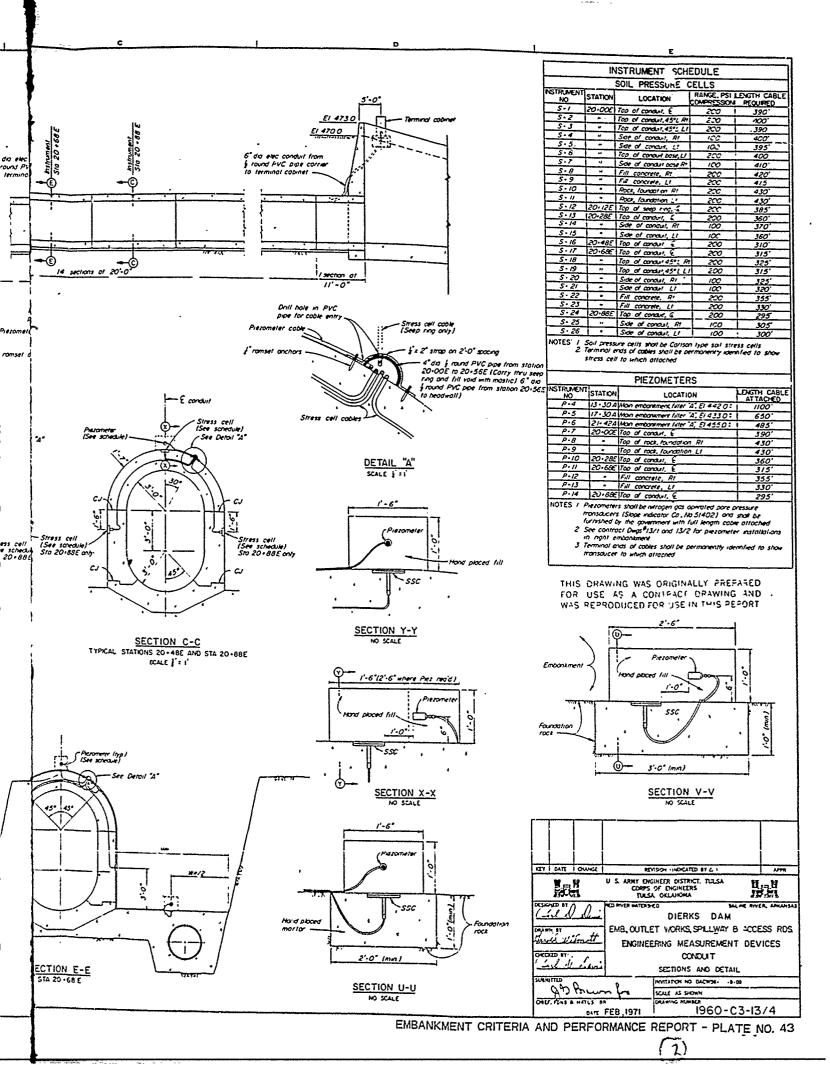


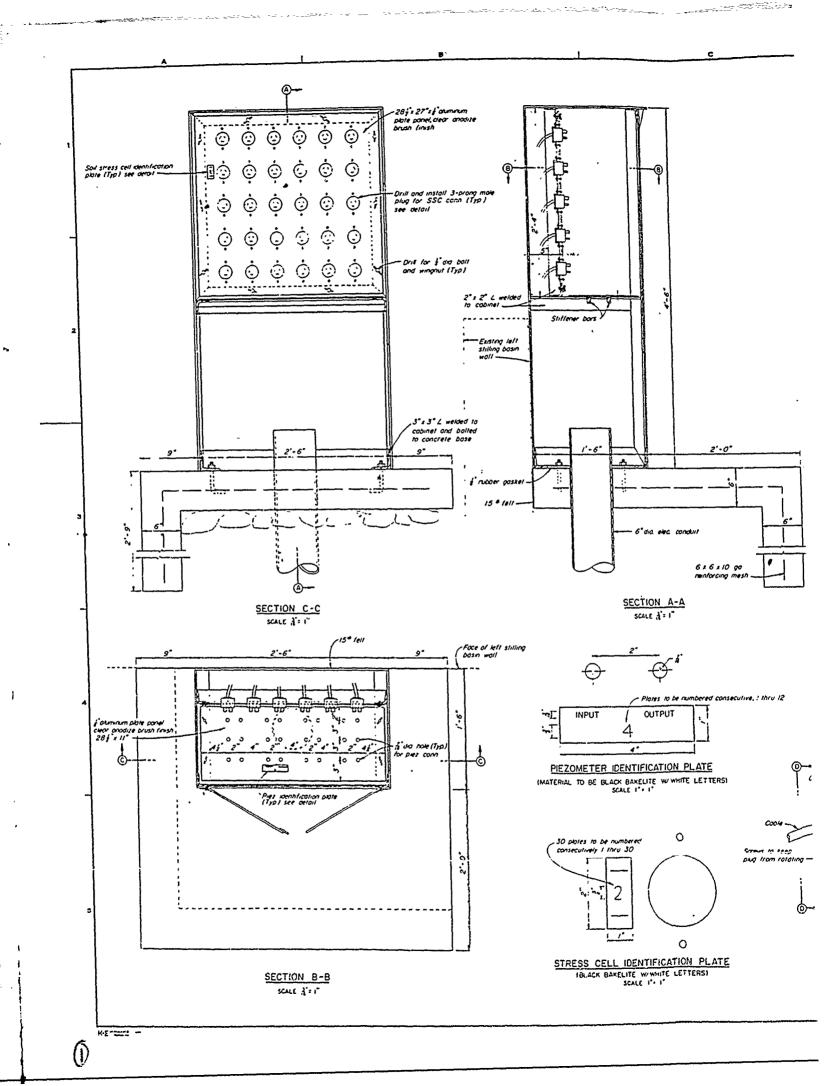


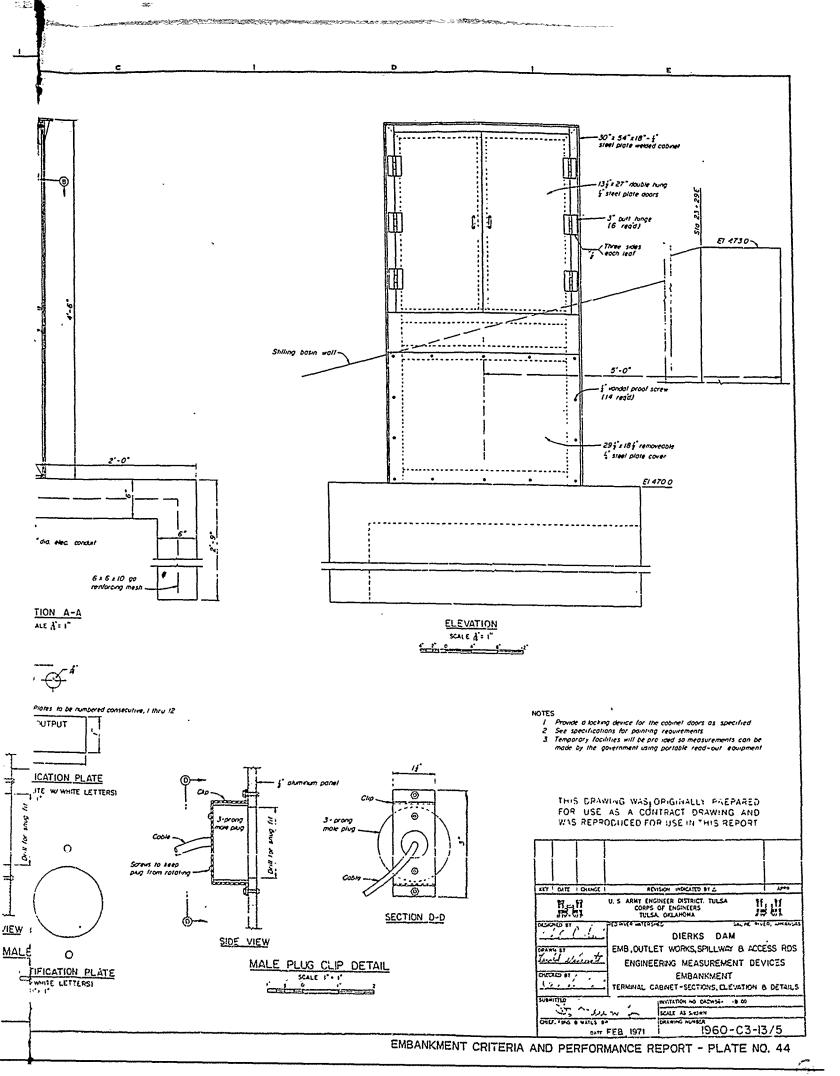


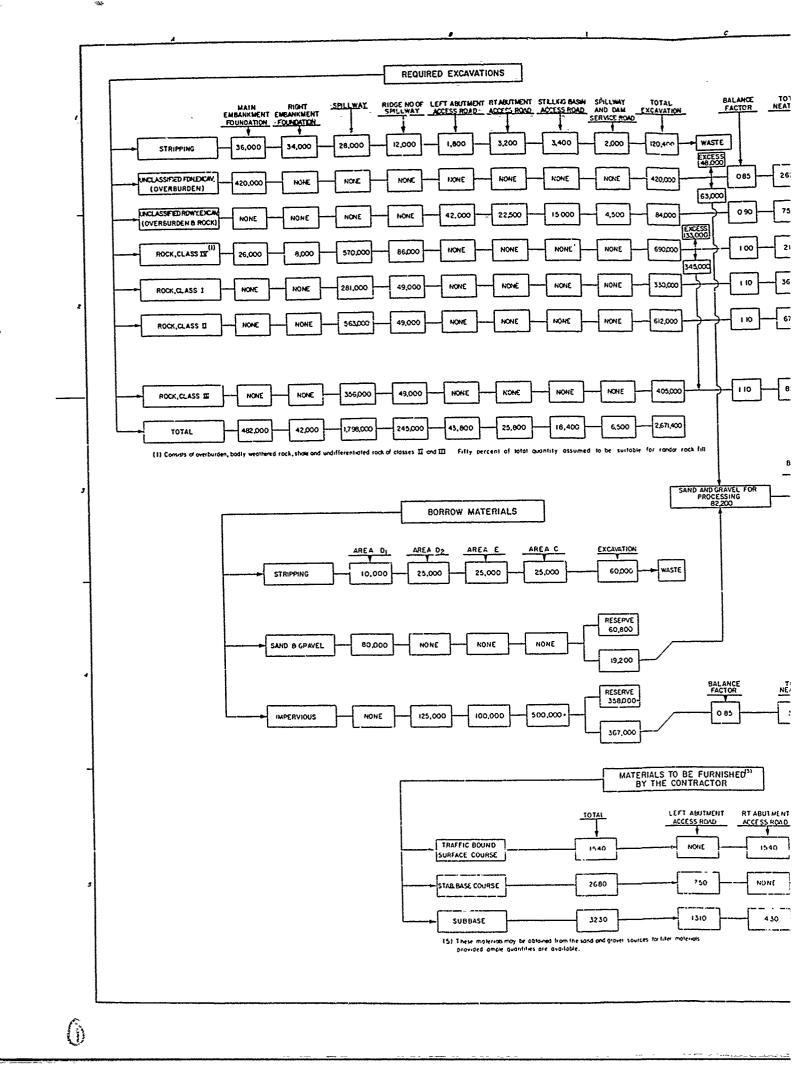


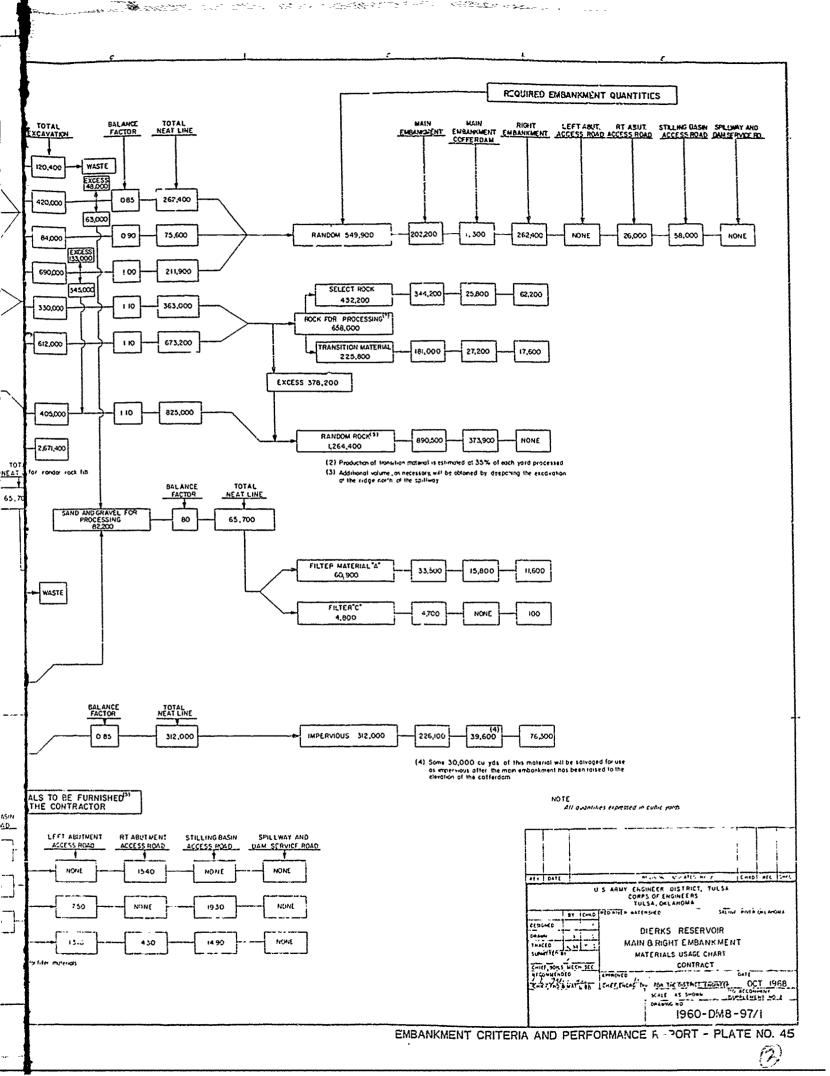


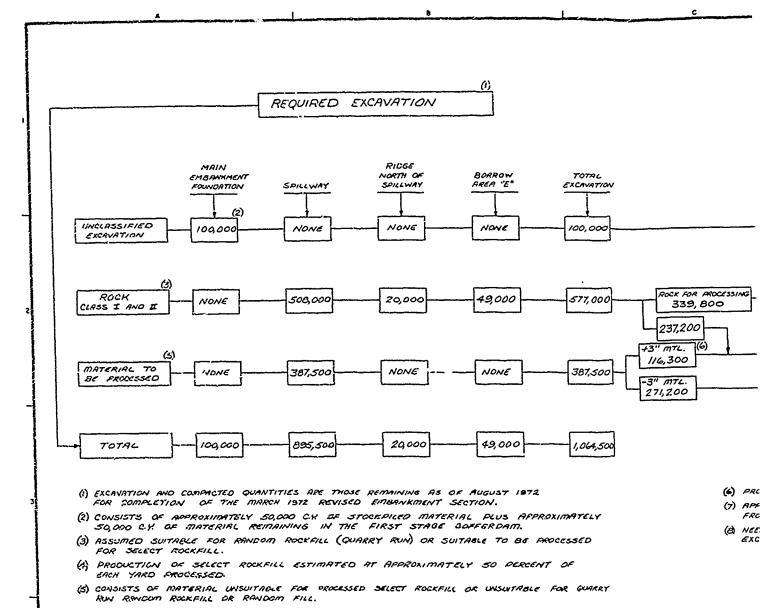












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